

Sierra Vista Specific Plan

CITY OF ROSEVILLE, CA

PRELIMINARY DRAINAGE AND STORMWATER MASTER PLAN

October 23, 2009

Prepared For
MacKay & Soms

Prepared By:



**CIVIL ENGINEERING
SOLUTIONS INC.**
1325 Howe Avenue Suite 202
Sacramento, CA 95825
(916) 563-7300

JOB # 2005.18



Sierra Vista Specific Plan

CITY OF ROSEVILLE

PRELIMINARY DRAINAGE AND STORMWATER MASTER PLAN

October 23, 2009

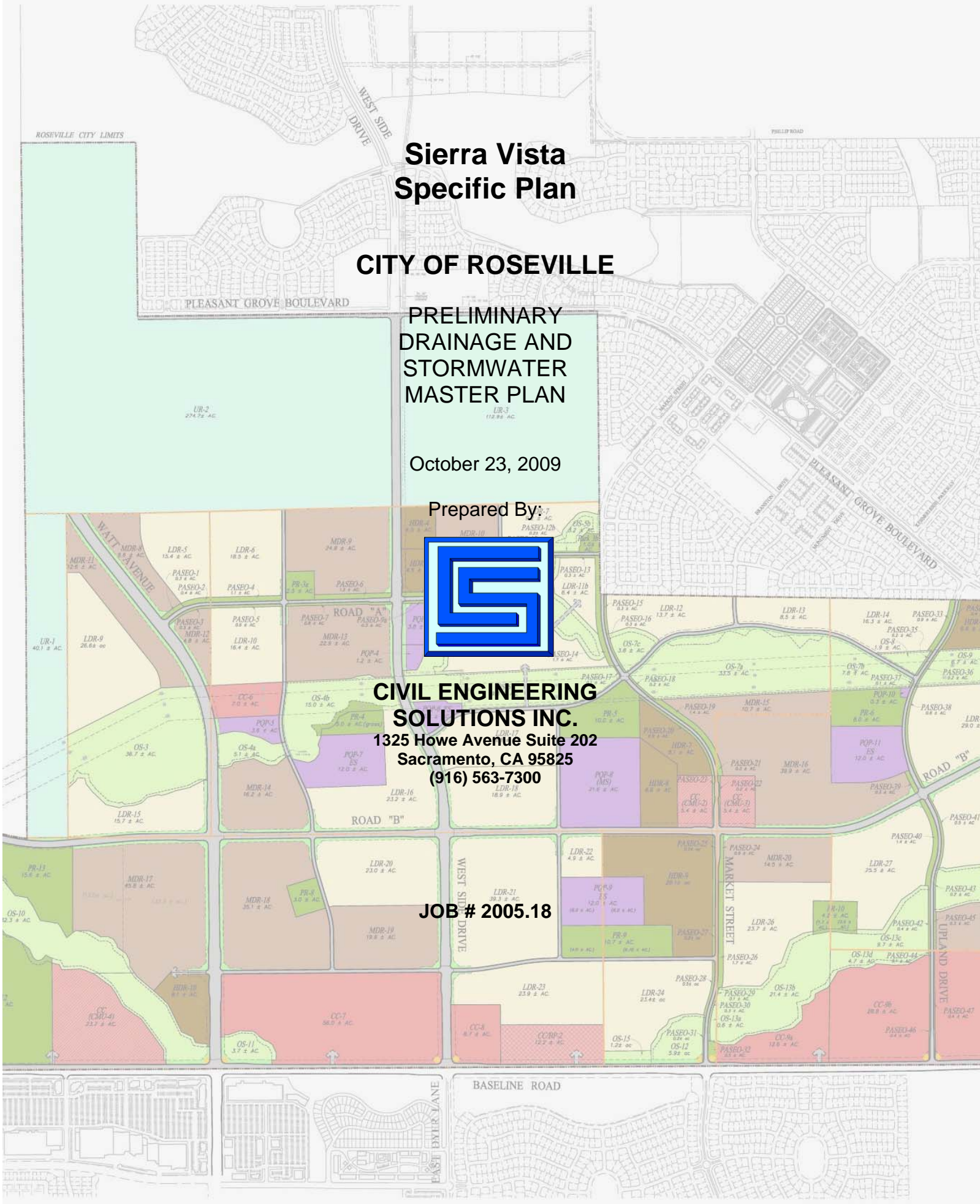
Prepared By:



**CIVIL ENGINEERING
SOLUTIONS INC.**

1325 Howe Avenue Suite 202
Sacramento, CA 95825
(916) 563-7300

JOB # 2005.18



Sierra Vista Specific Plan

CITY OF ROSEVILLE, CA

DRAINAGE AND STORMWATER MASTER PLAN

October 23, 2009

Prepared For
MacKay & Soms

Prepared By:



**CIVIL ENGINEERING
SOLUTIONS INC.**
1325 Howe Avenue Suite 202
Sacramento, CA 95825
(916) 563-7300

JOB # 2005.18



TABLE OF CONTENTS:

List of Appendices.....	3
List of TABLES	3
List of Figures.....	4
I. Introduction:.....	6
I.A Project Description	6
I.B Conclusions	9
I.C Project Phasing	10
II. Hydrology:	11
II.A Standards:	11
II.A.1 Soils:	11
II.A.2 Land Use:.....	13
II.A.3 Watershed Delineation:.....	13
II.A.4 Storm Centering:.....	14
II.A Pre-Project Model:.....	15
II.A.1 Base Model	15
II.B Post-Project “Future, Fully Developed, Unmitigated” Model:	16
II.C Post-Project Mitigated Model:.....	19
II.D Hydromodification Evaluation Post-Project Hydrology Model:	21
II.E Summary of Findings:	24
III. Hydraulics:.....	27
III.A Flood Plain Analysis:.....	27
III.A.1 Pre-Project Hydraulic Model Parameters:	28
III.A.2 Post-Project Mitigated Hydraulic Model Parameters:	28
III.A.3 Future, Fully Developed, Unmitigated Hydraulic Model Parameters:	29
III.B Storm Drain System:.....	50
III.C Mitigation and Drainage Improvement Phasing:	51
IV. Water Quality:.....	54
IV.A Stormwater Management During Construction Activities.....	54
IV.B Post Construction Stormwater Management.....	55
IV.B Low Impact Development Measures	56
IV.B.1 Volume Reductions from LID measures:	58
IV.B.2 End of Pipe Treatment:.....	61
IV.C Hydrograph Modification Benefits from LID:.....	64
IV.C.2 Peak Flow Response:	64
III.C.3 Hydrograph Characteristics:.....	66
III.C.4 Hydrograph “Flow Duration”:	66
V. Volumetric Impacts:.....	69

List of Appendices

A.	Created Wetlands Narrative
B.	HEC-1 100-year Pre-Project Model Input File
C.	HEC-1 100-year Post-Project Unmitigated Model Input File
D.	HEC-1 100-year Post-Project Mitigated Alternative Model Input File
E.	HEC-RAS Pre-Project Summary Table
F.	HEC-RAS Post-Project Summary Table
G.	Onsite Storm Drain Analysis 10-year and 100-year CSDS Reports
H.	Stormwater Quality Calculations – LID Calculations
I.	Project Disk
J.	Oversized Exhibits:
	SH1-SV Existing Regional Watershed Map
	SOIL-1 Hydrologic Soils Groups Map
	SH-1 Pre-project Watersheds
	SH-2 Post-project Watersheds
	FP-1 Pre-project Floodplain Map
	FP-2 Post-project Mitigated Floodplain Map
	FP-3 Future, Fully Developed, Unmitigated Floodplain Map
	FP-4 Comparison of Pre-project to Post-project Floodplain
	SD-1 Storm Drain Exhibit
	AT-1 Attenuation Plan

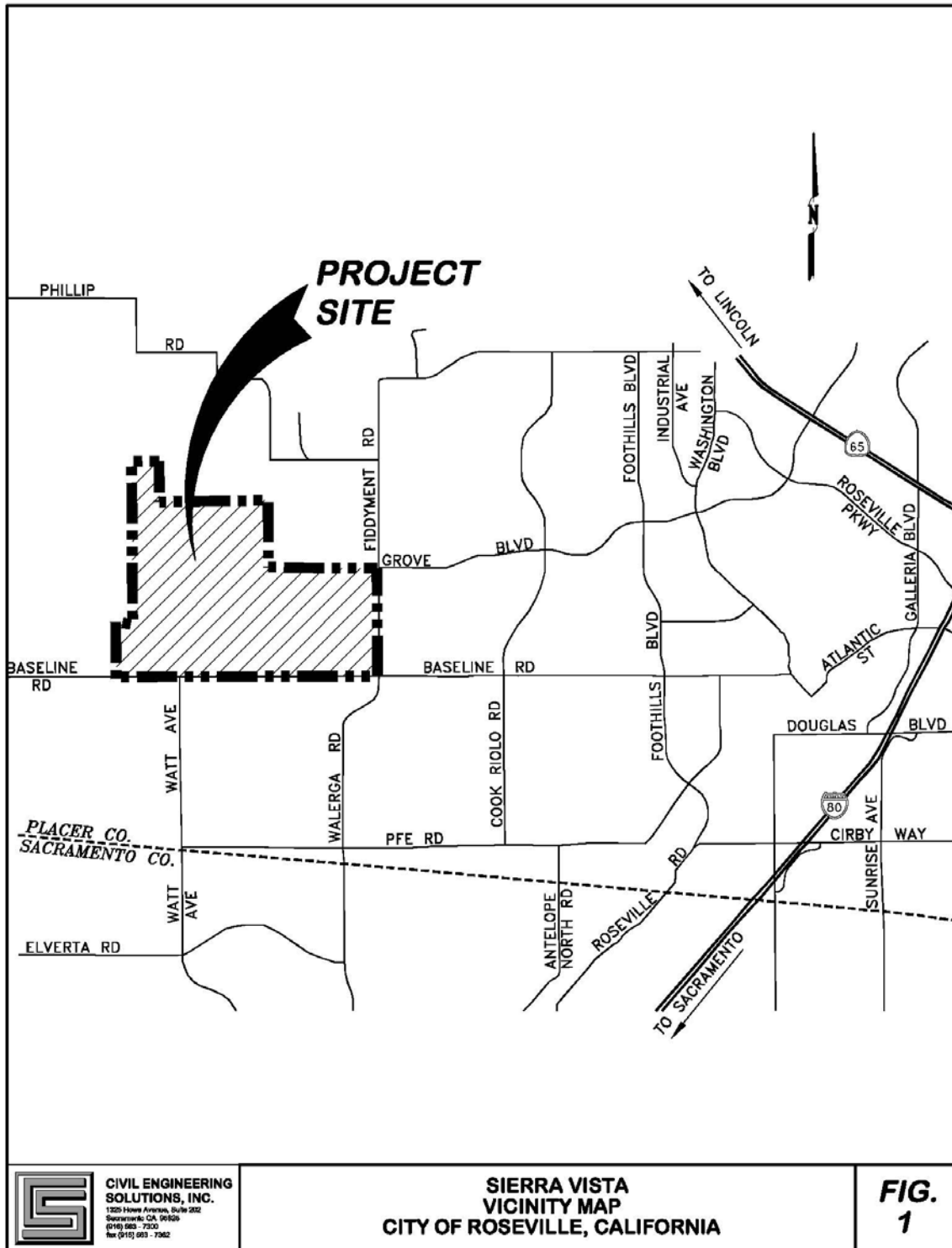
List of TABLES

TABLE II.A.1 – Pre-project Base Model Hydrologic Factors	15
TABLE II.B.1A – Post-Project Un-mitigated Hydrologic Factors	17
TABLE II.B.1B – Post-Project Un-mitigated Hydrologic Factors (Continued).....	18
TABLE II.D.1 – IMPERVIOUS AREA REDUCTIONS FOR LID	21
TABLE II.D.2A – HYDROLOGIC WATERSHED FACTORS – LID REDUCED	22
TABLE II.D.2B – HYDROLOGIC WATERSHED FACTORS – LID REDUCED (CONT).....	23
TABLE II.E.3A – 100-YEAR (& 10-YEAR) PEAK FLOW COMPARISON	24
TABLE II.E.3B – REQUIRED ATTENUATION CREATION AREAS	25
TABLE III.A.1 – PROPOSED CULVERT/BRIDGE SIZES	29
TABLE III.A.2 – PROPOSED PEDESTRIAN CROSSINGS	31
TABLE III.A.3 –Water Surface Comparison (Pre- and Post-Project).....	32
TABLE III.C.1.....	51
TABLE III.C.2 – PROPOSED PEAK FLOW MITIGATION VOLUME BY PHASE	52
TABLE IV.B.1 – LID Required Volume Reductions (RVR).....	58
TABLE IV.B.2 – Applicable LID Measures By Development Type:.....	59
TABLE IV.B.3 – Alternative LID Use to Achieve Minimum RVR	60
TABLE IV.B.1 – BMP SIZING AT OUTFALLS	62
TABLE IV.B.2 – MINIMUM VEGETATED SWALE SIZING AT OUTFALLS.....	63
TABLE IV.C.3: HYDROGRAPH CHARACTERISTICS AT PROJECT DISCHARGE.....	66
TABLE V.A – VOLUMETRIC IMPACT RATES IN TYPE D SOILS	69

List of Figures

FIGURE I.1 - VICINITY MAP.....	5
FIGURE II.A.1 – REGIONAL HYDROLOGIC SOILS GROUPS	12
FIGURE II.A.2 – SIERRA VISTA SPECIFIC PLAN PROPOSED LAND USE	13
FIGURE II.C – LOCATIONS OF IMPROVEMENTS PROPOSED IN STREAM CORRIDORS	20
FIGURE III.A-1 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	33
FIGURE III.A-2 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	34
FIGURE III.A-3 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	35
FIGURE III.A-4 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	36
FIGURE III.A-5 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	37
FIGURE III.A-6 - HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	38
FIGURE III.A-7 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	39
FIGURE III.A-8 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	40
FIGURE III.A-8 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	40
FIGURE III.A-9 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	41
FIGURE III.A-10 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	42
FIGURE III.A-11 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	43
FIGURE III.A-12 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	44
FIGURE III.A-13 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	45
FIGURE III.A-14 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	46
FIGURE III.A-15 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified).....	47
FIGURE III.1 - FIRM MAP 458F, June 8, 1998.....	48
FIGURE III.2 - FIRM MAP 475F, June 8, 1998.....	49
Figure III.C.1	51
Figure III.C.2.....	53
FIGURE IV.C.1 – Peak Flowrate Comparisons Leaving Project.....	65
FIGURE IV.C.2 – Comparison of Pre-project and Post-project Hydrographs 2-year EVENT	65
Figure III.C.4 – Flow-Duration Relative Comparison (logrythmic).....	67
Figure III.C.5 – Flow Duration Relative Comparison (non logrythmic).....	68

FIGURE I.1 - VICINITY MAP



I. Introduction:

I.A Project Description

The proposed Sierra Vista Specific Plan Area (SVSP), approximately 2064± acres (including 427 acres of Urban Reserve), is wholly contained within the Curry Creek watershed, which drains to the Pleasant Grove Canal, then to the Natomas Cross Canal, and then the Sacramento River. The project is located adjacent to the western and southern limits of the City of Roseville. The project extends east from Fiddymment Road approximately 2.5 miles to west of Watt Avenue. The project is bounded by the existing West Park Development to the north, and Baseline Road to the South.

The current use of the project area lands is mostly unimproved. Some actively farmed “row crop” and “grain” lands are present within the project. The predominant agricultural production of the lands is grain crops and grazing fields. Curry Creek enters the project from the east at Fiddymment Road as a small seasonal stream. At Fiddymment Road the stream receives flows from storm drain pipe discharging on the west side of the roadway. The tributary areas upstream of Fiddymment Road are completely urbanized. Curry Creek traverses the southeastern corner of the project, in a southwestern direction towards Baseline Road. Curry Creek then crosses under Baseline Road near the proposed intersection of Market Avenue and Baseline Road. South of Baseline Road, Curry Creek passes through the “Placer Vineyards” Specific Plan Area. Then Curry Creek re-enters the SVSP, near Watt Avenue. Curry Creek then traverses the project in a northwesterly direction until exiting the project boundary, approximately 3,000 feet north of Baseline Road.

Within the central area of the project, a small seasonal swale is present which conveys flows in a westerly direction, joining Curry Creek near Watt Avenue. This project establishes the name for this swale feature as Federico Creek (named after the dominant historical property owner of the area the creek passes through). Federico Creek collects runoff from areas within the SVSP, and offsite urbanized areas at the Westpark development (a 32.25 acre, and 62.22 acre watershed).

In the extreme north portion of the project, the northern tributary of Curry Creek passes through the project boundary in the area which will remain “Urban Reserve” for this Specific Plan. This drainage report has established baseline hydrology and floodplain analysis for this tributary, however, development impacts for the “Urban Reserve” areas are not being assessed in this document, except where those areas will eventually pass through the areas with development land uses identified.

The northern tributary of Curry Creek conveys runoff from the Westpark development, west to the future extension of Watt Avenue at the western boundary of this plan area. West of the Watt Avenue extension, flows are discharged into Placer County at the “Regional University Specific Plan” (RUSP) project areas.

The proposed Sierra Vista Specific Plan, includes a mixture of land uses typically associated with a master planned community including, Low Density Residential (LDR), Medium Density Residential (MDR), High Density Residential (HDR), Commercial, Business Professional, Parks, Schools and other Quasi-Public land uses, and open space land uses are proposed along the

existing Creek Corridors, and power line easements.

Master watershed modeling for Curry Creek has been adapted from the original “Cross Canal Watershed Study” models of 1992, by several projects including: Regional University Specific Plan (RUSP), Placer Vineyards, and Westpark. The initial adaptation of the model was performed with the Westpark development (approved by the City of Roseville), which created a Kinematic Wave hydrology model for only the local areas of the Westpark development. The Placer Vineyards (Preliminary Master Drainage Plan has been accepted by Placer County), similarly prepared a local and tributary areas only update to the Curry Creek hydrology Analysis. The RUSP (Preliminary Master Drainage Plan has been accepted by Placer County) incorporated the Westpark and Placer Vineyards hydrology into a comprehensive Curry Creek hydrology model (Kinematic Wave) per the Placer County “*Stormwater Management Manual (SWMM)*” dated February 1994 and the SWMM Addendum 1, dated October 1997. The RUSP comprehensive model (which covers the Plan area) is used as the base analysis models for this plan. The existing conditions model assumes the Westpark development and urban areas east of Fiddymont Road are fully developed in the pre-project conditions analysis. The RUSP is shown as undeveloped in the plan models of the pre-project conditions. This would have little impact on this plan analysis as it is downstream of the SVSP, and if developed would propose to lower water surface elevations at the proposed extension of Watt Avenue. The proposed development at the Placer Vineyards project is also included in the analysis as developed in the pre-project analysis.

A detailed hydraulic analysis (HEC-RAS) of Curry Creek and the tributaries which will remain in their native state after the project is built, is provided for the pre-project and post-project conditions. Hydrology (flows) are input into the hydraulic floodplain analysis models based on the Pre-project, post-project unmitigated, and Post-project mitigated HEC-1 analysis’. Floodplain Exhibits (FP-1, FP-2, and FP-3) delineate the pre-project, post-project mitigated, and post-project unmitigated conditions floodplain limits and base flood elevations for the 100-year and 10-year events. A limited amount of detailed FEMA floodplain or floodway limits is currently defined within the SVSP near the western plan boundary. These areas are shown on the “FP” drawings for reference, as approximate Zone A. These exhibits are included in APPENDIX J.

All Drainage Improvements in the plan will comply with the City Design Standards, “Section 10 – Drainage”. Stormwater Quality design elements will comply with the City Standards, “[Stormwater Quality Design Manual for the Sacramento and South Placer Regions](#)”.

Per the Placer County Stormwater Management Manual(SWMM), Section VII.D.1a specifies sufficient onsite detention is required to mitigate post project runoff rates to less than the pre-project runoff rates as follows:

When storage is to be used to mitigate downstream impacts due to increased flows generated by development of a site, the objective flow shall be taken as the estimated pre-development peak flow rate less 10 % of the difference between the estimated predevelopment and post-development peak flow rates from the site for all standard design storms ranging in frequency from the 2-year and up to and

including 100-year. In no case, however, shall the objective flow be less than 90 percent of the estimated pre-development flow. Figure 7-1 presents this criterion graphically.

This project proposes to mitigate potential impacts to peak flood rates for the 2-year, 10-year and 100-year event as specified in the SWMM. The project proposes to mitigate peak flow impacts through the creation of additional on-stream and floodplain overbank attenuation at wetland creation areas within the open space stream corridors. Additionally, the project will employ the use of Low Impact Development (LID) measures, which will have a minor effect on the directly connected imperviousness of the plan developments, and provide some offsets for the development impacts. Within the overbank areas, conveyance and attenuation in excess of the existing conditions will be constructed as wetland creation areas. These areas will enhance the attenuation characteristics of the overbank areas. The addition of bridges and culverts is also expected to increase the in-stream storage characteristics of the floodplain. The included analysis demonstrates that adequate mitigation is provided within these systems to reduce runoff rates exiting the project limits per the SWMM requirements, without increasing 100-year hydraulic grade line elevations offsite.

The plan will also construct two traditional permanent “detention basins” for the purpose of stormwater peak flow attenuation. These basins will be constructed in series within the Power line corridor in the northcentral area of the project, providing ~ 20 acre feet of added storage within the Phase A project area. The project will mitigate peak flow increases via the proposed open space construction features, including: detention basins, created wetlands and stormwater quality features. Attenuation benefits of the proposed wetland creation areas, and stormwater quality features are quantified on Figure AT-1, the “Attenuation Plan”.

The plan will construct “vegetated swales” within the Open Space corridors to convey and treat stormwater prior to discharging into the receiving creeks. Some of these features are within the overbank areas of the creeks and will be within the floodplain, and therefore will also contribute to overall floodplain storage.

The project will construct both: on-site LID, and treatment “Best Management Practices” (BMPs) to mitigate and treat the discharge of development constituents into the creeks and streams. Low Impact Development (LID) measures will be employed within the developments to reduce the amounts of runoff and required treatment from the projects. This plan will establish LID required volumetric removal (RVR) targets/quotas for each type of land use. This report also provides a sample plan identifying an example of how those targets will be obtained for each type of land use. The RVR’s in conjunction with the proposed outfall vegetated swales, will obtain 100% mitigation of increased project runoff volumes for the 85th percentile rainfall event. The LID RVR’s will be the minimum threshold of LID elements allowed in the plan, although LID elements may be substituted, and greater levels of LID will be permitted, which could further reduce BMP treatment sizing. The combination of Best Management Practices and Low Impact Development Measures complete the elements of Source Control, Runoff Reduction, and Treatment Control measures included in the plan, which are referred to as the “Project Treatment Train”, providing treatment and volumetric reduction measures to the maximum extent practicable for this plan.

The LID measures proposed, and the flood storage volumes being added within the floodplain areas will assist in the reduction of the potential for hydrograph modification. Hydrograph Modification (otherwise known as Hydromodification or Hydromod) can impact stream systems causing them to move into a state of accelerated evolution. Hydromodification impacts can occur resulting from non-mitigated development impacts, wherein added impervious areas can increase the total volume of runoff, and development storm drainage systems can accelerate the timing of the runoff, increasing the frequency of flows especially in smaller events. Hydromod is known, in some streams, to trigger stream evolution changes to occur in a more rapid time scale than would have normally occurred. Sudden evolution of a stream can result in the removal of significant amounts of soils from the project streams, and deposition of those soils elsewhere in the watershed. Sudden evolution is therefore a significant concern for water quality regulated streams (see Clean Water Act, Waters of the US and other water bodies). The Curry Creek stream includes areas identified as seasonal and perennial wetlands and therefore would be considered a water of the US. Migration which is a more natural occurrence, as the “S” curves of all streams naturally migrate in the downstream direction. In our surveys of the Curry Creek stream system we have seen evidence of scour and deposition which may be indicators of either migration and evolutionary issues. Because of the potential concerns over the Hydromodification impacts of this plan, an analysis is provided in this study which evaluates the hydromod related potential impacts of the project, and the benefits of the proposed project measures which are likely to mitigate most of the modification concerns.

Finally, as this development is contained within a watershed which discharges through the Natomas Cross Canal, it is subject to volumetric impacts analysis and potential mitigation requirements. A situation exists within Sutter County, within the sump areas upstream of the Natomas Cross Canal, wherein flooding is known to occur when the Sacramento River rises above a flood stage of 37.0 at the Verona Gage. This occurs as a result of the limited discharge capacity of the Natomas Cross Canal when the Sacramento River is flooding. As a result, the discharge of additional volumes of runoff during this type of event, could make the depth of flooding worse (per Cross Canal Watershed Study CH2MHILL – 1992-1994). Additionally, flooding of the sump area could occur from local runoff of the streams and watersheds which are tributary to the Natomas Cross Canal. As a result the City of Roseville collects a development fee, “The Pleasant Grove Watershed Mitigation Fee”, which will ultimately construct facilities (Reason Farms) to mitigate such impacts. This report will quantify the estimated volumetric impact of the project for a 8-day 100-year event.

I.B Conclusions

Peak flow rate increases which would result from the proposed development will be mitigated through the excavation of wetland creation areas, vegetated treatment swales, detention basins and additional conveyance within the overbank areas of Curry Creek. The project plan will create **91.1** additional acre feet of storage within the 100-year floodplain, which will mitigate peak flow increases for the 100-year event.

The development of the project has the potential to impact water quality in Curry Creek and its tributaries. As a result Best Management Practices (BMP’s) and Low Impact Development (LID) measures will be implemented to reduce the potential of impact. LID measures will be

incorporated into the various development plans consistent with the guidelines of the proposed NPDES Stormwater Permit (http://www.waterboards.ca.gov/water_issues/programs/stormwater/constpermits.shtml), such that 100% of the volumetric increase of a 85th percentile event will be mitigated onsite through LID measures.

The development of the project will impact the total volume of runoff contributing to the Natomas Cross Canal/Pleasant Grove Canal flooding sump at Sutter County. The estimated impact has been computed to be 196.7 acre feet for a 100-year 8-day storm event, which does not include the potential impacts of the urban reserve designated areas. The project impacts will be mitigated by the payment of the Pleasant Grove Watershed Mitigation Fee to the City of Roseville, from which funds are being used to construct volumetric mitigation at the Reason Farms project.

I.C Project Phasing

The Sierra Vista Specific Plan Area is designed to allow the backbone infrastructure to be phased. Phases may be developed in any sequence provided public safety, health and welfare issues are addressed, subject to the approval of the City of Roseville. Parcel specific improvements will be determined with Small Lot Tentative Maps or subsequent entitlements. Peak flow discharges and stormwater management requirements will be further evaluated for each phase with mitigation occurring to accommodate the need for the particular phase being developed.

Four major phased areas of the project have been identified. Section II.E discusses the mitigation and impacts of each of the major phases and identifies which mitigation improvements will be needed by each phase to be able to be constructed independent of the others, providing sufficient peak flow attenuation to reduce peak flows from the pre-project conditions, downstream of each phase.

II. Hydrology:

The Army Corps HEC-1 software was utilized to develop the included hydrologic models for the Sierra Vista Specific Plan project.

II.A Standards:

Master watershed modeling for Curry Creek has been adapted from the original “Cross Canal” models of 1992, by several projects including: Regional University Specific Plan (RUSP), Placer Vineyards, and Westpark. The initial adaptation of the model was performed with the Westpark development, which created a Kinematic Wave hydrology model only for the local areas of the Westpark development. Placer Vineyards, similarly prepared a local and tributary areas only update to the Curry Creek hydrology analysis for that project. The RUSP incorporated the Westpark and Placer Vineyards hydrology into a comprehensive Curry Creek hydrology model (Kinematic Wave) per the “*Stormwater Management Manual (SWMM)*” dated February 1994 and the SWMM Addendum 1, dated October 1997. The RUSP comprehensive model is used as a base model for this project. The existing conditions model assumes the Westpark and development areas east of Fiddymont Road as developed (per the existing conditions). The RUSP is shown as undeveloped, but this has no impact on this project as it is downstream of the Sierra Vista project, and if developed would propose to lower water surface elevations at the proposed extension of Watt Avenue. The proposed development at the Placer Vineyards project is included in the base analysis.

II.A.1 Soils:

The soil type delineation was obtained from the “Soil Survey of Placer County, California Western Part”. Hydrologic soil type “B” and “D” are the predominate soil types within the limits of the project boundary. Soils are classified into four hydrologic Categories:

Group A: Consist of soils that have a high infiltration rate when thoroughly wet. These soils have a high rate of water transmission and low runoff potential. They are deep, well drained or excessively drained, and consist chiefly of sand, gravel, or both. No soils in this project are in Group A.

Group B: Consist of soils having a moderate infiltration rate when thoroughly wet. These soils have a moderate runoff potential. They are moderately deep, well drained, and are medium in texture to moderately course in texture.

Group C: Consist of soils having a slow infiltration rate when thoroughly wet. These soils have a slow rate of water transmission and high runoff potential. They have soil layers that impede downward movement of water and have a slow infiltration rate.

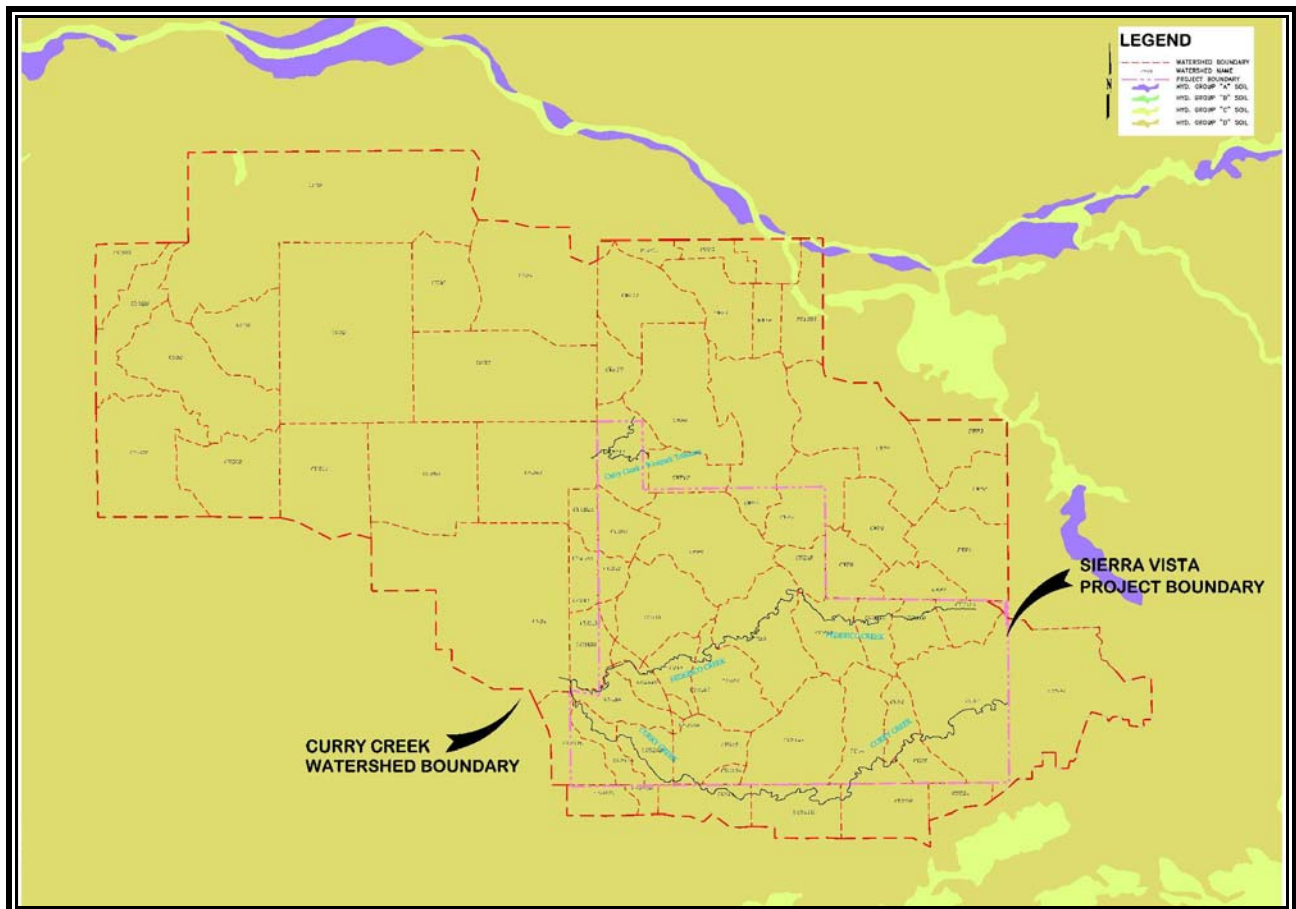
Group D: Consist of soils having a slow infiltration rate when thoroughly wet. The rate of water transmission is very slow, and runoff potential is very high. This group includes:

- a. clay soils that have high shrink-swell potential

- b. soils that have a permanent high water table
- c. soils that have a clay pan or clay layer at or near the surface and
- d. soils that are shallow over nearly impervious material

Figure II.A.1 shows the hydrologic soils group delineations as provided by the National Resources Conservation Service (NRCS – Formerly Soils Conservation Service). As seen on this exhibit, all areas of the watershed model are predominantly Soils Group “D”, which indicates low permeability of the soils and high runoff rates. The Storm Water Management Manual for the Placer County flood Control and Water Conservation District prescribes constant infiltration rates for various land uses. This study defines the following values of use: 0.07 in/hour for native ground, 0.09 in/hr for developed landscaped areas, and 0.12 in/hr for developed park areas. Impervious surfaces are not considered to have infiltration in all included studies, except for the LID related studies, were non-directly connected impervious areas, and LID treated areas were considered to be “non-impervious”.

FIGURE II.A.1 – REGIONAL HYDROLOGIC SOILS GROUPS



An oversized copy of this exhibit is provided as an attachment to this report as “SOIL-1”.

II.A.2 Land Use:

Civil Solutions was provided with a proposed land use map for the Sierra Vista Specific Plan project area as shown in Figure II.A.2 and on Exhibits SH-2 and FP-2. For the areas which are proposed for development with this project, separate watersheds were created in the post-project analysis. Factors for the Land Use were developed as shown in Tables II.A.1 and II.A.1.

FIGURE II.A.2 – SIERRA VISTA SPECIFIC PLAN PROPOSED LAND USE



II.A.3 Watershed Delineation:

The lands of the proposed project are wholly contained within the Curry Creek watershed (see Exhibit SH1-SV - Oversized). The “natural” watershed boundaries have been modified by the existing roadways and agricultural operations.

For the Pre-Project analysis, the RUSP regional model was adapted for the more detailed project topography maps. Some minor shed limit boundaries were changed and corresponding areas were re-calculated. The project watershed limits for the pre-project model are shown on exhibit SH-1.

For the Post-Project analysis, watershed boundaries were adjusted to conform to the proposed development areas. Sub-watersheds were further subdivided to account for the

proposed site alterations including: changes in impermeability due to roofs and paving, and changes in boundary locations due to the proposed piping and grading. Tributary areas to each watershed closely match the Pre-Project parameters, but minor adjustments were made for the above-mentioned purpose. Flow paths through and off the site for the Post-Project are identified on the shed map SH-2.

Urban Reserve Areas Treatment: A portion of the area shown as Urban Reserve, north of the areas with designated land use, currently drains through the proposed development areas of the Plan. These areas do not have a land use type specified in the plan other than “urban reserve”. For this analysis we have assumed only the areas which drain towards the project defined land use areas to be “Medium Density Residential – MDR” for all aspects of the analysis, and have treated these areas as if they were a part of the project with that land use. Mitigation alternatives will also consider these areas as developed. The remaining “Urban Reserve” areas which drain north and west of the project are considered in this analysis to remain un-altered in land use, and maintain their hydrologic open space equivalent factors in all aspects of this analysis.

II.A.4 Storm Centering:

The HEC-1 watershed model includes more than 1 square mile of area, and therefore, the effects of spatial distribution of a storm may have an impact on the computed peak flow rates. The Placer County Flood Control and Water Conservation District provides a software tool called “PDP”, for use in generating HEC-1 precipitation for storm centering per the SWMM specifications. In late 2008 and early 2009, the district identified issues with the storm centering procedures and generated a beta test version PDP2 of the program which makes corrections for:

- The Area Reduction Coefficient is adjusted such that a minimum value is substituted so that precipitation values for the peak 1-hour will not drop below the precipitation rate at the edge of the peak 1-hour
- The 6-hour 100-year precipitation value from the SWMM tables is altered from 2.23 inches to 2.49 inches which is a statistically more normalized value with record data
- Log-linear interpolation is substituted for log-log interpolation

The analysis included in this report utilized the PDP2 version of the district’s software.

Storm centering will be computed for CU2A3 in the pre-project events, and the CU2A8 location for the post-project events. These watershed identifiers are in the same location coordinate locations, but the post-project analysis includes substantial subdivision of watersheds, and the equivalent identifier was not carried forward to the post-project analysis. This location has been determined to control peak discharges through and downstream of the Plan area.

II.A Pre-Project Model:

II.A.1 Base Model

A Base HEC-1 data file for the base model was created using the RUSP pre-project conditions model. The Westpark development, urban areas upstream of Fiddlyment Road and the Placer Vineyards developments are shown in this model per their ultimate buildout conditions. The complete listing of the HEC-1 input file is included in Appendix A.

TABLE II.A.1 – Pre-project Base Model Hydrologic Factors

NODE	BA	PLANE 1 - NON URBAN WATERHSED FACTORS							PLANE 2 - URBAN WATERHSED FACTORS						
		Init Inf 1 (in)	Const Inf 1 (in/hr)	% Imp 1 (%)	Length 1 (ft)	Slope 1 (ft/ft)	n Val 1	% Shed 1 (%)	Init Inf 2 (in)	Const Inf 2 (in/hr)	% Imp 2 (%)	Length 2 (ft)	Slope 2 (ft/ft)	n Val 2	% Shed 2 (%)
CUS3G	0.43953	0.1	0.07	2	150	0.01	0.4	1	0.1	0.12	41	200	0.005	0.24	99
CUS3	0.37719	0.1	0.0713	2	200	0.005	0.4	96.78	0.1	0.12	90	150	0.01	0.24	3.22
CUS4	0.06109	0.1	0.0717	2	200	0.005	0.4	100							
CUS5A	0.05301	0.1	0.07	2	200	0.005	0.4	96.83	0.1	0.12	90	50	0.01	0.24	3.17
CUS5	0.05453	0.1	0.0737	2	200	0.005	0.4	90.33	0.1	0.12	90	150	0.01	0.24	9.67
CUS5B	0.14453	0.1	0.07	2	200	0.005	0.4	12.43	0.1	0.1172	60.6	50	0.01	0.24	87.57
CUS6	0.16844	0.1	0.0738	2	200	0.005	0.4	96.63	0.1	0.12	90	150	0.01	0.24	3.37
CUS11B	0.13531	0.1	0.0748	2	200	0.005	0.4	23.78	0.1	0.12	49.84	50	0.01	0.24	76.22
CUS12A	0.24688	0.1	0.071	2	200	0.005	0.4	98	0.1	0.12	90	150	0.01	0.24	2
CUS13A	0.01922	0.1	0.07	2	200	0.005	0.4	95	0.1	0.12	90	150	0.01	0.24	5
CUS14	0.09641	0.1	0.076	2	200	0.005	0.4	17.02	0.1	0.1179	48.98	50	0.01	0.24	82.98
CUS16B	0.02156	0.1	0.072	2	200	0.005	0.4	2	0.1	0.1158	80.47	50	0.01	0.24	98
CUS15	0.11427	0.1	0.072	2	300	0.01	0.4	100							
CUS15B	0.07977	0.1	0.072	2	250	0.0167	0.4	100							
CUS17A	0.06	0.1	0.07	2	200	0.005	0.4	2	0.1	0.1166	80.38	50	0.01	0.24	98
CUS17B	0.06844	0.1	0.0741	2	200	0.005	0.4	97	0.1	0.12	90	150	0.01	0.24	3
CUS17C	0.11938	0.1	0.0761	2	200	0.005	0.4	99	0.1	0.12	90	50	0.01	0.24	1
CWP7	0.07797	0.1	0.06	41	150	0.02	0.11	99	0.1	0.06	2	300	0.02	0.35	1
CU2A1A	0.06625	0.1	0.07	2	280	0.01	0.4	100							
CU2A1B	0.08063	0.1	0.07	2	250	0.01	0.4	100							
CU2A1C	0.07188	0.1	0.07	2	300	0.01	0.4	100							
CWP8	0.09922	0.1	0.07	40	150	0.02	0.11	99	0.1	0.06	2	300	0.02	0.35	1
CU2A9	0.12672	0.1	0.07	2	200	0.01	0.4	100							
CU2A8	0.06188	0.1	0.07	2	200	0.01	0.4	100							
CU2A3	0.24438	0.1	0.07	2	300	0.01	0.4	100							
CU2A7	0.16641	0.1	0.07	2	200	0.02	0.4	100							
CU2A4	0.07478	0.1	0.07	2	600	0.004	0.4	100							
CU2A4C	0.01166	0.1	0.07	2	200	0.025	0.4	100							
CU2A4B	0.04077	0.1	0.07	2	200	0.015	0.4	100							
CU2A5	0.20587	0.1	0.07	2	600	0.0067	0.4	100							
CU2A5B	0.01354	0.1	0.07	2	200	0.025	0.4	100							
CU2A6B	0.00529	0.1	0.07	2	100	0.02	0.4	100							
CU2A6	0.08665	0.1	0.07	2	300	0.027	0.4	100							
CU2A1	0.015	0.1	0.07	2	200	0.01	0.4	100							
CU2A5C	0.09156	0.1	0.07	2	200	0.01	0.4	100							
CU2A2	0.03766	0.1	0.07	2	200	0.01	0.4	100							
CU2A3A	0.05969	0.1	0.07	2	200	0.01	0.4	100							
CU2A	0.75828	0.1	0.06	2	600	0.015	0.4	100							
CWP1	0.14469	0.1	0.07	2	300	0.008	0.35	51	0.1	0.06	100	150	0.01	0.11	49
CWP2	0.19688	0.1	0.07	100	150	0.008	0.11	27	0.1	0.06	2	300	0.01	0.35	73
CWP4	0.3627	0.1	0.07	2	300	0.01	0.35	54	0.1	0.06	100	200	0.02	0.11	46
CWP3	0.3	0.1	0.07	2	300	0.01	0.35	54	0.1	0.06	100	200	0.01	0.11	46
CWP6	0.3039	0.1	0.09	2	300	0.01	0.35	72	0.1	0.06	100	200	0.008	0.11	28
CWP9	0.29926	0.1	0.07	2	300	0.008	0.4	92	0.1	0.07	50	400	0.005	0.24	8
CWP13	0.09422	0.1	0.07	2	300	0.008	0.35	67	0.1	0.07	100	150	0.008	0.11	33
CWP14	0.1525	0.1	0.06	2	300	0.008	0.35	98	0.1	0.06	100	150	0.01	0.11	2
CU2B1	0.115	0.1	0.06	2	200	0.005	0.4	100							
CU2B1A	0.04563	0.1	0.06	2	200	0.005	0.4	100							
CU2B2	0.33563	0.1	0.06	2	600	0.005	0.4	100							
CU2B3	0.3455	0.1	0.06	2	600	0.005	0.4	100							
CU2C1	0.2913	0.1	0.06	2	600	0.005	0.4	100							
CU2C2	0.229	0.1	0.06	2	100	0.005	0.4	100							
CU2C3	0.33844	0.1	0.06	2	600	0.005	0.4	100							
CWP11A	0.075	0.1	0.06	2	300	0.01	0.35	60	0.1	0.06	100	150	0.02	0.11	40
CWP11	0.123	0.1	0.06	2	300	0.01	0.35	95	0.1	0.06	100	200	0.01	0.11	5
CWP12	0.0678	0.1	0.06	100	600	0.01	0.11	2	0.1	0.06	2	600	0.01	0.4	98
CU3A	0.3864	0.1	0.06	2	600	0.005	0.4	100							
CU3B	0.4803	0.1	0.06	2	600	0.001	0.4	100							
CU3C	0.1723	0.1	0.06	2	600	0.001	0.4	100							
CU3D	0.7278	0.1	0.06	2	600	0.001	0.4	100							
CU3E	0.1747	0.1	0.06	2	600	0.001	0.4	100							
CU3F	1.0026	0.1	0.06	2	600	0.001	0.4	100							
CU3G	0.3125	0.1	0.06	2	600	0.001	0.4	100							
CU3HA	0.08969	0.1	0.06	2	600	0.005	0.4	100							
CU3HB	0.1658	0.1	0.06	2	300	0.001	0.4	100							
CU4	3.58	0.1	0.06	2	600	0.015	0.4	100							
CU5	0.28	0.1	0.06	2	600	0.015	0.4	100							
CU6	0.87	0.1	0.06	2	600	0.015	0.4	100							

II.B Post-Project “Future, Fully Developed, Unmitigated” Model:

Civil Solutions prepared a Post Project model based on the proposed project land use information and the soils delineation boundaries. This study is based on the updated plan information shown on Exhibits SH-2. Factors for the developing watershed areas were added as shown in TABLE II.B.1 in the Zone 2 watershed (secondary plane). Directly connected imperviousness was computed based on average coverage rates assumed for the various contributing area types as follows:

<u>Land Use Type:</u>	<u>% Impervious</u>
Low Density Residential (LDR)	40%
Medium Density Residential (MDR)	50%
High Density Residential (HDR)	60%
Commercial (Comm)	70%
Park	5%
Public/Quasi Public	50%
Roadway	85%

Overland flow factors are also adjusted in the secondary (development) plane per typical values for the various development types.

Files for the post-project hydrology(HEC-1) models are included on the CD-ROM with this report, and a complete listing of this file is provided in Appendix C. This model is the “unmitigated” analysis for this project which means that no mitigation detention is included. This model is also adapted for the “Future, Fully Developed, Unmitigated” scenario, which means that all existing offsite detention has been removed, that all offsite areas are assumed to be fully developed, and that project detention is also removed.

This model is used as a base of comparison to the pre-project model for determining peak flow mitigation requirements per Chapter VII of the SWMM. This model is also used for the hydrology which is fed into the “Future Fully Developed Unmitigated” hydraulics analysis which forms the basis of the flow rates used in the final flood plain mapping where freeboard requirements are to be tested, and culvert and bridge sizing evaluated.

TABLE II.B.1A – Post-Project Un-mitigated Hydrologic Factors

NODE	BA	PLANE 1 WATERSHED FACTORS (non-urban plane)						PLANE 2 WATERSHED FACTORS (Urban Plane)							
		Init Inf 1	Const Inf 1	% Imp 1	Length 1	Slope 1	n Val 1	% Shed 1	Init Inf 2	Const Inf 2	% Imp 2	Length 2	Slope 2	n Val 2	% Shed 2
CUS3G	0.4396	0.1	0.12	41.369	200	0.005	0.24	100							
SS3	0.0707	0.1	0.07	4.982	150	0.01	0.4	5	0.1	0.12	56.659	96	0.0108	0.24	95
SS3B	0.0252	0.1	0.07	2.089	300	0.01	0.4	94	0.1	0.12	84.855	89	0.0122	0.24	6
SS3C	0.0981	0.1	0.07	4.997	150	0.01	0.4	7	0.1	0.12	42.681	130	0.01	0.24	93
SS3D	0.1037	0.1	0.07	4.993	150	0.01	0.4	1	0.1	0.12	65.734	85	0.013	0.24	99
SS3E	0.0236	0.1	0.07	2.084	300	0.01	0.4	100							
SS5	0.0316	0.1	0.12	70	81	0.0138	0.24	100							
SS3F	0.1151	0.1	0.07	4.999	150	0.01	0.4	6	0.1	0.12	47.061	115	0.01	0.24	94
CUS5A	0.053	0.1	0.07	2	200	0.005	0.4	100							
SS4A	0.0381	0.1	0.07	2.254	300	0.01	0.4	100							
SS5C	0.0281	0.1	0.12	71.093	79	0.0142	0.24	100							
SS4	0.1281	0.1	0.07	5	150	0.01	0.4	5	0.1	0.12	57.303	95	0.011	0.24	95
SS5D	0.0097	0.1	0.07	5	150	0.01	0.4	8	0.1	0.12	71.792	84	0.0132	0.24	92
CUS6B	0.0103	0.1	0.07	2.971	150	0.01	0.4	29	0.1	0.12	71.974	400	0.005	0.24	71
SS6A	0.0058	0.1	0.12	40	130	0.01	0.24	100							
SS6	0.0122	0.1	0.07	2.425	150	0.01	0.4	88	0.1	0.12	85	115	0.01	0.24	12
CUS5B	0.147	0.1	0.07	2	200	0.005	0.4	100							
CUS11B	0.1374	0.1	0.0748	2	200	0.005	0.4	100							
CUS14	0.097	0.1	0.076	2	200	0.005	0.4	100							
CUS16B	0.0208	0.1	0.072	2	200	0.005	0.4	100							
SS12A	0.0283	0.1	0.12	70.939	79	0.0142	0.24	100							
SS13A	0.2204	0.1	0.07	5	150	0.01	0.4	6	0.1	0.12	55.922	99	0.0102	0.24	94
SS15	0.0104	0.1	0.07	2.585	300	0.01	0.4	69	0.1	0.12	84.484	89	0.0122	0.24	31
CUS17A	0.0597	0.1	0.07	2	200	0.005	0.4	100							
SS15B	0.0089	0.1	0.07	2.724	150	0.01	0.4	1	0.1	0.12	72.173	78	0.0144	0.24	99
SS15A	0.1168	0.1	0.07	4.996	150	0.01	0.4	13	0.1	0.12	57.177	94	0.0112	0.24	87
SS15F	0.0297	0.1	0.07	2.125	150	0.01	0.4	96	0.1	0.12	80.401	121	0.01	0.24	4
SS15C	0.0066	0.1	0.07	4.978	150	0.01	0.4	100							
SS15D	0.0108	0.1	0.07	2.742	150	0.01	0.4	1	0.1	0.12	40	130	0.01	0.24	99
SS15E	0.0306	0.1	0.07	2	150	0.01	0.4	1	0.1	0.12	70.638	79	0.0142	0.24	99
SS17F	0.0281	0.1	0.07	2.016	150	0.01	0.4	100							
SS17B	0.0529	0.1	0.07	4.998	150	0.01	0.4	95	0.1	0.12	84.405	290	0.0074	0.24	5
SS17D	0.024	0.1	0.07	4.501	150	0.01	0.4	7	0.1	0.12	46.994	115	0.01	0.24	93
SS17E	0.0119	0.1	0.07	4.967	150	0.01	0.4	100							
CUS17C	0.0629	0.1	0.07	2.004	150	0.01	0.4	100							
CUS2A1A	0.0765	0.1	0.07	3.162	150	0.01	0.4	28	0.1	0.12	46.652	400	0.005	0.24	72
S2A1B	0.0203	0.1	0.07	4.953	150	0.01	0.4	62	0.1	0.12	49.962	178	0.0096	0.24	38
CWP7	0.078	0.1	0.06	41.023	300	0.02	0.35	100							
S2A1C	0.0313	0.1	0.07	4.811	150	0.01	0.4	6	0.1	0.12	45.362	127	0.01	0.24	94
S2A1D	0.0617	0.1	0.07	2.005	150	0.01	0.4	100							
S2A1E	0.0057	0.1	0.07	5	150	0.01	0.4	8	0.1	0.12	40	136	0.01	0.24	92
S2A1F	0.0062	0.1	0.07	2.011	150	0.01	0.4	99	0.1	0.12	40	160	0.01	0.24	1
S2AB2	0.0083	0.1	0.07	4.895	150	0.01	0.4	13	0.1	0.12	43.348	132	0.01	0.24	87
S2AB3	0.0116	0.1	0.07	4.418	150	0.01	0.4	2	0.1	0.12	40.056	132	0.01	0.24	98
S2AB4	0.0114	0.1	0.07	2.053	150	0.01	0.4	100							

TABLE II.B.1B – Post-Project Un-mitigated Hydrologic Factors (Continued)

NODE	BA	PLANE 1 WATERSHED FACTORS (non-urban plane)							PLANE 2 WATERSHED FACTORS (Urban Plane)						
		Init Inf 1	Const Inf 1	% Imp 1	Length 1	Slope 1	n Val 1	% Shed 1	Init Inf 2	Const Inf 2	% Imp 2	Length 2	Slope 2	n Val 2	% Shed 2
CWP8	0.0992	0.1	0.06	40	300	0.02	0.35	100							
S2AC2	0.0052	0.1	0.07	2,132	150	0.01	0.4	100							
S2AC1	0.0029	0.1	0.07	3,286	150	0.01	0.4	3	0.1	0.12	40.016	130	0.01	0.24	97
CWP8A	0.0274	0.1	0.12	50	150	0.005	0.24	100							
S2AC3	0.0259	0.1	0.07	4,955	150	0.01	0.4	19	0.1	0.12	47.34	132	0.01	0.24	81
S2AC6	0.035	0.1	0.07	2,025	150	0.01	0.4	97	0.1	0.12	82.339	77	0.0146	0.24	3
S2AC4	0.0157	0.1	0.12	40	130	0.01	0.24	100							
S2A9	0.0573	0.1	0.07	4,723	150	0.01	0.4	6	0.1	0.12	55.939	98	0.0104	0.24	94
S2AB1	0.0523	0.1	0.07	4.44	150	0.01	0.4	29	0.1	0.12	57.7	127	0.01	0.24	71
S2A1G	0.007	0.1	0.07	2,201	600	0.015	0.4	88	0.1	0.12	82.72	68	0.0173	0.24	12
S2AC5	0.0571	0.1	0.07	2,823	150	0.01	0.4	40	0.1	0.12	46.029	121	0.01	0.24	60
S2AC7	0.0087	0.1	0.12	42,931	124	0.01	0.24	100							
S2A8	0.1294	0.1	0.07	4,991	150	0.01	0.4	6	0.1	0.12	46.529	118	0.01	0.24	94
S2A7A	0.0583	0.1	0.07	4,911	150	0.01	0.4	14	0.1	0.12	52.734	112	0.01	0.24	86
S2A7	0.0276	0.1	0.07	2,039	150	0.01	0.4	94	0.1	0.12	83.349	97	0.0106	0.24	6
S2A7B	0.0224	0.1	0.07	4,987	150	0.01	0.4	36	0.1	0.12	40	154	0.01	0.24	64
S2A7C	0.0812	0.1	0.07	5	150	0.01	0.4	5	0.1	0.12	45.12	132	0.01	0.24	95
S2A4A	0.0648	0.1	0.07	4,911	150	0.01	0.4	8	0.1	0.12	56.44	98	0.0104	0.24	92
S2A4	0.0105	0.1	0.07	2,124	150	0.01	0.4	83	0.1	0.12	84.481	79	0.0142	0.24	17
S2A5	0.17	0.1	0.07	4,944	150	0.01	0.4	7	0.1	0.12	49.625	109	0.01	0.24	93
S2A6A	0.0468	0.1	0.07	4,828	150	0.01	0.4	4	0.1	0.12	40.041	132	0.01	0.24	96
S2A6B	0.0161	0.1	0.07	4,622	150	0.01	0.4	6	0.1	0.12	40.014	134	0.01	0.24	94
S2A6	0.0578	0.1	0.07	2,031	150	0.01	0.4	100							
S2A6C	0.0594	0.1	0.07	4,981	150	0.01	0.4	9	0.1	0.12	47.276	118	0.01	0.24	91
S2A2	0.0275	0.1	0.07	2	200	0.01	0.4	100							
S2A1	0.0131	0.1	0.07	2	200	0.01	0.4	100							
S2A5C	0.1082	0.1	0.07	2.03	200	0.01	0.4	100							
CU2A3A	0.0597	0.1	0.07	2	200	0.01	0.4	100							
CU2A	0.7583	0.1	0.06	2	600	0.015	0.4	100							
CWP1	0.1447	0.1	0.06	2	300	0.008	0.35	51	0.1	0.06	100	150	0.01	0.11	49
CWP2	0.1969	0.1	0.07	100	150	0.008	0.11	27	0.1	0.06	2	300	0.01	0.35	73
CWP4	0.3627	0.1	0.06	2	300	0.01	0.35	54	0.1	0.06	100	200	0.02	0.11	46
CWP3	0.3	0.1	0.07	2	300	0.01	0.35	54	0.1	0.06	100	200	0.01	0.11	46
CWP6	0.3039	0.1	0.09	2	300	0.01	0.35	72	0.1	0.06	100	200	0.008	0.11	28
SP9A	0.0305	0.1	0.07	4,948	150	0.01	0.4	2	0.1	0.12	51.361	100	0.01	0.24	98
CWP9	0.2329	0.1	0.07	2	300	0.008	0.4	98	0.1	0.07	84.266	400	0.005	0.24	2
CWP13	0.0942	0.1	0.07	2	300	0.008	0.35	67	0.1	0.07	100	150	0.008	0.11	33
CWP14	0.1525	0.1	0.06	2	300	0.008	0.35	98	0.1	0.06	100	150	0.01	0.11	2
CU2B1	0.115	0.1	0.06	2	200	0.005	0.4	100							
CU2B1A	0.0456	0.1	0.06	2	200	0.005	0.4	100							
CU2B2	0.3356	0.1	0.06	2	600	0.005	0.4	100							
CU2B3	0.3455	0.1	0.06	2	600	0.005	0.4	100							
CU2C1	0.2913	0.1	0.06	2	600	0.005	0.4	100							
CU2C2	0.229	0.1	0.06	2	100	0.005	0.4	100							
CU2C3	0.3384	0.1	0.06	2	600	0.005	0.4	100							
CWP11A	0.075	0.1	0.06	2	300	0.01	0.35	60	0.1	0.06	100	150	0.02	0.11	40
CWP11	0.123	0.1	0.06	2	300	0.01	0.35	95	0.1	0.06	100	200	0.01	0.11	5
CWP12	0.0678	0.1	0.06	100	600	0.01	0.11	2	0.1	0.06	2	600	0.01	0.4	98
CU3A	0.3864	0.1	0.06	2	600	0.005	0.4	100							
CU3B	0.4803	0.1	0.06	2	600	0.001	0.4	100							
CU3C	0.1723	0.1	0.06	2	600	0.001	0.4	100							
CU3D	0.7278	0.1	0.06	2	600	0.001	0.4	100							
CU3E	0.1747	0.1	0.06	2	600	0.001	0.4	100							
CU3F	1.0026	0.1	0.06	2	600	0.001	0.4	100							
CU3G	0.3125	0.1	0.06	2	600	0.001	0.4	100							
CU3HA	0.0897	0.1	0.06	2	600	0.005	0.4	100							
CU3HB	0.1658	0.1	0.06	2	300	0.001	0.4	100							
CU4	3.58	0.1	0.06	2	600	0.015	0.4	100							
CU5	0.28	0.1	0.06	2	600	0.015	0.4	100							
CU6	0.87	0.1	0.06	2	600	0.015	0.4	100							

II.C Post-Project Mitigated Model:

Hydrologic (HEC-1) watershed factors for the two post project conditions are identical. This mitigation model also uses the Unsteady State HEC-RAS hydraulic analysis for the evaluation of the routing benefits of the proposed project improvements within the open space which will provide attenuation offsets to mitigate the development impacts.

Additionally, this model includes the evaluation of the need for a potential temporary detention basin in the north area of the project where a ~20 acre MDR site will drain north to the Urban Reserve areas. The analysis evaluated the total shed changes that occurred in cleaning up the boundaries for land use areas and the urban reserve boundary at the north end of the project, and found that approximately 12 acres total will be diverted from flowing north, and put into the Federico Creek system. As a result of the diversion of the runoff from the 12 acres to the Federico Tributary, the model demonstrates that peak flows to the north are reduced, even in the alternative where the 20 acre MDR site is developed without detention. Therefore no detention is proposed at this location. Attenuation within the Federico Tributary accounts for the flow diversion acreage mitigation needs.

Two detention basins constructed in series will be located within the power line corridor adjacent to Federico Creek in the north central area of the project, providing approximately 20 acre feet of short term (less than 72 hour drawdown) detention storage. Flows will be diverted to the detention areas via a 42 inch diameter culvert to be located at the Market Street crossing of Federico Creek. The detention basins will also receive flows from the local areas which are directly tributary to their locations. The two detention basins will be interconnected by a 30 inch diameter pipe at the future roadway crossing. And the downstream basin will have a 30 inch diameter exit pipe crossing the proposed roadway, and exiting into the created wetland areas of the open space.

The proposed remaining attenuation areas will be located in the overbank areas of the existing Curry Creek streams, providing additional conveyance and storage capacity within the stream system. The added storage will function passively on the system, requiring no special operational maintenance. Additionally, the design of the floodplain areas, and attenuation features accounts for the type of vegetation which may develop in these areas as a result of added nuisance waters and nutrients known to occur with types of development planned in this project. Exhibit AT-1 shows the proposed areas where improvements are proposed within the stream corridors. A reduced version of Exhibit AT-1 is provided here in Figure II.C.

Figures II.C and AT-1 also identify areas shown as “Alternate Outfall Locations”. These locations have been identified as potential locations where additional storm drain outfalls and vegetated swales will be needed. If they are used in final design, a portion or all of the storm drain flows from an adjacent outfall will be relocated to this facility.

Preliminary sizing shown on the exhibit assumed complete redundancy, and areas identified are for a complete relocation of flow discharges from an adjacent location. However, the grading for only the main outfall locations identified was included in the

mitigation hydrology and hydraulics analysis.

FIGURE II.C – LOCATIONS OF IMPROVEMENTS PROPOSED IN STREAM CORRIDORS



An oversized plot of this exhibit is provided in the back of this report, as Exhibit AT-1. Proposed project facilities and wetland resources within the open space are also more easily identified on the larger exhibit.

The post project models for the Unmitigated and Mitigated conditions do not include adjustments for the proposed use of Low Impact Development measures planned by this project. Those adjustments are only included in the Hydromodification Evaluation Hydrology Model (labeled some places as the “with LID” model).

II.D Hydromodification Evaluation Post-Project Hydrology Model:

For the purposes of testing the potential Hydrograph Modification Benefits of the proposed LID program of this project, an alternate hydrology (HEC-1) analysis was performed which adjusts the imperviousness of the project for the LID components which will contribute to retention and infiltration of stormwater, which reduces runoff. This model includes the impervious area modifications for LID improvements by land use per TABLE II.D.1. Impervious area reductions are based on the Required Volume Reductions (RVR's) specified in section IV of this report. Timing of runoff passing through LID measures was not adjusted in this analysis compared to the other post-project hydrology models. Meaning that while a portion of area was converted to non-impervious type, it was retained in the urban plane of the watershed, and it is assumed that runoff coming from those areas will runoff in an urban timing. While changes in impervious area on type D soils are not a significant factor during the peak of flood events such as the 100-year, we have included these modifications so that impacts in smaller events such as SWQ and Hydromod events can be tested. Watershed HEC-1 factors are listed in Tables II.D.2A and II.D.2B.

TABLE II.D.1 – IMPERVIOUS AREA REDUCTIONS FOR LID

Land Use Type	Non-modified Average Impervious %	Modified for LID Average Impervious %	Impervious Reduction for LID RVR's
LDR	40%	9%	31%
MDR	50%	12%	38%
HDR	60%	19%	41%
Commercial	70%	20%	50%
Public/Quasi Public	40%	9%	31%
Park	5%	2%	3%
Road	85%	26%	59%

TABLE II.D.2A – HYDROLOGIC WATERSHED FACTORS – LID REDUCED

NODE	BA	PLANE 1 WATERSHED FACTORS (non-urban plane)							PLANE 2 WATERSHED FACTORS (Urban Plane)						
		Init Inf 1	Const Inf 1	% Imp 1	Length 1	Slope 1	n Val 1	% Shed 1	Init Inf 2	Const Inf 2	% Imp 2	Length 2	Slope 2	n Val 2	% Shed 2
CUS3G	0.4396	0.1	0.12	9.502	200	0.005	0.24	100							
SS3	0.0707	0.1	0.07	4.982	150	0.01	0.4	5	0.1	0.12	15.178	96	0.0108	0.24	95
SS3B	0.0252	0.1	0.07	2.089	300	0.01	0.4	94	0.1	0.12	25.945	89	0.0122	0.24	6
SS3C	0.0981	0.1	0.07	4.997	150	0.01	0.4	7	0.1	0.12	10.013	130	0.01	0.24	93
SS3D	0.1037	0.1	0.07	4.993	150	0.01	0.4	1	0.1	0.12	18.294	85	0.013	0.24	99
SS3E	0.0236	0.1	0.07	2.084	300	0.01	0.4	100							
SS5	0.0316	0.1	0.12	20	81	0.0138	0.24	100							
SS3F	0.1151	0.1	0.07	4.999	150	0.01	0.4	6	0.1	0.12	11.297	115	0.01	0.24	94
CUS5A	0.053	0.1	0.07	2	200	0.005	0.4	100							
SS4A	0.0381	0.1	0.07	2.254	300	0.01	0.4	100							
SS5C	0.0281	0.1	0.12	20.437	79	0.0142	0.24	100							
SS4	0.1281	0.1	0.07	5	150	0.01	0.4	5	0.1	0.12	16.119	95	0.011	0.24	95
SS5D	0.0097	0.1	0.07	5	150	0.01	0.4	8	0.1	0.12	20.717	84	0.0132	0.24	92
CUS6B	0.0103	0.1	0.07	2.971	150	0.01	0.4	29	0.1	0.12	20.79	400	0.005	0.24	71
SS6A	0.0058	0.1	0.12	9	130	0.01	0.24	100							
SS6	0.0122	0.1	0.07	2.425	150	0.01	0.4	88	0.1	0.12	26	115	0.01	0.24	12
CUS5B	0.147	0.1	0.07	2	200	0.005	0.4	100							
CUS11B	0.1374	0.1	0.0748	2	200	0.005	0.4	100							
CUS14	0.097	0.1	0.076	2	200	0.005	0.4	100							
CUS16B	0.0208	0.1	0.072	2	200	0.005	0.4	100							
SS12A	0.0283	0.1	0.12	20.376	79	0.0142	0.24	100							
SS13A	0.2204	0.1	0.07	5	150	0.01	0.4	6	0.1	0.12	15.093	99	0.0102	0.24	94
SS15	0.0104	0.1	0.07	2.585	300	0.01	0.4	69	0.1	0.12	25.794	89	0.0122	0.24	31
CUS17A	0.0597	0.1	0.07	2	200	0.005	0.4	100							
SS15B	0.0089	0.1	0.07	2.724	150	0.01	0.4	1	0.1	0.12	20.869	78	0.0144	0.24	99
SS15A	0.1168	0.1	0.07	4.996	150	0.01	0.4	13	0.1	0.12	15.384	94	0.0112	0.24	87
SS15F	0.0297	0.1	0.07	2.125	150	0.01	0.4	96	0.1	0.12	24.262	121	0.01	0.24	4
SS15C	0.0066	0.1	0.07	4.978	150	0.01	0.4	100							
SS15D	0.0108	0.1	0.07	2.742	150	0.01	0.4	1	0.1	0.12	9	130	0.01	0.24	99
SS15E	0.0306	0.1	0.07	2	150	0.01	0.4	1	0.1	0.12	20.255	79	0.0142	0.24	99
SS17F	0.0281	0.1	0.07	2.016	150	0.01	0.4	100							
SS17B	0.0529	0.1	0.07	4.998	150	0.01	0.4	95	0.1	0.12	25.762	290	0.0074	0.24	5
SS17D	0.024	0.1	0.07	4.501	150	0.01	0.4	7	0.1	0.12	11.642	115	0.01	0.24	93
SS17E	0.0119	0.1	0.07	4.967	150	0.01	0.4	100							
CUS17C	0.0629	0.1	0.07	2.004	150	0.01	0.4	100							
CU2A1A	0.0765	0.1	0.07	3.162	150	0.01	0.4	28	0.1	0.12	11.902	400	0.005	0.24	72
S2A1B	0.0203	0.1	0.07	4.953	150	0.01	0.4	62	0.1	0.12	11.989	178	0.0096	0.24	38
CWP7	0.078	0.1	0.06	9.307	300	0.02	0.35	100	0	0	0	0	0	0	0
S2A1C	0.0313	0.1	0.07	4.811	150	0.01	0.4	6	0.1	0.12	11.026	127	0.01	0.24	94
S2A1D	0.0617	0.1	0.07	2.005	150	0.01	0.4	100							
S2A1E	0.0057	0.1	0.07	5	150	0.01	0.4	8	0.1	0.12	9	136	0.01	0.24	92
S2A1F	0.0062	0.1	0.07	2.011	150	0.01	0.4	99	0.1	0.12	9	160	0.01	0.24	1
S2AB2	0.0083	0.1	0.07	4.895	150	0.01	0.4	13	0.1	0.12	10.265	132	0.01	0.24	87
S2AB3	0.0116	0.1	0.07	4.418	150	0.01	0.4	2	0.1	0.12	9.021	132	0.01	0.24	98
S2AB4	0.0114	0.1	0.07	2.053	150	0.01	0.4	100							

TABLE II.D.2B – HYDROLOGIC WATERSHED FACTORS – LID REDUCED (CONT)

NODE	BA	PLANE 1 WATERSHED FACTORS (non-urban plane)							PLANE 2 WATERSHED FACTORS (Urban Plane)						
		Init Inf 1	Const Inf 1	% Imp 1	Length 1	Slope 1	n Val 1	% Shed 1	Init Inf 2	Const Inf 2	% Imp 2	Length 2	Slope 2	n Val 2	% Shed 2
CWP8	0.0992	0.1	0.06	9	300	0.02	0.35	100							
S2AC2	0.0052	0.1	0.07	2.132	150	0.01	0.4	100							
S2AC1	0.0029	0.1	0.07	3.286	150	0.01	0.4	3	0.1	0.12	9.005	130	0.01	0.24	97
CWP8A	0.0274	0.1	0.12	12	150	0.005	0.24	100							
S2AC3	0.0259	0.1	0.07	4.955	150	0.01	0.4	19	0.1	0.12	11.629	132	0.01	0.24	81
S2AC6	0.035	0.1	0.07	2.025	150	0.01	0.4	97	0.1	0.12	24.995	77	0.0146	0.24	3
S2AC4	0.0157	0.1	0.12	9	130	0.01	0.24	100							
S2A9	0.0573	0.1	0.07	4.723	150	0.01	0.4	6	0.1	0.12	15.343	98	0.0104	0.24	94
S2AB1	0.0523	0.1	0.07	4.44	150	0.01	0.4	29	0.1	0.12	16.098	127	0.01	0.24	71
S2A1G	0.007	0.1	0.07	2.201	600	0.015	0.4	88	0.1	0.12	25.139	160	0.01	0.24	12
S2AC5	0.0571	0.1	0.07	2.823	150	0.01	0.4	40	0.1	0.12	11.278	121	0.01	0.24	60
S2AC7	0.0087	0.1	0.12	10.107	250	0.008	0.24	100							
S2A8	0.1294	0.1	0.07	4.991	150	0.01	0.4	6	0.1	0.12	11.467	118	0.01	0.24	94
S2A7A	0.0583	0.1	0.07	4.911	150	0.01	0.4	14	0.1	0.12	13.094	112	0.01	0.24	86
S2A7	0.0276	0.1	0.07	2.039	150	0.01	0.4	94	0.1	0.12	25.376	97	0.0106	0.24	6
S2A7B	0.0224	0.1	0.07	4.987	150	0.01	0.4	36	0.1	0.12	9	154	0.01	0.24	64
S2A7C	0.0812	0.1	0.07	5	150	0.01	0.4	5	0.1	0.12	10.867	132	0.01	0.24	95
S2A4A	0.0648	0.1	0.07	4.911	150	0.01	0.4	8	0.1	0.12	14.576	98	0.0104	0.24	92
S2A4	0.0105	0.1	0.07	2.124	150	0.01	0.4	83	0.1	0.12	25.802	79	0.0142	0.24	17
S2A5	0.17	0.1	0.07	4.944	150	0.01	0.4	7	0.1	0.12	12.39	109	0.01	0.24	93
S2A6A	0.0468	0.1	0.07	4.828	150	0.01	0.4	4	0.1	0.12	9.012	132	0.01	0.24	96
S2A6B	0.0161	0.1	0.07	4.622	150	0.01	0.4	6	0.1	0.12	9.005	134	0.01	0.24	94
S2A6	0.0578	0.1	0.07	2.031	150	0.01	0.4	100							
S2A6C	0.0594	0.1	0.07	4.981	150	0.01	0.4	9	0.1	0.12	11.475	118	0.01	0.24	91
S2A2	0.0275	0.1	0.07	2	200	0.01	0.4	100							
S2A1	0.0131	0.1	0.07	2	200	0.01	0.4	100							
S2A5C	0.1082	0.1	0.07	2.03	200	0.01	0.4	100							
CU2A3A	0.0597	0.1	0.07	2	200	0.01	0.4	100							
CU2A	0.7583	0.1	0.06	2	600	0.015	0.4	100							
CWP1	0.1447	0.1	0.06	2	300	0.008	0.35	51	0.1	0.06	100	150	0.01	0.11	49
CWP2	0.1969	0.1	0.07	100	150	0.008	0.11	27	0.1	0.06	2	300	0.01	0.35	73
CWP4	0.3627	0.1	0.06	2	300	0.01	0.35	54	0.1	0.06	100	200	0.02	0.11	46
CWP3	0.3	0.1	0.07	2	300	0.01	0.35	54	0.1	0.06	100	200	0.01	0.11	46
CWP6	0.3039	0.1	0.09	2	300	0.01	0.35	72	0.1	0.06	100	200	0.008	0.11	28
SP9A	0.0305	0.1	0.07	4.948	150	0.01	0.4	2	0.1	0.12	12.544	100	0.01	0.24	98
CWP9	0.2329	0.1	0.07	2	300	0.008	0.4	98	0.1	0.07	25.706	400	0.005	0.24	2
CWP13	0.0942	0.1	0.07	2	300	0.008	0.35	67	0.1	0.07	100	150	0.008	0.11	33
CWP14	0.1525	0.1	0.06	2	300	0.008	0.35	98	0.1	0.06	100	150	0.01	0.11	2
CU2B1	0.115	0.1	0.06	2	200	0.005	0.4	100							
CU2B1A	0.0456	0.1	0.06	2	200	0.005	0.4	100							
CU2B2	0.3356	0.1	0.06	2	600	0.005	0.4	100							
CU2B3	0.3455	0.1	0.06	2	600	0.005	0.4	100							
CU2C1	0.2913	0.1	0.06	2	600	0.005	0.4	100							
CU2C2	0.229	0.1	0.06	2	100	0.005	0.4	100							
CU2C3	0.3384	0.1	0.06	2	600	0.005	0.4	100							
CWP11A	0.075	0.1	0.06	2	300	0.01	0.35	60	0.1	0.06	100	150	0.02	0.11	40
CWP11	0.123	0.1	0.06	2	300	0.01	0.35	95	0.1	0.06	100	200	0.01	0.11	5
CWP12	0.0678	0.1	0.06	100	600	0.01	0.11	2	0.1	0.06	2	600	0.01	0.4	98
CU3A	0.3864	0.1	0.06	2	600	0.005	0.4	100							
CU3B	0.4803	0.1	0.06	2	600	0.001	0.4	100							
CU3C	0.1723	0.1	0.06	2	600	0.001	0.4	100							
CU3D	0.7278	0.1	0.06	2	600	0.001	0.4	100							
CU3E	0.1747	0.1	0.06	2	600	0.001	0.4	100							
CU3F	1.0026	0.1	0.06	2	600	0.001	0.4	100							
CU3G	0.3125	0.1	0.06	2	600	0.001	0.4	100							
CU3HA	0.0897	0.1	0.06	2	600	0.005	0.4	100							
CU3HB	0.1658	0.1	0.06	2	300	0.001	0.4	100							
CU4	3.58	0.1	0.06	2	600	0.015	0.4	100							
CU5	0.28	0.1	0.06	2	600	0.015	0.4	100							
CU6	0.87	0.1	0.06	2	600	0.015	0.4	100							

II.E Summary of Findings:**TABLE II.E.3A – 100-YEAR (& 10-YEAR) PEAK FLOW COMPARISON**

Stream Station	Description	Pre-Project Peak Flow (cfs)	Post-Project Future, Fully Developed, Unmitigated Peak Flow (cfs)	Post-Project Mitigated Peak Flow (cfs)	Post-Project Mitigated Net Peak Flow Reduction (cfs)
Curry Creek – Main Channel					
1.44	Fiddymment Road at Upstream end of Analysis	285 (146)	285 (146)	285 (145)	0 (1) (via diversion)
1.385	Upland Drive Crossing	331 (170)	272 (153) (note 2)	255 (153)	76 (17)
1.333	Pedestrian Crossing #1	477 (253)	326 (184) (note 2)	334 (186)	143 (67)
1.265	Market Street Crossing	406 (202)	364 (226) (note 2)	349 (219)	57 (-17) (onsite)
1.215	Baseline Road Eastern Upstream Crossing	458 (226)	380 (237) (note 2)	366 (230)	88 (-4) (see note 1)
1.025	Baseline Road Western Downstream Crossing	455 (210)	397 (263) (note 2)	414 (272)	39 (-62) (onsite)
0.87	Watt Avenue Crossing	431 (214)	414 (274) (note 2)	434 (280)	-3 (-5) (onsite)
0.552	Pedestrian Crossing #2	428 (215)	424 (279) (note 2)	454 (282)	-26 (-67) (onsite)
0.33	Pedestrian Crossing #3 At Existing Driveway Br.	424 (216)	425 (277) (note 2)	455 (277)	-31 (-61) (onsite)
0.03	Downstream end of Curry Creek at Confl. with Federico Creek	432 (220)	429 (278) (note 2)	461 (273)	-29 (-53) (onsite)
Federico Creek					
1.15	Pedestrian Crossing #4	116 (61)	182 (99)	121 (64)	-5 (-3)
0.9954	East-West Road	185 (95)	168 (105) (note 2)	130 (76)	55 (19)
0.975	Market Street	194 (100)	165 (105) (note 2)	128 (76)	66 (24)
0.931	Pedestrian Crossing #5	228 (121)	172 (105)	104 (59)	124 (62) (onsite)
0.865	Westside Drive	319 (176)	223 (142) (note 2)	157 (71)	162 (105)
0.805	North-South Road	366 (202)	312 (213) (note 2)	240 (132)	126 (70)
0.775	Watt Avenue	376 (207)	320 (230) (note 2)	252 (150)	124 (57)
CURRY CREEK DOWNSTREAM OF CONFLUENCE					
6.9	Downstream Project Boundary	804 (466)	799 (488)	740 (426)	64 (40) (1 cfs reduction required by SWMM in 100-year)

Note 1: Peak flow increases occur in the 10-year thru the reach south of Baseline Road. 100-year generates substantial flow reductions through this reach.

Note 2: An apparent flow reduction in the post-project un-mitigated results from the need for the Future Fully Developed Unmitigated alternative to include all bridges and worst case 'n' values for the purpose of determining the maximum floodplain elevations for design. Inherently this increases the amount of attenuation represented in this model.

Table II.E.3A compares the findings of the various hydrology analysis studies for the Peak 100-year flow rates at several key locations within the project. Flowrates reported in this table result from the hydraulic routing analysis, and are reported as the “Max” flows in the HEC-RAS files for these locations.

Table II.E.3A identifies that for all project exit points and downstream of the project, that peak flow rates will be reduced for the 100-year event. The net impact along the main corridor of Curry Creek at the west project boundary is a reduction of 37 cfs in the 100-year event.

TABLE II.E.3B – REQUIRED ATTENUATION CREATION AREAS

Attenuation Location Name	Description	Pre-Project Storage this Reach (AF)	Post-Project Storage this Reach (AF)	Added Storage this Reach (AF)
Curry Creek				
OS-16	Downstream of Fiddymt, Upstream of Upland	11.4	17.1	5.7
OS-15C OS-15D	Downstream of Upland Upstream of Pedestrian Crossing #1	11.8	14.2	2.4
OS-15A OS-15B	Downstream of Ped Crossing #1, Upstream of Market Street	26.5	34.5	8.0
OS-14	Downstream of Market Upstream of Baseline Road	26.7	11.8	-14.9
OS-12	Downstream of Baseline Road, Upstream of Watt Avenue	15.1	15.9	0.8
OS-11C	Downstream of Watt, Upstream of Ped. Crossing #2	9.1	21.6	12.5
OS-11B	Downstream of Ped Crossing #2, Upstream of Ped Crossing #3	6.6	24.7	18.2
OS-11A	Downstream of Ped Crossing #3 to Confluence with Federico Creek	17.4	31.7	14.3
Federico Creek				
OS-8B OS-8C	Downstream of east-west road, Upstream of Ped Crossing #4	3.3	3.8	0.5
OS-8A	Downstream of Ped Crossing #4 Upstream of east-west road	3.4	5.0	1.6
OS-7A	Detention Basin within power line corridor downstream of Market Street	0	20	20
OS-3	Downstream of east-west road, Upstream of Market Street	1.1	1.2	0.1
OS-2B	Downstream of Market Street, Upstream of Ped Crossing #5	6.3	8.8	2.4
OS-2A	Downstream of Ped Crossing #5 Upstream of Westside Drive	12.0	22.0	9.9
OS-6B OS-6A	Downstream of Westside Drive Upstream of north-south road	14.0	17.3	3.2
OS-5	Downstream of north-south road Upstream of Watt Avenue	6.8	7.8	1.0
OS4B	Downstream of Watt Avenue to Confluence with Curry Creek	29.8	36.7	6.9
TOTAL STORAGE ADDED = 91.1 AF				

Table II.E.3B identifies the estimated amount of additional storage volume proposed at the various Attenuation Areas shown on Exhibit AT-1. The creation of these volumes provides storage in the creek system which mitigates peak flow increases from the proposed development areas. The table indicates that the project will construct 91,1 acre feet of added attenuation within the 100-year floodplain.

III. Hydraulics:

III.A Flood Plain Analysis:

A portion of the west end of the Sierra Vista Specific Plan is shown on the FEMA Flood Insurance Rate Map (FIRM) panel 06061C0475F dated June 8, 1998, as “Approximate Zone A” floodplain. This area does not have floodway delineation on the FIRM, nor are base flood elevations specified. The City of Roseville may require the project to submit Conditional Letter of Map Revision (CLOMR) and Letter of Map Revision (LOMR) applications to FEMA for the proposed Base Flood Elevation data where changes are proposed, and to remap the approximate Zone A floodplain to Zone A with Floodway throughout the project. A CLOMR is typically applied for once a project planning document is approved and the grading concept (for areas within the floodplain) is designed to a point where substantial revisions are not expected. A LOMR application is requested once all work within the floodplain is complete, and as-built information can be certified for FEMA.

The project proposes to provide onsite attenuation within the over bank areas as well as to cross the floodplain at several locations with bridge/culvert crossings for roadways and pedestrian trails as shown on exhibit AT-1.

The results of the detailed hydraulic study of the pre-project floodplains for all tributaries of Curry Creek within the project is shown on exhibit FP-1. The estimated Post-project floodplains for the mitigated condition are shown on exhibit FP-2. FP-3 identifies the “future fully developed, unmitigated” floodplains for the project which is the assumed post-project floodplain for design purposes.

Floodway analysis was not performed with the hydraulic analysis included with this study and will be performed with FEMA application studies.

The Army Corps HEC-RAS software version 3.1.3 was utilized to develop the included hydraulic models for the project. Floodplain elevations were determined for the 100-year event, for the Pre-Project and Post-Project conditions. The HEC-RAS base models used in this analysis were obtained from the RUSP analysis and the Placer Vineyards analysis. Detailed cross sections within the project areas were re-cut based on the project topography. Where grading is proposed for the project attenuation features, roadway and pedestrian trails, and stormwater quality improvements, the post-project models were updated accordingly. Figures III.A-1 to III.A-19 show the cross sections and locations of excavation where side-channel excavation for wetland creation and stormwater quality vegetated swales are proposed.

Future roadway culvert sizing for the crossing of Curry Creek and Baseline Road, were previously identified in the Placer Vineyards Study to require a single 14’x5.6’ Conspan Arch at the upstream crossing (station 1.215) and a single 16’x5.5’ Conspan Arch at the downstream crossing. We have not changed those recommendations, but have corrected the culvert lengths and inverts in our hydraulic analysis. Table III.A.1 identifies the

proposed culvert and bridge sizes per the HEC-RAS analysis throughout the project.

A comparison of the pre-project and post-project water surface elevations for the 100-year event, at project boundary and critical locations are presented in TABLE III.A.2.

III.A.1 Pre-Project Hydraulic Model Parameters:

The HEC-RAS version 3.1.3 software was utilized to perform an unsteady state hydraulic analysis of the existing floodplains. A site visit was performed to evaluate the stream conditions and estimate 'n' values for channel friction at the low flow and overbank areas. We have established creek wide values of 0.045 for the low flow channel and 0.06 for the overbank areas for this model. Two culverts and a small bridge exist within the modeling reach. The two culverts are located at the two Baseline Road crossings on the main branch of Curry Creek, where runoff waters 1st leave the project into the Placer Vineyards Specific Plan, then Return into this Plan Area. The existing culverts at these locations are:

Station 1.21: Upstream Baseline Road Culvert = 8.5ft wide x 6 ft high metal arch

Station 1.04: Downstream Baseline Road Culvert = 2 culverts each is 5ft wide by 3.5 ft high metal arch.

A small agricultural bridge crossing also exists on the main Reach of Curry Creek within the project, downstream of the 2nd Baseline Road crossing. A cross section of this bridge is shown below. It is estimated that this bridge is overtopped in less than a 100-year event, from its appearance in the field.

The pre-project floodplain, mapped on the FP-1 exhibits is our best estimate of the 100-year flood limits in the existing conditions through the project. The pre-project model also generates the pre-project hydrology for the basis of comparison with post-project conditions analysis.

III.A.2 Post-Project Mitigated Hydraulic Model Parameters:

The post-project mitigated condition hydraulic model includes all bridges, culverts, and attenuation features planned for the project within the creek corridors. 'n' values are adjusted for this model to account for nuisance flow contributions to vegetation growth along the stream corridor. 'n' values of 0.08 are assumed in the main channel and overbank areas. An 'n' value of 0.10 was applied to the wetland creation areas and the stormwater quality vegetated swales at the pipe outfalls.

Hydrology produced from this analysis is compared to the pre-project analysis to demonstrate mitigation objectives are being achieved by project improvements.

III.A.3 Future, Fully Developed, Unmitigated Hydraulic Model Parameters:

For this scenario, ‘n’ values from the post-project mitigated scenario are used. Wetland Creation areas were removed from the hydraulic model, so that the attenuation resulting from the floodplain volumes created by these features would not be counted. The model removes the routing within the hydrology models for detention basins within the Westpark subdivision, and assumes that mitigation is no longer functioning. Additionally, within the model, the edges of the channel were obstructed approximately 5 feet horizontally into the floodplain to account for future variability in the design of the edge features.

This scenario forms the basis floodplain for use in design of the project. The floodplain limits identified by this model will be used in the project CLOMR when that is submitted to FEMA. The base flood elevations established in this analysis are used for determining the elevations with required freeboard of structures to be constructed on the project. Since the depths of the creeks within this project are generally less than 8 feet, the City of Roseville requires that all structures constructed within this project will be required to have a minimum of three (3) feet of freeboard between the base 100-year flood elevations and the proposed pad elevation.

TABLE III.A.1 – PROPOSED CULVERT/BRIDGE SIZES

Station	Description	Approx. Crossing Length	Culverts
Main CURRY CREEK			
1.385	Curry Creek – Upland Drive	108’	1-14’w x 4.3’h CONSPAN
1.265	Curry Creek – Market Street	60’	1-14’w x 5.3’h CONSPAN
1.215	Curry Creek - Baseline Road upstream leaving project.	214’	1-14’w x 5.6’h CONSPAN
1.025	Curry Creek – Baseline Road downstream entering project	289’	1-16’w x 5.6’h CONSPAN
0.87	Curry Creek Main – Watt Avenue	248’	1-16’w x 5.6’h CONSPAN
Federico Creek			
0.9954	East-West Road	128’	1-12’w x 3.5’h CONSPAN
0.975	Market Street	95’	1-12’w x 3.5’h CONSPAN
0.975	Market Street to Detention Basin	250’	1-42” pipe culvert
0.865	Westside Drive	160’	1-12’w x 3.5’h CONSPAN
0.805	North-South Road	75’	1-14’w x 4.2’h CONSPAN
0.775	Watt Avenue	175’	1-14’w x 4.2’h CONSPAN

Note: the above design bridge and culvert sizes may be modified in final design, with alternate bridge and culvert types with equivalent capacity. Culvert design will impact the hydraulic grade lines, which in turn impact the proposed attenuation of the project, therefore, detailed analysis demonstrating the proposed hydraulic grade lines will be required with design plans for these culverts.

Baseline Road Culverts at stations 1.215 and 1.025 are located at the transition between Roseville and Placer County. Placer County has approved a project, “Placer Vineyards” which identifies the need for these culverts to be as noted in the below excerpt from that report. The

culverts specified in that study are hydraulically and dimensionally equivalent to the culverts specified for these locations in this study.

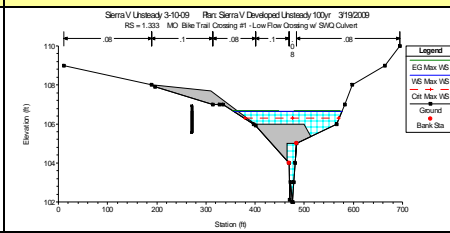
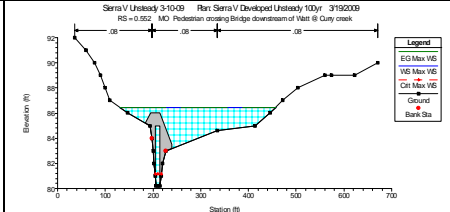
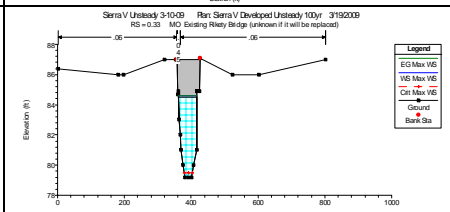
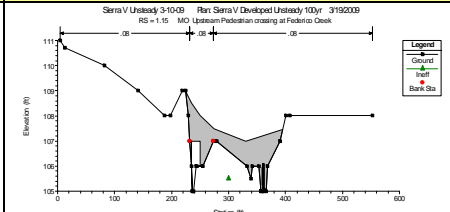
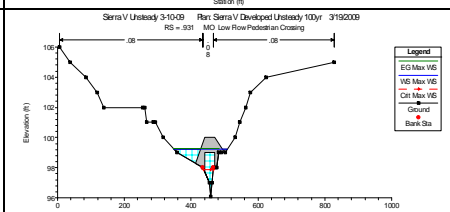
Shed	Station	Culvert Description	Overtopping Flow Rate (cfs)	Post Project overtopping event (yr)	Post-project 100-year flowrate (cfs)
CURRY CREEK					
CUS6	1.215	Baseline Road – Replace Existing Culvert with 150' of Dual 14'x5' CM Arch Culvert	830	500	714 772 (2)
CUS11B	1.158	Inline Detention Weir – Notched at existing low flow channel	665	200	478 509 (2)
CUS11B	1.115	Roadway Crossing 1-16x5.5 CM Arch	NONE	NONE	403 411 (2)
CUS14	1.025	Baseline Road – Replace existing culvert with 200' of 16'x5.5' CM Arch	NONE	NONE	407 439 (2)

Excerpt from Placer Vineyards Master Project Drainage Study TABLE IIIB1 – Culvert Sizing

Final design of the project culverts will require detailed hydraulic analysis demonstrating that the proposed hydraulic elevations will result for the proposed flow rates for the post –project mitigated and unmitigated peak flow rates in the 10-year and 100-year events, so as to maintain instream attenuation levels at the levels proposed by this study.

Pedestrian Trail Crossings are assumed to be flatcar or other bridge type of construction, to span the low flow creek resources. These crossings and the trails leading up to the bridges were placed at existing grade. Bridges were sized to span the natural resources in the low flow channel of the creeks. In some cases abutment rises were sloped so that the bridge could be raised so that flows could pass beneath the bridge deck soffit. The bridges and the ped trail paths at the open space crossing locations will not meet the current City design standards which require the bridges to be elevated above the 10-year floodplain elevation, and are expected to be overtopped in the 100-year event..

TABLE III.A.2 – PROPOSED PEDESTRIAN CROSSINGS

Station	Description	Approx. Crossing Length	Description	Cross Section
Main CURRY CREEK				
1.333	Curry Creek – Pedestrian Crossing #1	15'	Bridge spanning creek main channel, and 2x12" culverts at SWQ channel crossing	
0.552	Curry Creek – Pedestrian Crossing #2	15'	Bridge spanning creek main channel	
0.33	Curry Creek – Pedestrian Crossing #3	15'	Make use of existing bridge structure or replace structure in kind	
FEDERICO CREEK				
1.15	FC – Pedestrian Crossing #4	15'	Bridge spanning creek main channel, and 2x12" culverts at SWQ channel crossing	
0.931	FC – Pedestrian Crossing #5	15'	Bridge spanning Main Creek Channel	

Note: the above design bridge and culvert sizes may be modified in final design, with alternate bridge and culvert types with equivalent capacity.

Table III.A.3 compares the hydraulic modeling results for the pre-project and post-project mitigated conditions at various project boundary locations for the peak flowrates of the 100-year event. The Table also demonstrates the project will not be adversely impacting offsite peak 100-year water surface elevations. Finally, Future, Fully Developed, Unmitigated conditions resulting water surface elevations are also reported, which are the basis for floodplain design considerations.

TABLE III.A.3 –Water Surface Comparison (Pre- and Post-Project)

Station	Description:	Pre-project 100-year HGL	Post- project 100-year HGL	Reduction (ft)
MAIN CURRY CREEK				
1.44	Downstream side of Fiddymet Road	114.55	114.55	0.00
1.22	Upstream side of Baseline Road	104.71	102.97	1.74
1.21	Downstream side of Baseline Road (offsite)	100.37	100.18	0.19
1.03	Upstream side of Baseline Road (offsite)	92.09	90.72	1.37
1.02	Downstream Side of Baseline Road	89.40	88.96	0.44
0.0007	Flow at Confluence with Federico Creek	80.49	80.37	0.12
FEDERICO CREEK				
1.2	Flow entering the northern boundary of the site at the Northern Tributary (flow added from CWP7)	107.74	108.04	-0.30*
0.937	Flow entering the east boundary of the site at the Northern Tributary. (added flow from CWP8)	99.46	99.47	-0.01*

- Increases result from required ‘n’ value modification in channel for nuisance flow contribution to vegetation propagation.
- Note: Elevations for the post-project shown in this table are not intended for design. The values represented are for the post-project mitigated scenario, and design will be based on the post-project unmitigated values.

FIGURE III.A-1 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

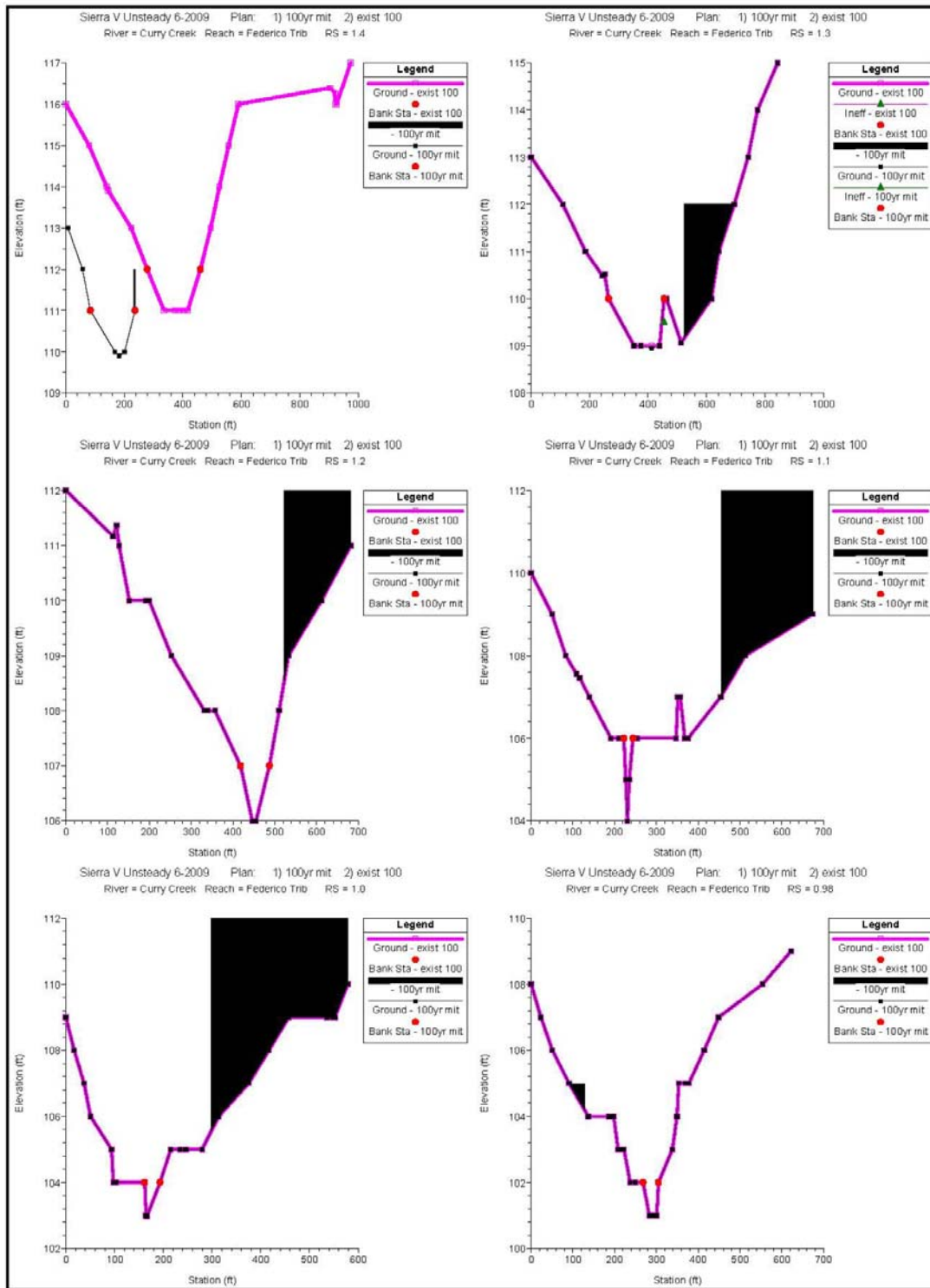


FIGURE III.A-2 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

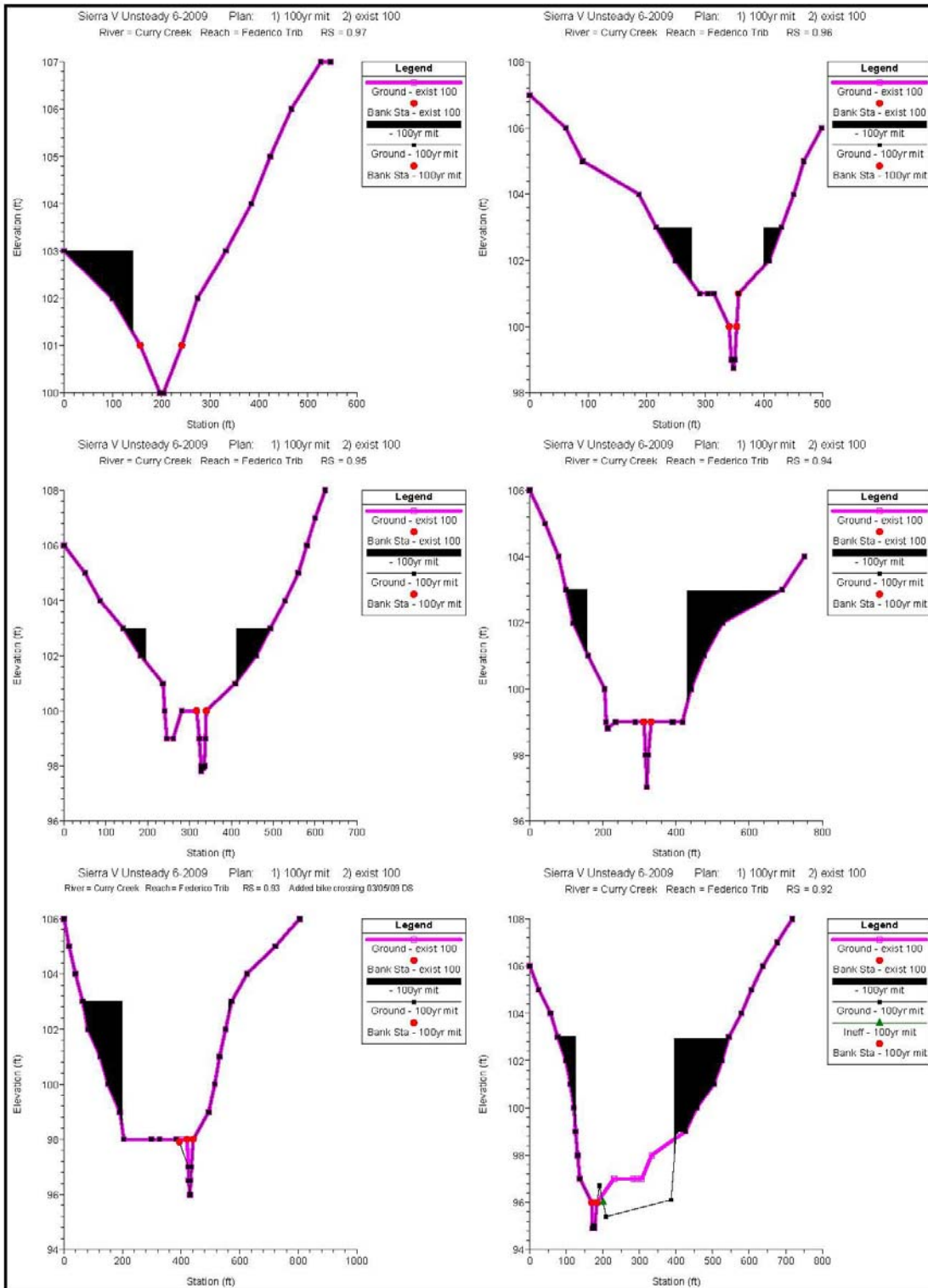


FIGURE III.A-3 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

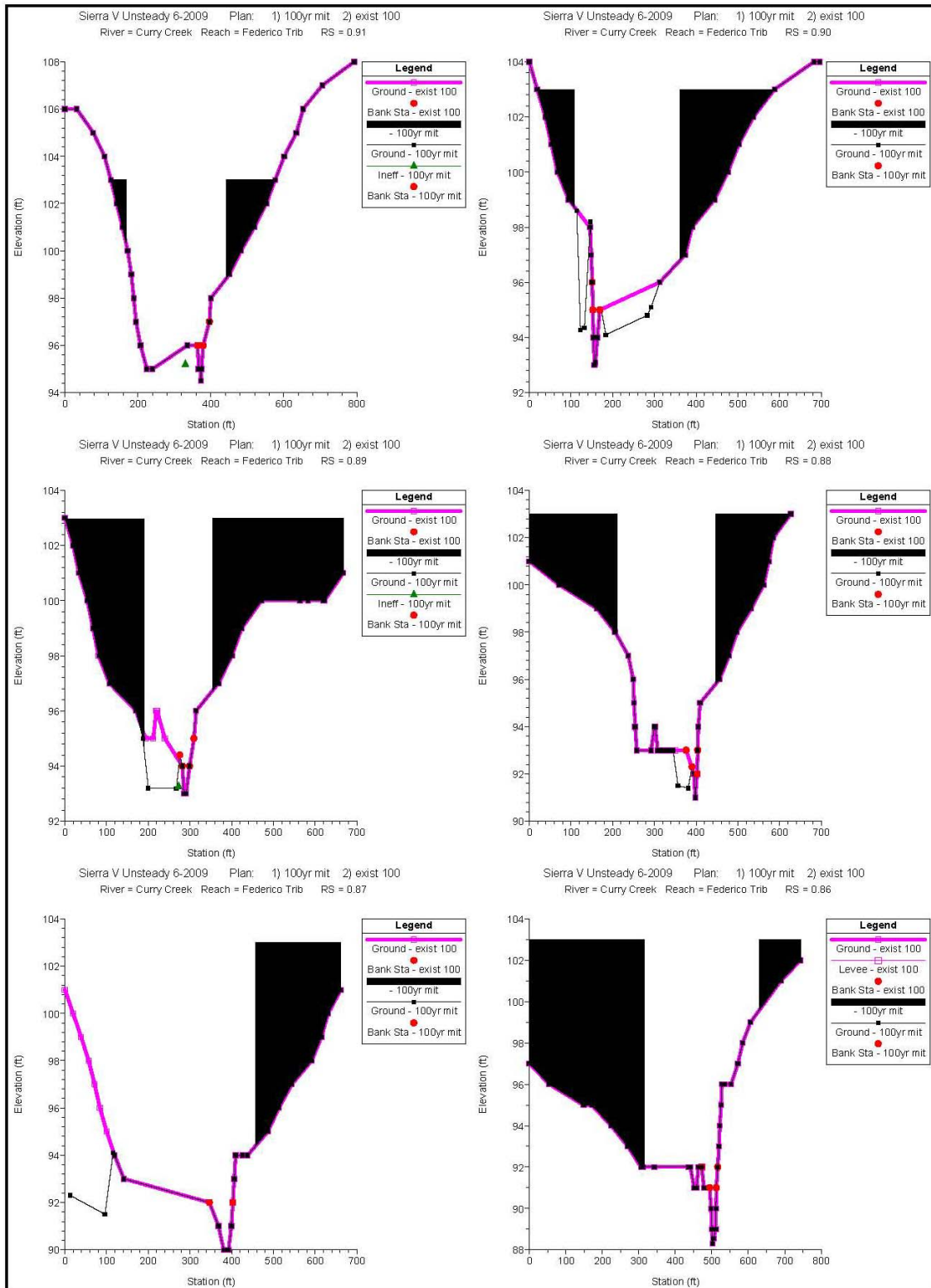


FIGURE III.A-4 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

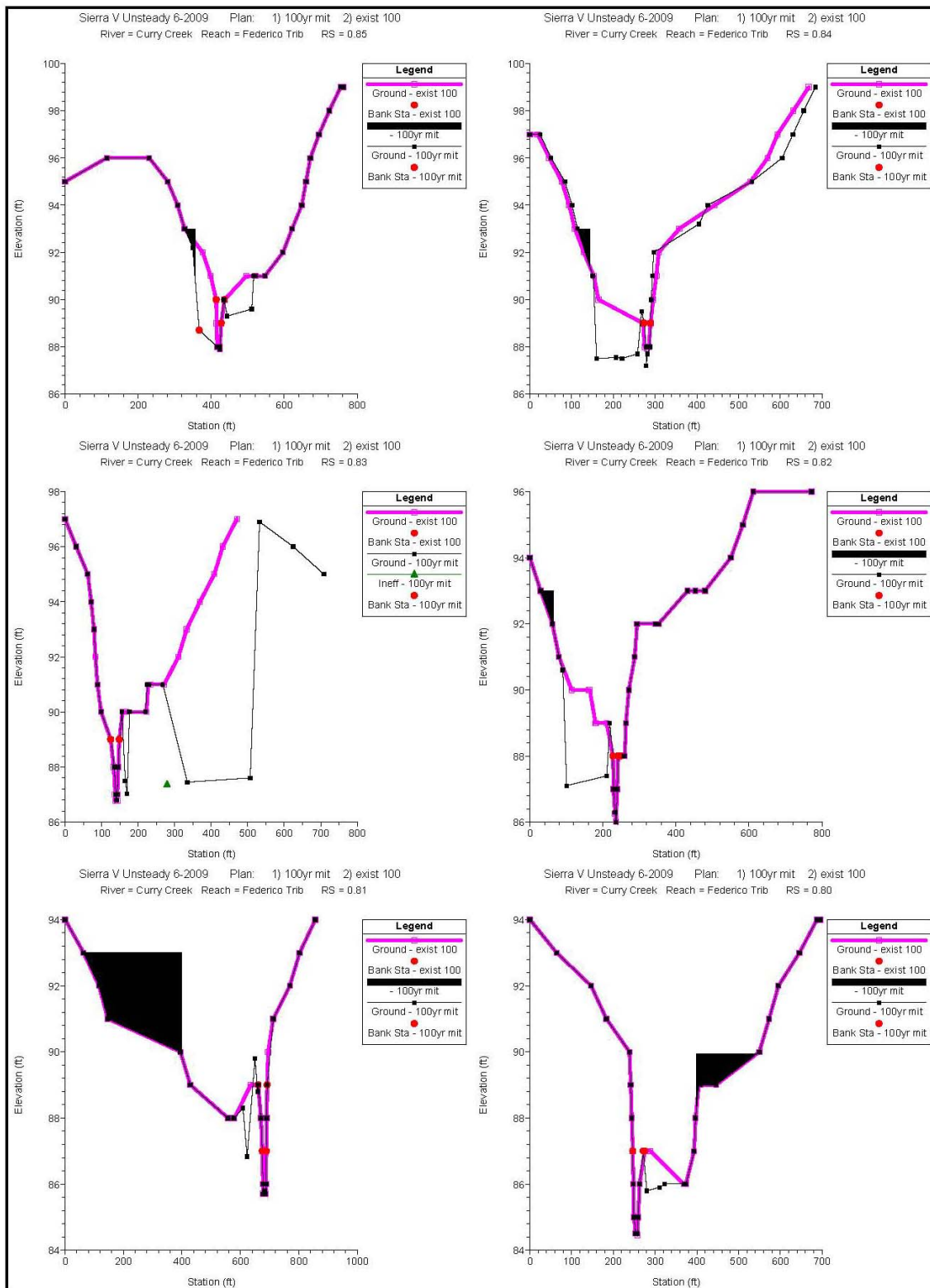


FIGURE III.A-5 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

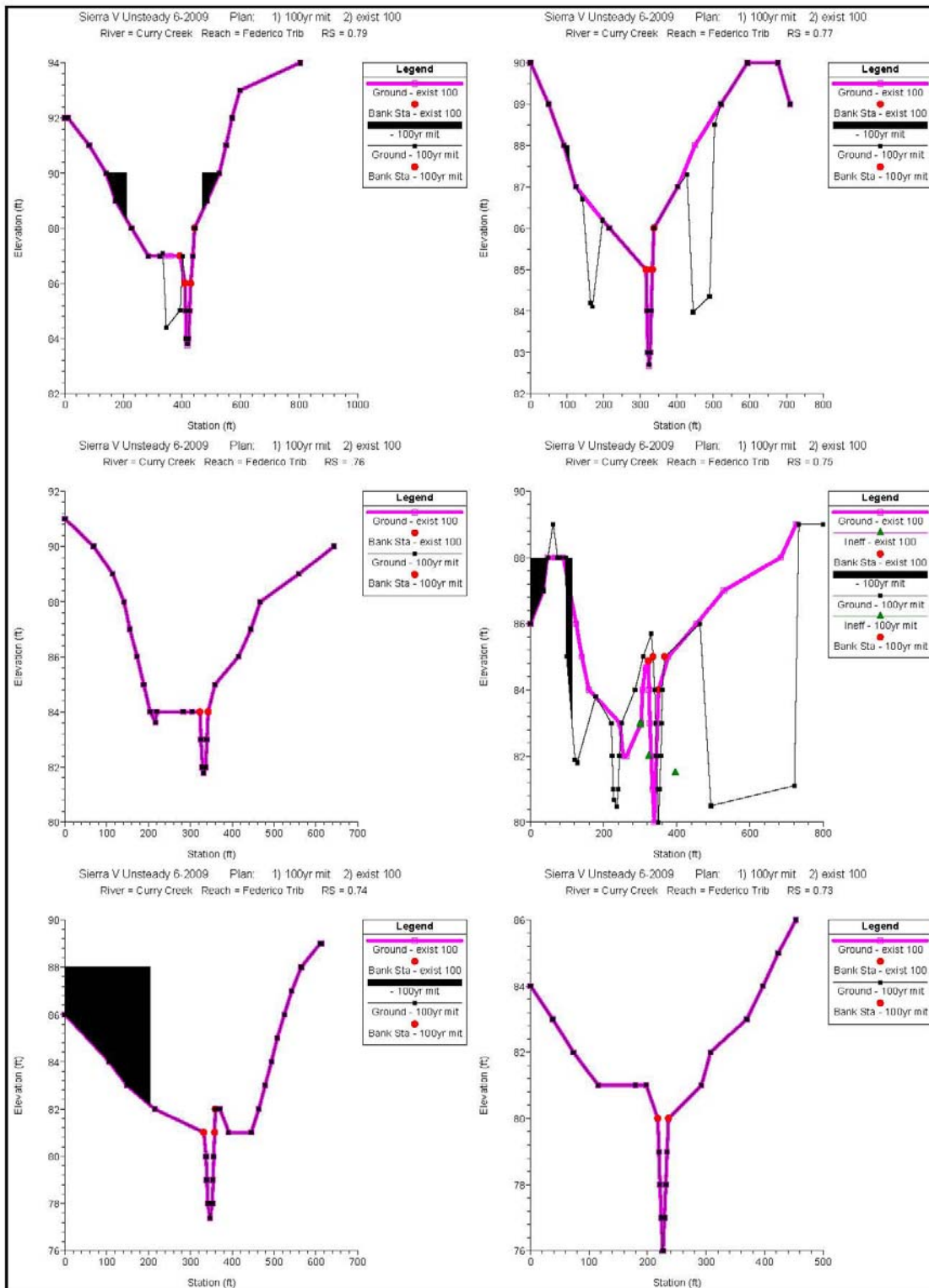


FIGURE III.A-6 - HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

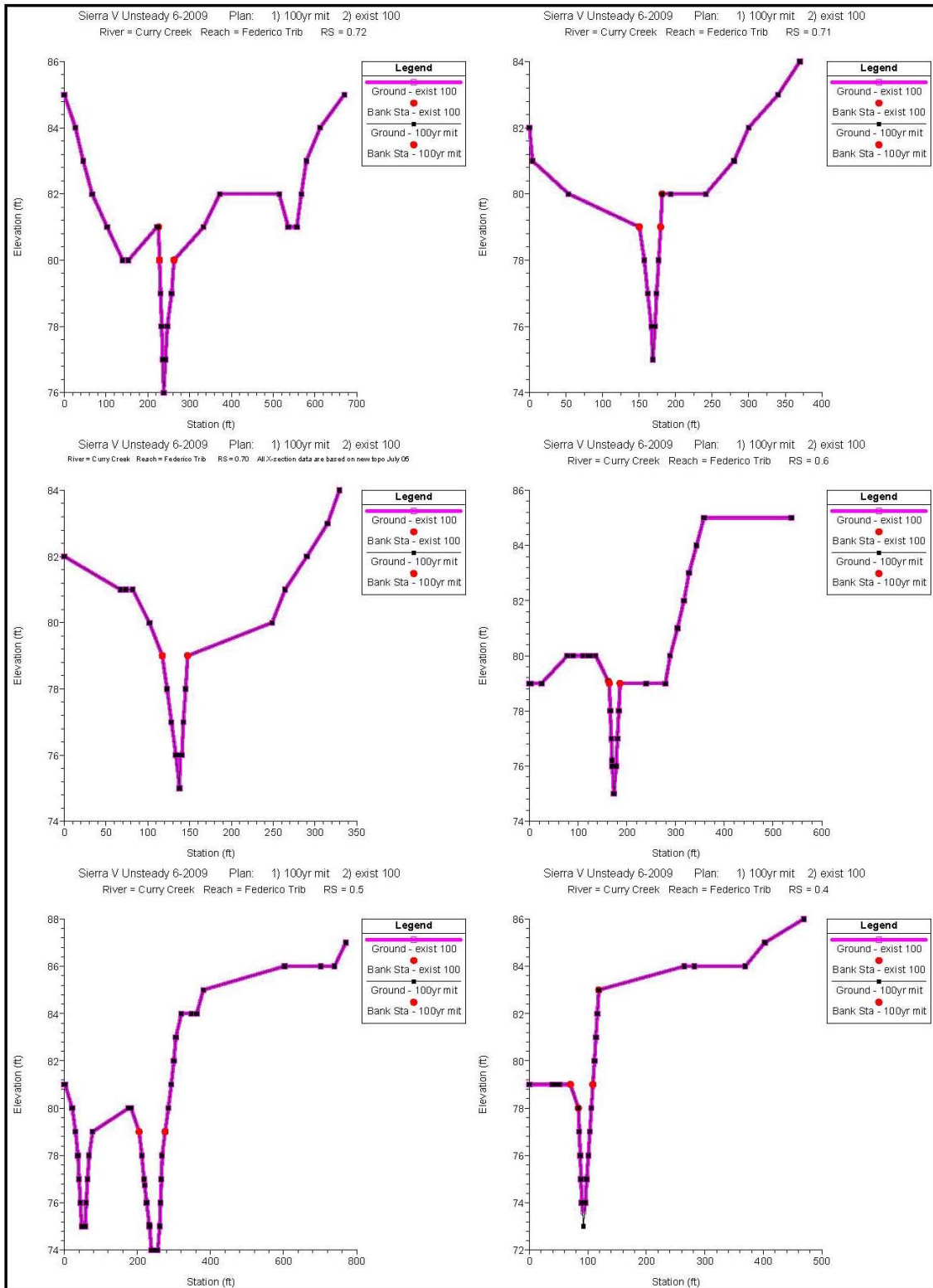


FIGURE III.A-7 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

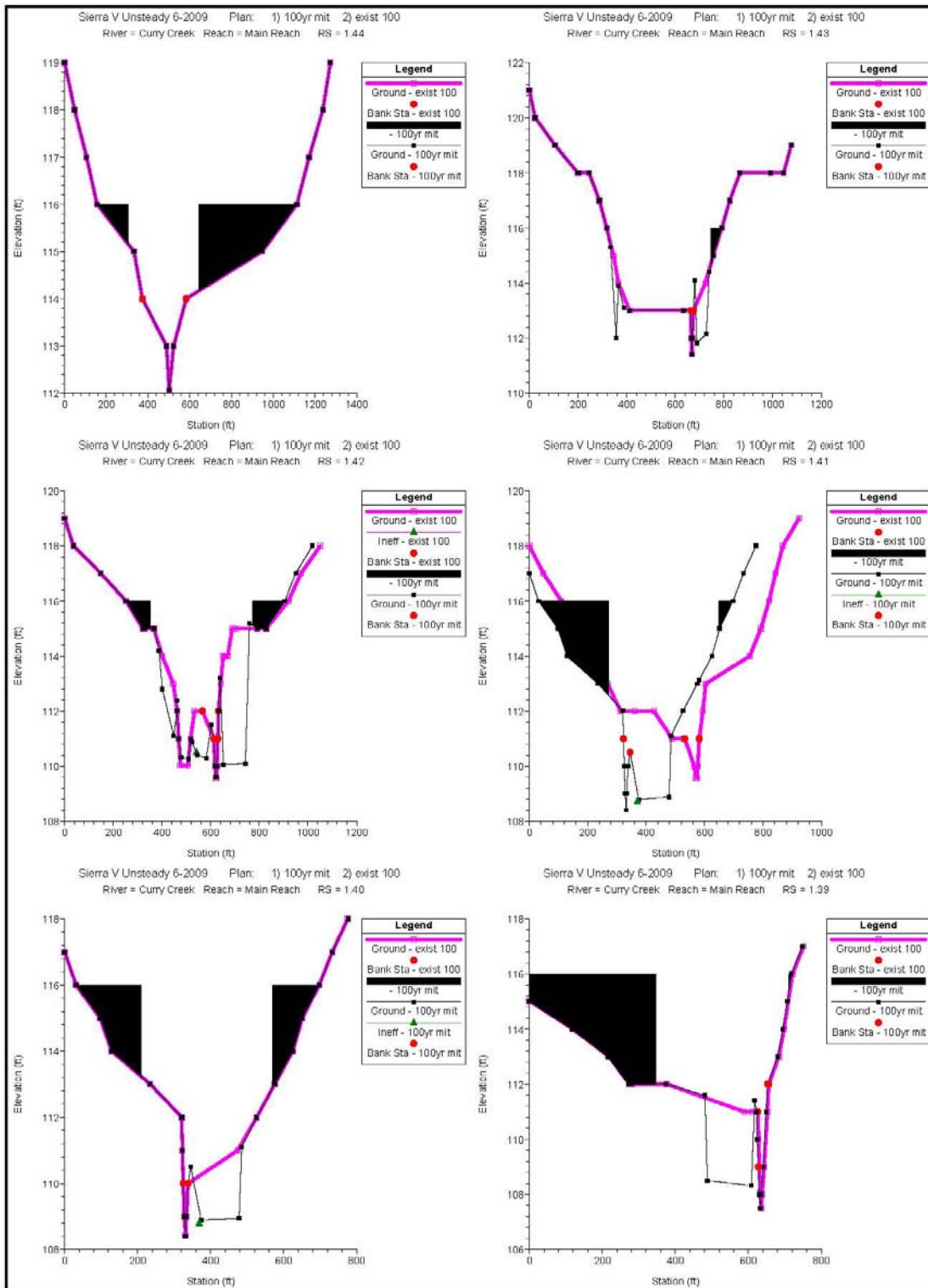


FIGURE III.A-8 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

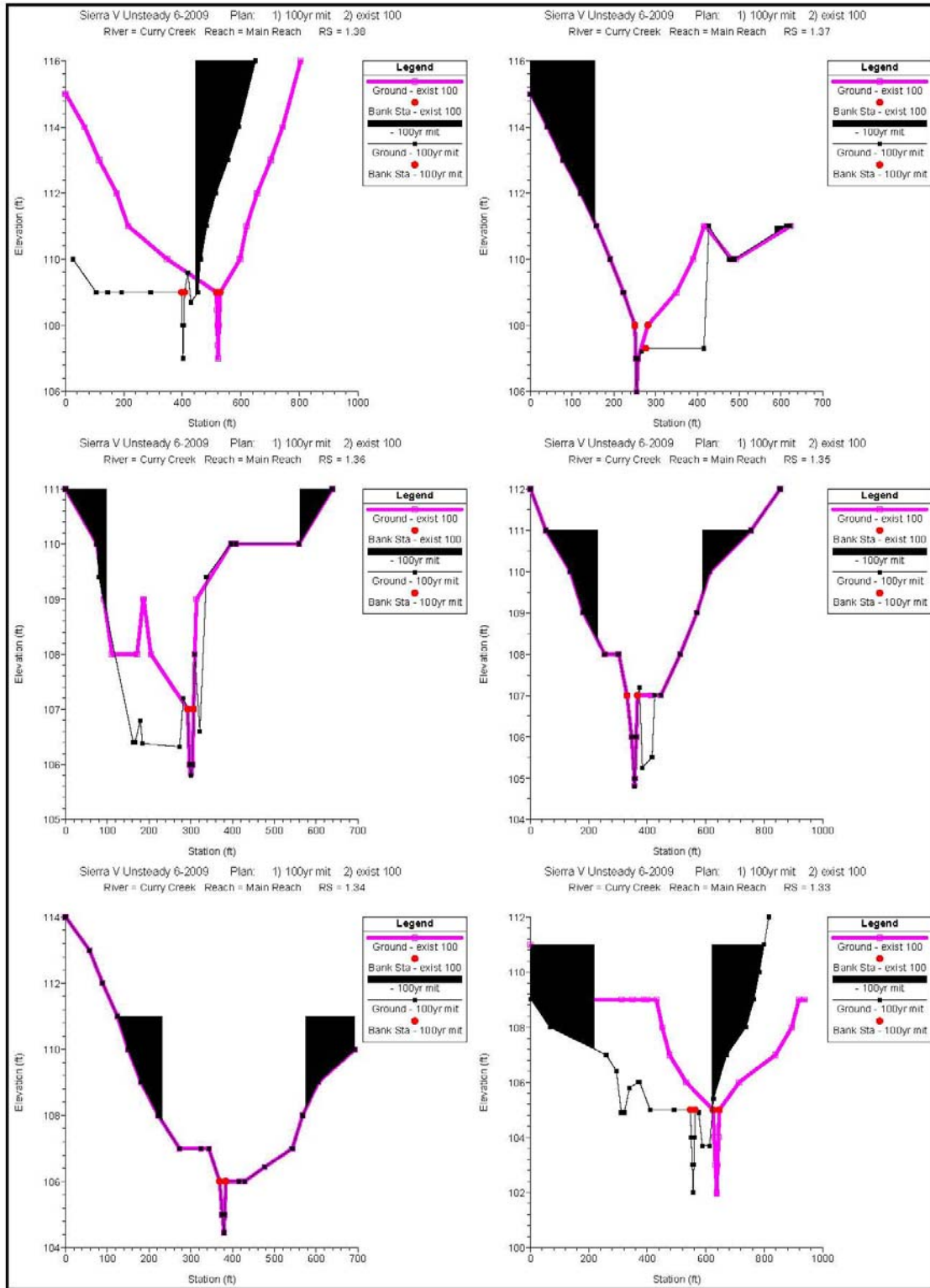


FIGURE III.A-9 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

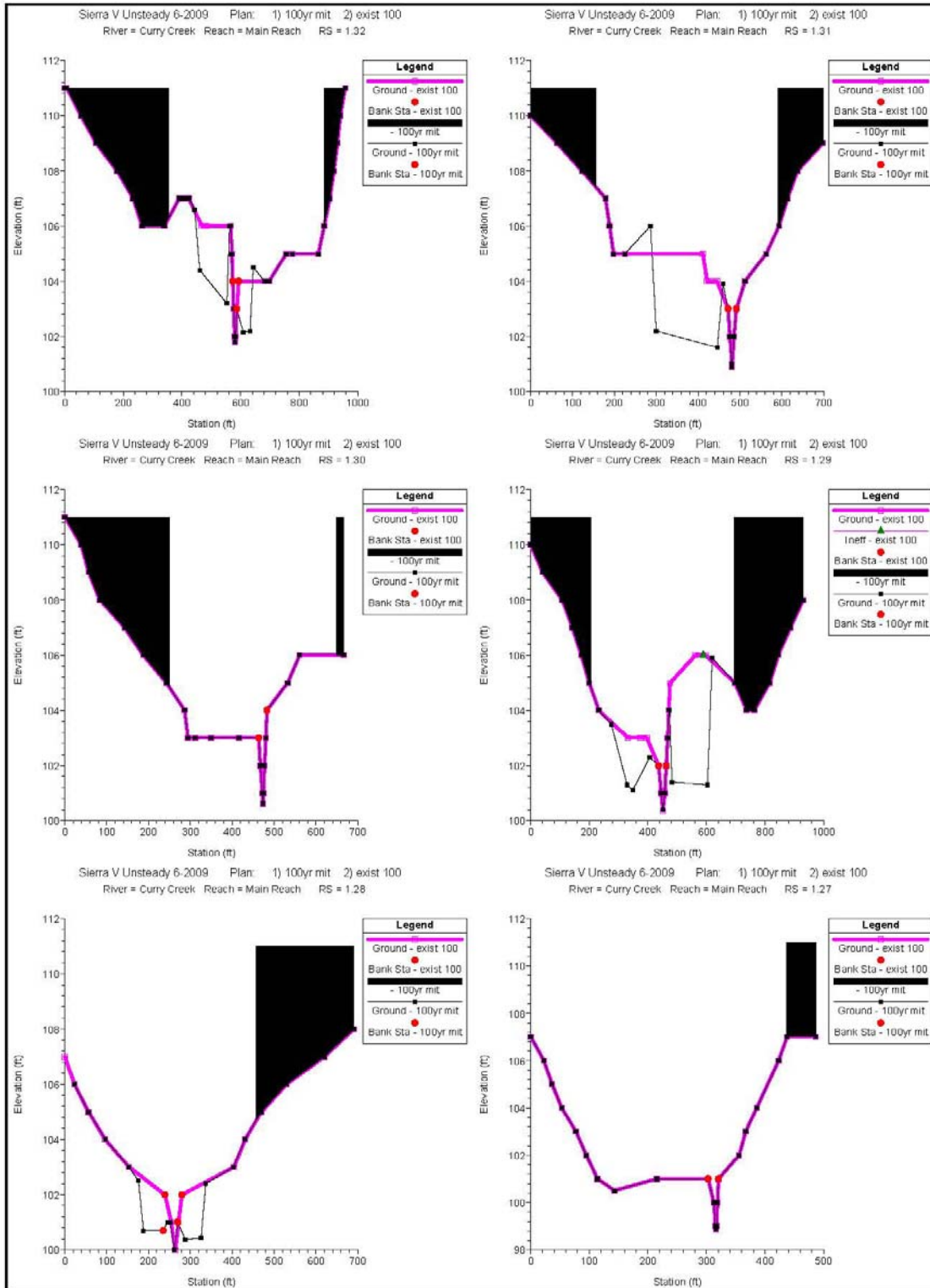


FIGURE III.A-10 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

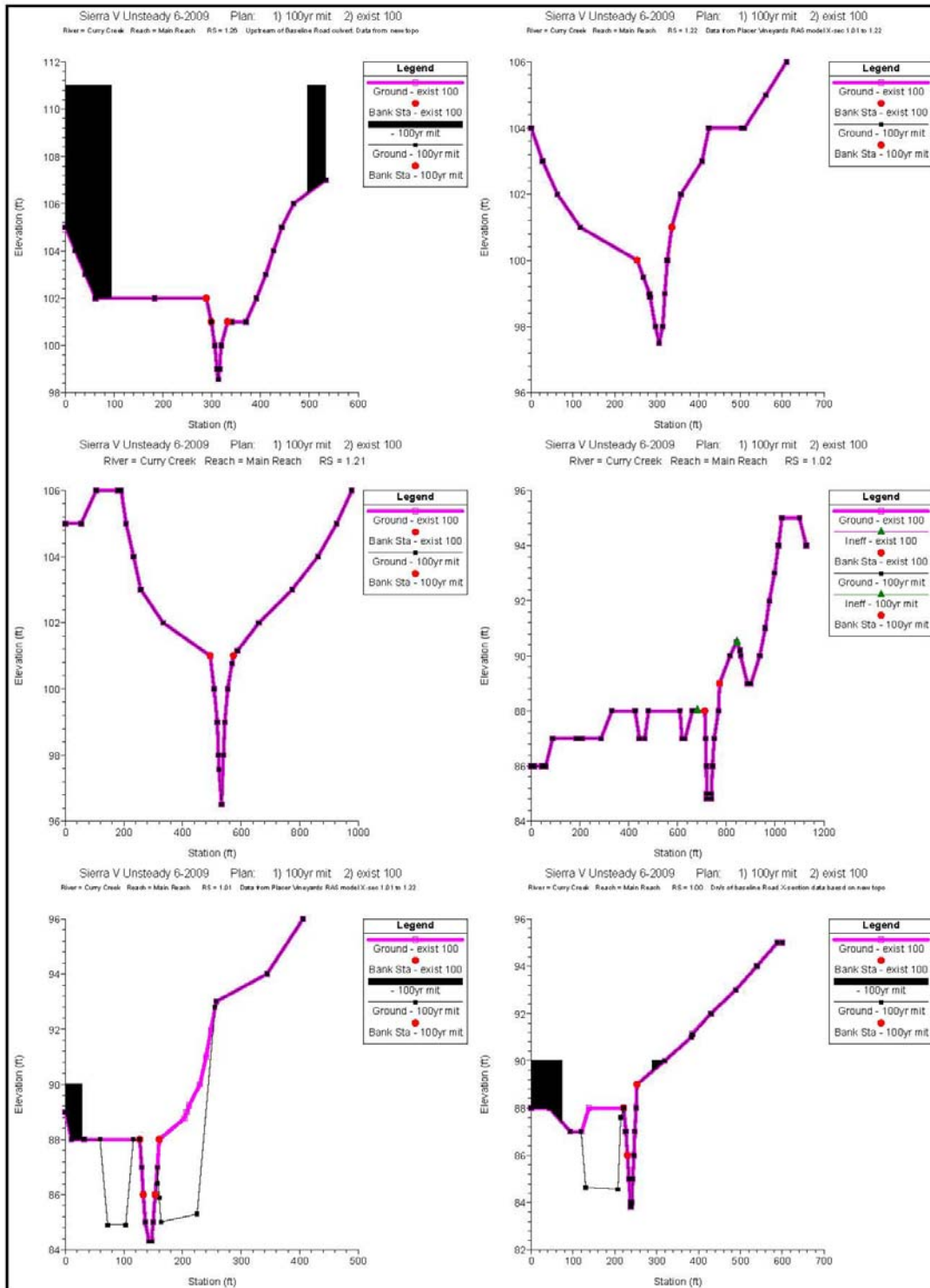


FIGURE III.A-11 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

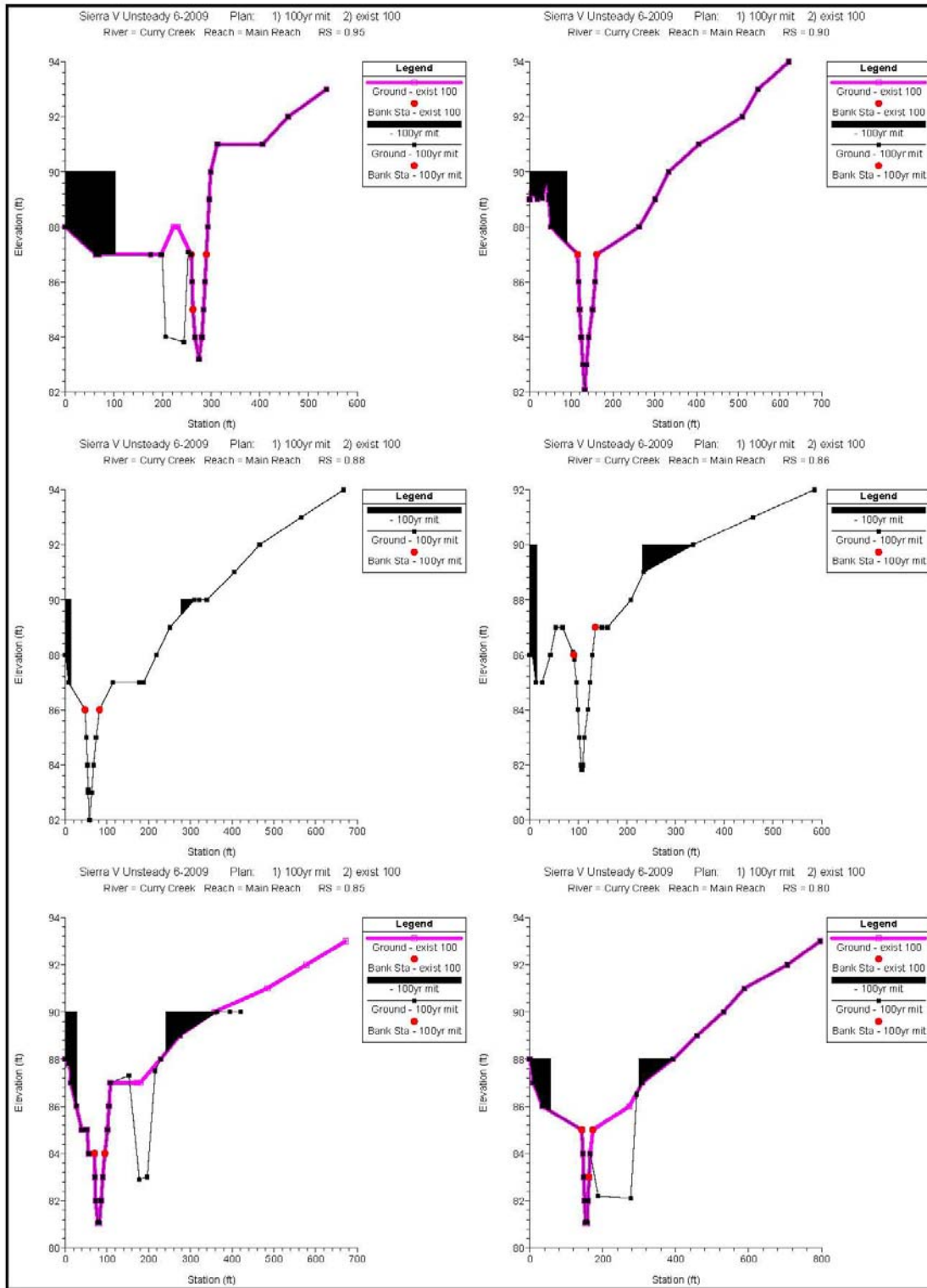


FIGURE III.A-12 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

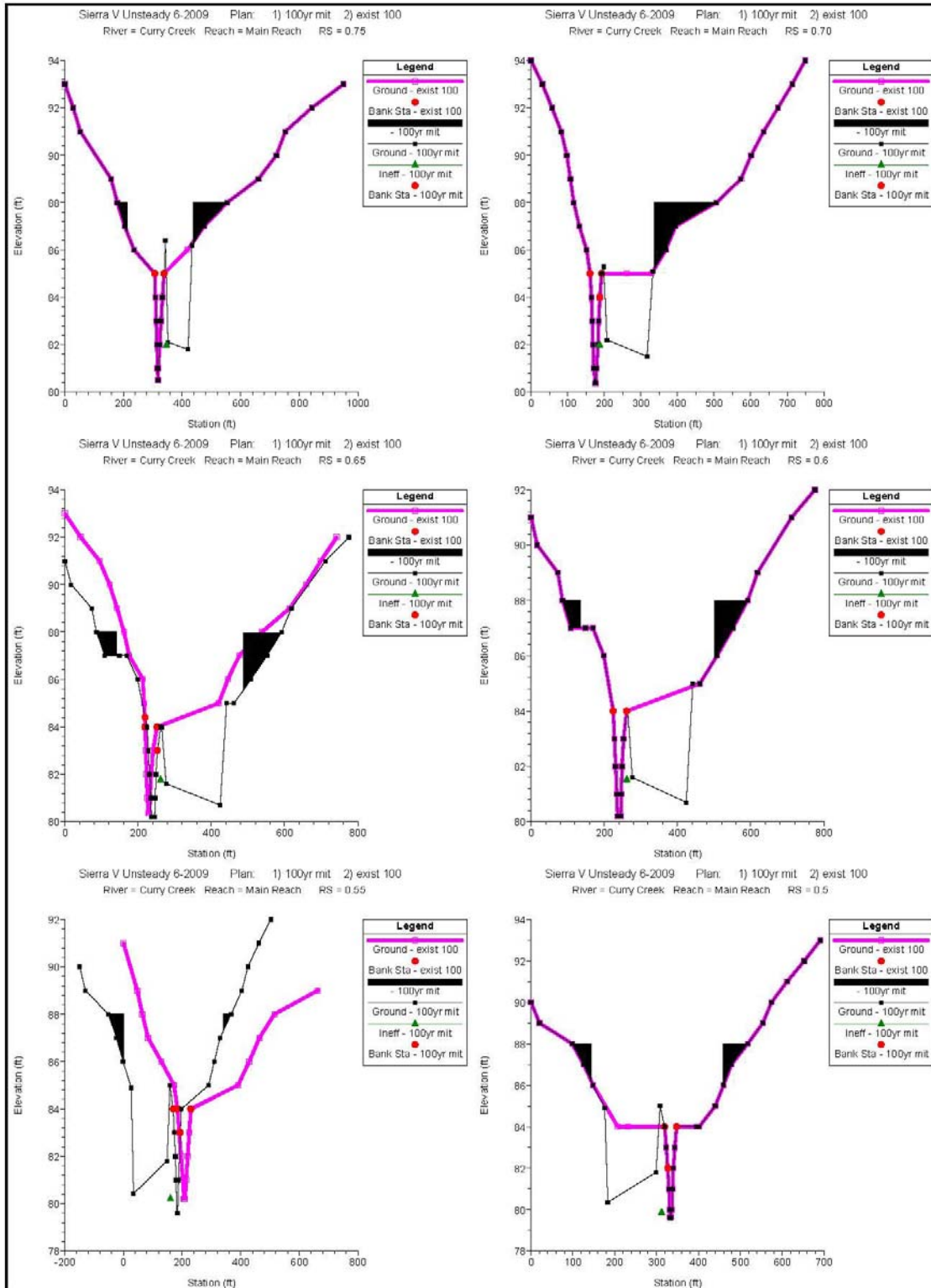


FIGURE III.A-13 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

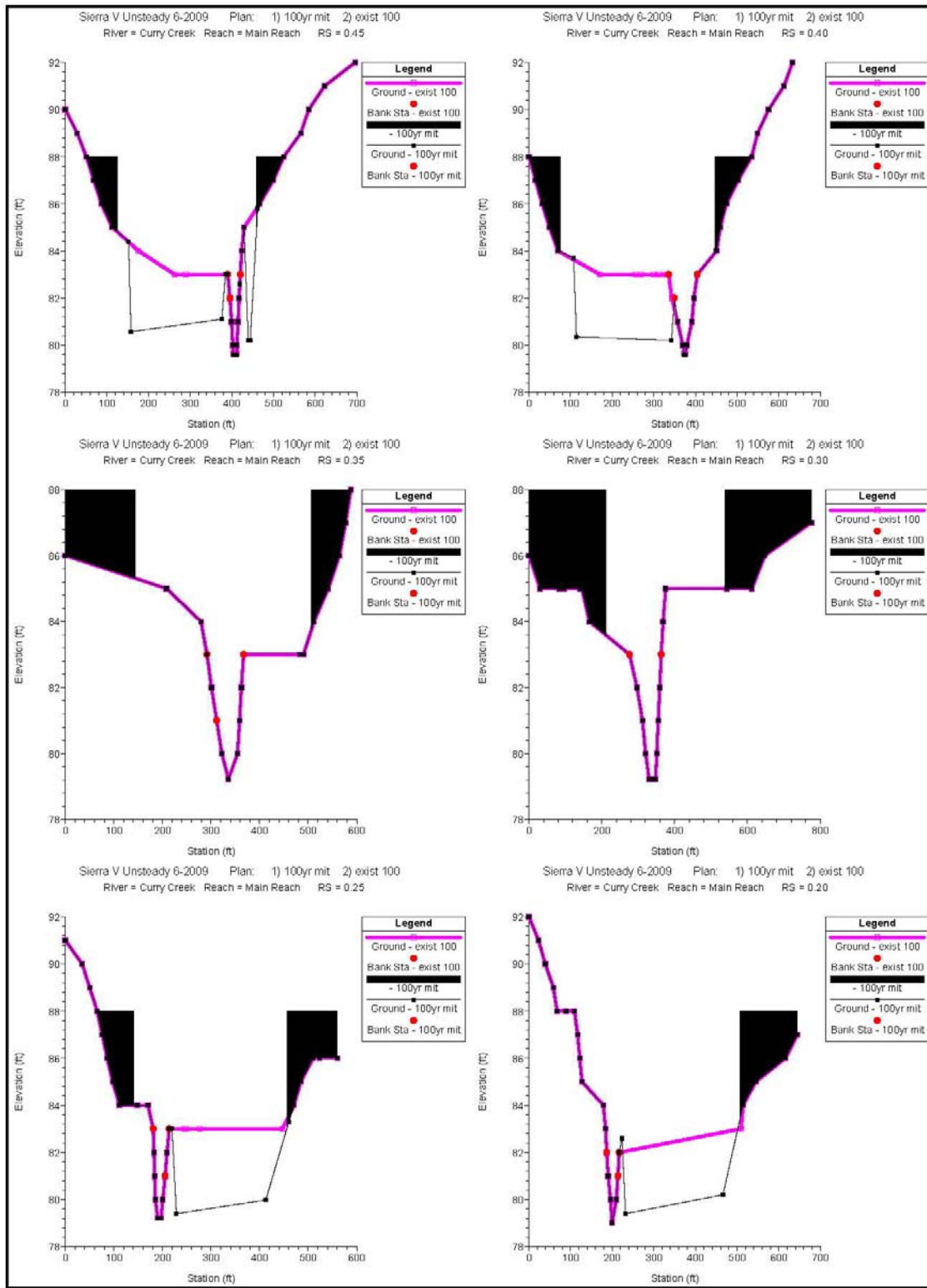


FIGURE III.A-14 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

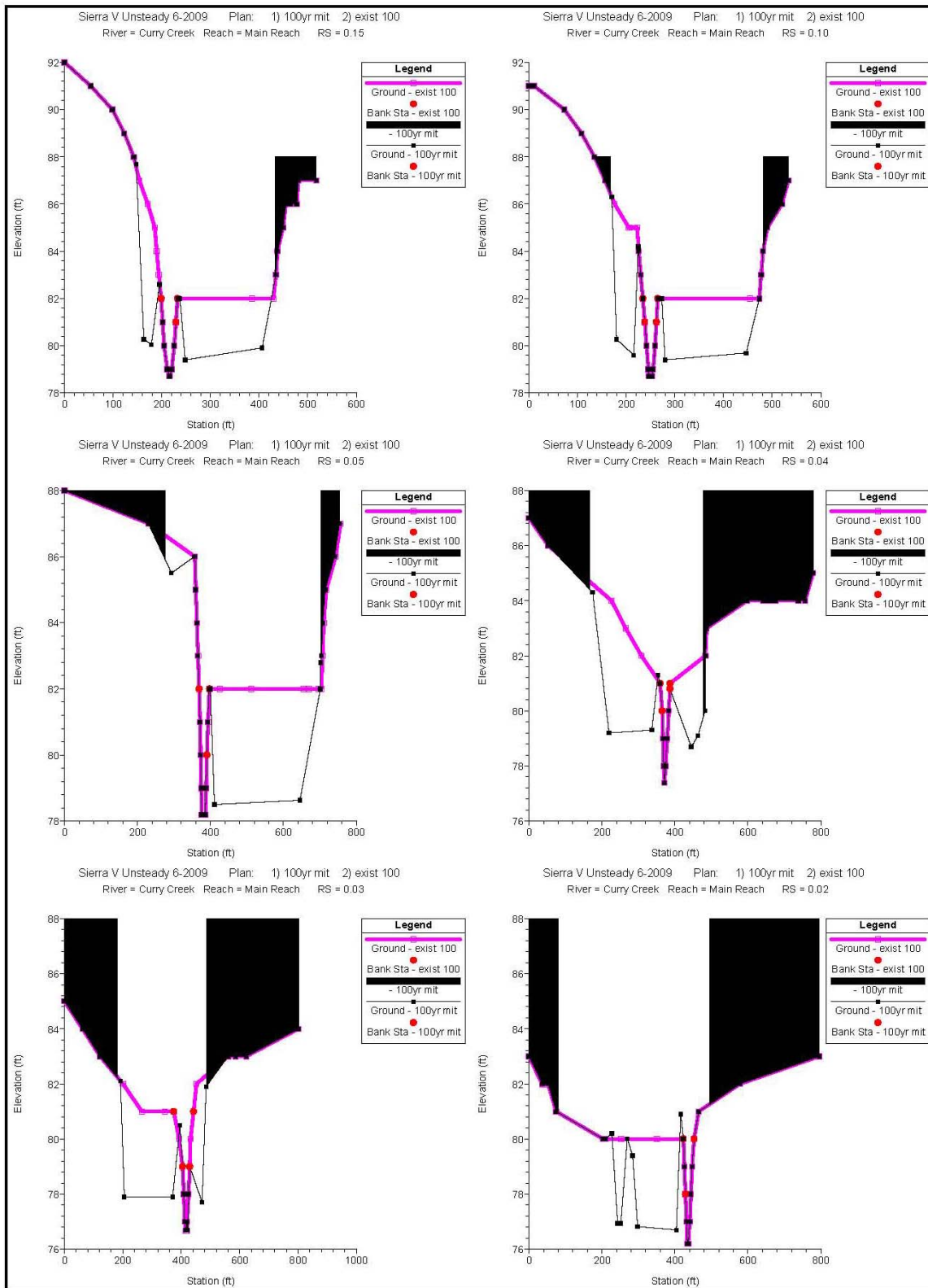


FIGURE III.A-15 – HEC-RAS CROSS SECTIONS (Pre- and Post-Project Modified)

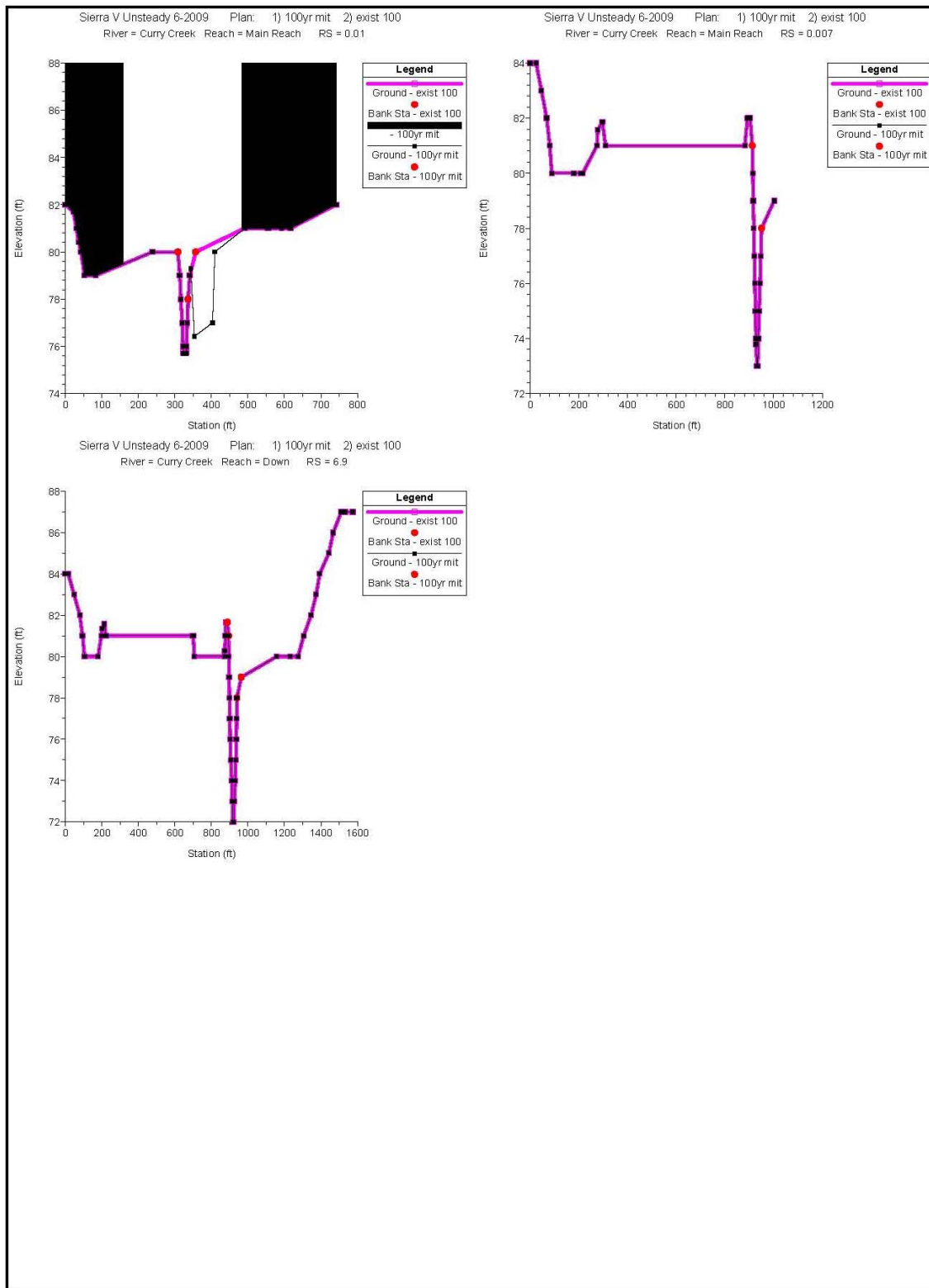


FIGURE III.1 - FIRM MAP 458F, June 8, 1998



III.B Storm Drain System:

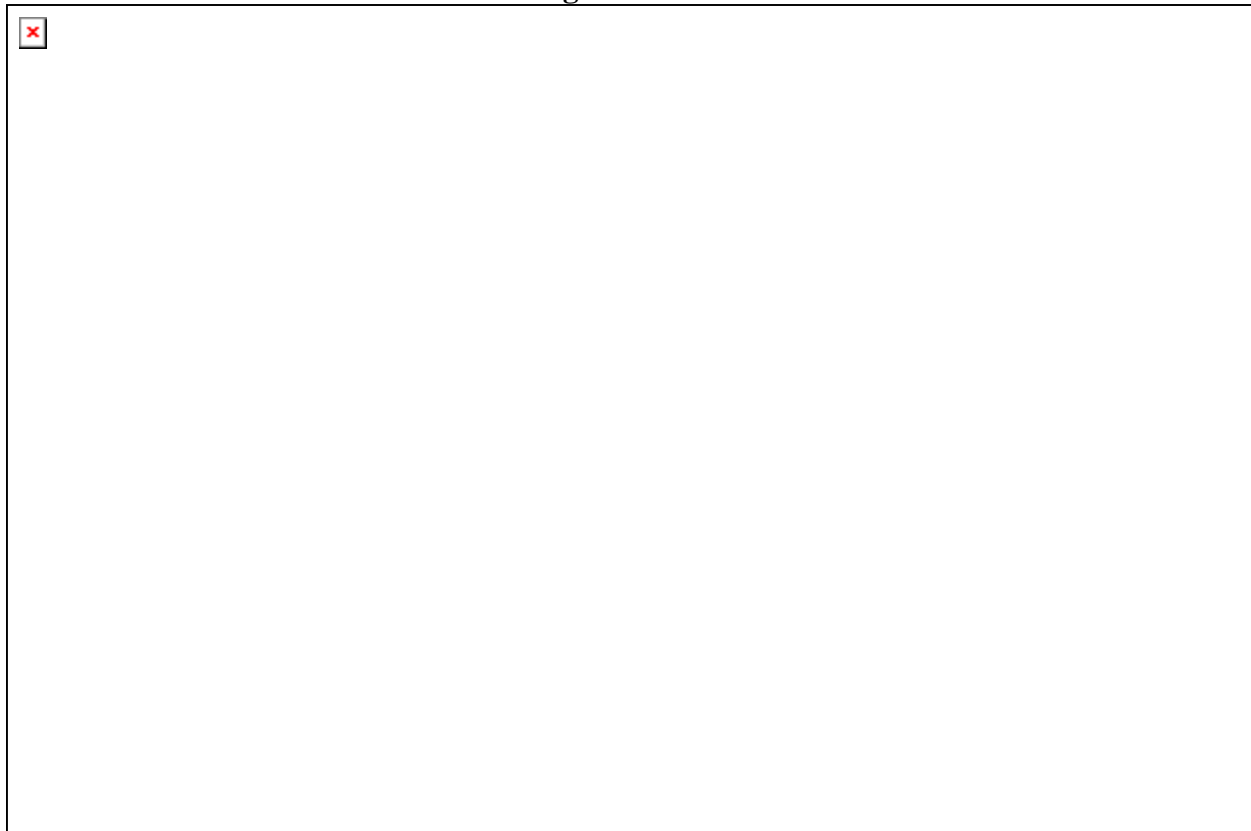
A preliminary storm drainage system is shown on Exhibit SD-1. This system has been analyzed using the CS-DRAINAGE STUDIO software program. The optimized pipe sizing is provided in the summary calculations provided in Appendix G for the 10-year and 100-year storm events. The pipes have been sized for one foot of freeboard below manhole rim elevations in the 10-year event. At Arterial and major collector streets, where street encroachments of the floodplain are limited to the non-traveled way areas in the 100-year event, the pipes have been designed to convey the 100-year flows without surface flow. Preliminary site grading was used to establish RIM grades as shown on the exhibit. These grades were also used to input surface flow channels. For both the 10-year and 100-year events, downstream floodplain elevations were specified per the post-project unmitigated floodplain analysis (see Exhibit FP-3), as the downstream “known water surface elevations”.

The storm drainage pipes discharge at several locations into the creek corridors. Generally, a vegetated swale is proposed at these discharge locations for a designed distance (per the SWQ calculations for contact time), before discharging into the creeks.

III.C Mitigation and Drainage Improvement Phasing:

A phasing plan for the project was provided by Mackay and Soms as shown in Figure III.C.1.

Figure III.C.1



The project infrastructure will be split into Four regional phases of construction. Landuse by Phase is estimated in Table III.C.1.

TABLE III.C.1

PHASE	Landuse Areas								
	COM	HDR	LDR	MDR	UR	OS	PARK	PQP	ROAD
A	34.8	22.2	357.6	94.7	94.8	94.5	45.9	60.8	29.8
B	102.3	28.1	151.1	34.8	38.1	58.7	25.5	12.0	24.0
C	87.2	7.6	156.7	120.5	291.5	80.8	77.6	14.8	48.3
D	0.0	8.3	18.5	62.9	5.7	23.2	10.7	7.3	10.1

We assume that the open space corridors will be constructed with the major infrastructure of each phase. The following drainage items are easily resolved by phase:

Volumetric Mitigation: A fee is collected by the City for these impacts, therefore, the fee will be paid at the appropriate time as the impact is constructed.

Conveyance: Trunk Facilities shown on Figure SD-1 will need to be constructed with the roadways of each phase. There are systems which will cross phase boundaries. Where this occurs, the quantity of facility that must be built will be dependant on which phase goes 1st. Where an upstream phase goes 1st, the conveyance will need to be constructed far enough downstream of the phase boundary that the facility can daylight to natural grades and can be discharged into an existing swale with proper erosion protection measures. In some cases, an upstream phase may have to excavate an outfall channel through a downstream phase to achieve daylight grade of the flows in a positive drainage, or to get to an existing swale. Where downstream phases are constructed 1st, the downstream phase will need to do sufficient analysis of the upstream phase grading and drainage to determine an appropriate pipe invert and size (see Storm Drain Analysis included herein for preliminary trunk facility sizing), such that the upstream phase can drain through the conveyance when it develops.

Stormwater Quality Treatment: For an upstream phase where temporary discharges are constructed into a downstream phase, temporary stormwater quality treatment will be necessary to treat discharges prior to release into a creek or delineated wetland (which has not been mitigated with the concurrent phase). For a downstream phase, permanent stormwater quality treatment will be built with the permanent outfall to the creek.

Low Impact Development Measures: These items will be integrated into the development. Each phase of construction will have to demonstrate that their RVR is met on a development land use area weighted basis, for each discharge location.

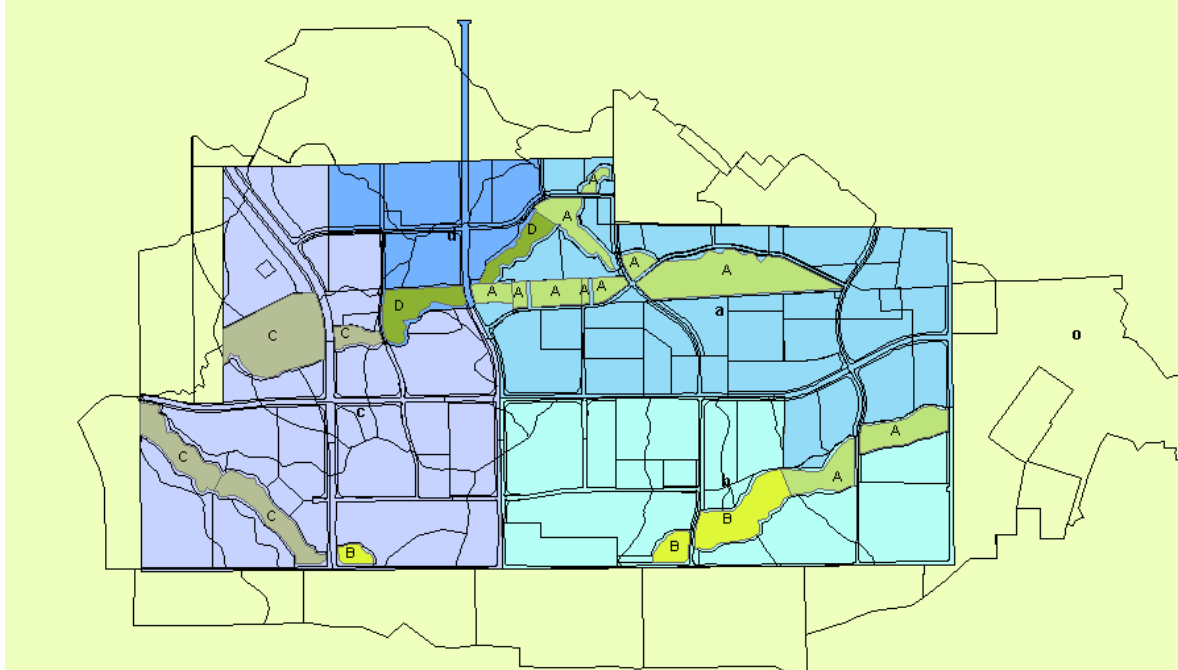
Peak Flow Mitigation: Four independent plans of analysis were developed, one for each of the major phases included in the plan. Plans were developed such that each analysis included development only within the phase under consideration. Then portions of the open space and detention mitigation areas were added until it could be demonstrated that peak flows were not increased for the 10-year event, at the downstream phase boundary, and downstream of the project. Where possible, mitigation areas within the phase or adjacent to the phase were included with each phase. Phase B includes a small portion of mitigation within Phase C, because trunk storm drain facilities will ultimately need to outfall into that location, stormwater quality improvements will be necessary and additional attenuation was needed above the amount contained within or adjacent to the phase. Figure III.C.2 shows the mitigation areas which will be required for each phase.

TABLE III.C.2 – PROPOSED PEAK FLOW MITIGATION VOLUME BY PHASE

PHASE	Area of Phase (ac)	Mitigation Volume Provided (Acre feet)
A	501.5	30.4
B	399.1	11.4
C	586.2	52.8
D	145.9	13.3

Table II.C.2 summarizes the amount of peak flow mitigation volumes proposed to be constructed by phase.

Figure III.C.2



As phases are initiated, follow-up impact and mitigation calculations will be needed to demonstrate that the proposed mitigation amounts will not result in an increase in peak flows for the post project. It is likely that early phases will have to perform more mitigation than the pro-rata share, and that later phases will have to do less, as the efficiencies of mitigation are reduced on a smaller scale.

IV. Water Quality:

The Sierra Vista Specific Plan project intends to install improvements which will comply with the City of Roseville's Stormwater Quality criteria. The City of Roseville is a Phase II community and has developed, jointly with the regions Phase I communities, "Stormwater Quality Design Manual for the Sacramento and South Placer Regions"(SQDM). This document has been adopted by the City of Roseville on July 18, 2007.

The State is beginning to draft new requirements to the Storm Water Management permits. At the time that Sierra Vista develops it is anticipated that the NPDES permit will include requirements that hydrograph modification impacts be addressed to some level". The project drainage study includes a section addressing the potential hydrograph modification impacts of this project, and the net impacts of the project with the mitigations proposed.

IV.A Stormwater Management During Construction Activities

The release of on-site stormwater runoff during Construction activities is regulated by the State General Construction Permit issued by the Regional Water Quality Control Board for all construction sites greater than one acre. The General Construction permit requires that a Storm Water Pollution Prevention Plan (SWPPP) is created to address how the storm water from a particular construction site will be maintained and treated prior to being discarded from the site. The SWPPP is an evolving document that changes with the dynamics of the site development.

The use of Best Management Practices (BMPs) during the construction process will generally incorporate erosion controls and sediment controls. Erosion and sediment control BMPs include such things as applying straw mulch to disturbed areas, the use of fiber rolls and silt fences, sedimentation basins, drain inlet protection, stabilized construction accesses, and material management. The final sizing and selection of BMPs will consider requirements specific to the Curry Creek watershed and proposed developed activities.

A Stormwater Pollution Prevention Plan (SWPPP) will be required to describe the BMPs which will be used to prevent erosion and to clean site discharge waters before entering State Waters. A permit with the Central Valley Regional Water Quality Control Board of the State of California will be obtained for the proposed construction activities. If construction occurs during the wet season, additional winterization improvements will be required to stabilize the disturbed areas of the site, prevent erosion, and clean discharge waters. All construction related BMP improvements must comply with the "NPDES General Permit for Storm Water Discharges Associated with Construction Activities, NPDES No. CAS000002, Order No. 99-08DWQ".

When the project reaches the point in processing at which the City is reviewing plan documents, a final Master Drainage Plan will be required which updates this plan for the final planned improvements. In addition, a detailed analysis of the proposed permanent and construction activities Best Management Practices shall be included either in the Final Master Drainage Plan, or as a separate "Water Quality BMP Plan". The BMP Plan shall identify expected pollutants, the expected activities, the effectiveness of the proposed BMPs and the maintenance plans for the BMP improvements.

IV.B Post Construction Stormwater Management

Post construction stormwater management is intended to treat the urban runoff generated on-site in perpetuity. The BMP techniques within the plan area will reduce and/or eliminate the pollutants from the urban stormwater runoff and prevent the contamination of receiving waters. Sierra Vista will work with the then current permit criteria applicable at the time of development and in conformance with the City of Roseville Improvement Standards, the City's Stormwater Quality Design Manual, the Placer County Flood Control Agency's Stormwater Management Manual, the open space preserve Operations and Maintenance (O&M) Plan, to design and address post construction stormwater treatment.

Post construction stormwater treatment is composed of three general elements: source control, runoff reduction and treatment of runoff. All three elements will be used in the Sierra Vista SWMP. The basic practice of source control is to minimize the potential for constituents to enter runoff at the source. The tool the project will employ towards the goal of runoff reduction, is the use of Low Impact Development(LID) measures. Implementation of LID includes the construction of decentralized small scale improvements that provide for local infiltration and treatment opportunities that reduce the quantity of runoff which enters the storm drain systems during a rainfall event. LID will be implemented to offset for runoff increases that occur with the development as a matter of the conversion of native ground surfaces to impervious cover. Additional Treatment control BMPs will be located at the end of the pipe and provide further treatment of the stormwater before it enters into the natural creek system.

The final selection of Best Management Practices (BMPs) shall consider requirements specific to the Curry Creek watershed. Other best management practices will involve prompt re-vegetation of disturbed areas, and proper erosion protection per the NPDES permit during construction.

IV.B Low Impact Development Measures

Low impact development (LID) is an approach to stormwater management that emphasizes the use of small-scale, natural, constructed and proprietary drainage features integrated throughout the city to capture urban runoff and precipitation. LID measures can slow, clean, infiltrate and evapotranspire runoff, which reduce the quantity of urban runoff entering the city storm drain systems. The added opportunities for infiltration offered by the use of LID can add water to local aquifers, increasing water reuse. It is a sustainable practice that benefits water quality protection, stream stability and can contribute to water supply. The intent is to weave the textures of natural processes into the fabric of development. Unlike traditional storm water management, which collects and conveys storm water runoff through storm drains, pipes, or other conveyances to a centralized storm water facility, LID within Sierra Vista will take a different approach by using site design elements, LID and storm water management to minimize changes to the site's pre-development runoff rates and volumes. Sierra Vista's LID elements will assist with the goal of mimicking the site's predevelopment hydrology by using design techniques that infiltrate, filter, store, evaporate, and detain runoff close to the source of rainfall.

Key principles of low impact development include:

- Decentralize and manage urban runoff to integrate storm water management throughout the watershed
- Preserve the ecosystem's natural hydrological functions and cycles.
- Account for a site's topographic features in its design.
- Reduce directly connected impervious surfaces to slow runoff and provide additional infiltration opportunities.
- Reduce impervious ground cover and maximize infiltration on-site.

It is also likely that the State Water Resources Control Board will adopt a new General Construction Permit before this project starts construction. Part of that permit addresses requirements for the use of permanent LID measures to mitigate runoff volume increases from development for the 85th percentile runoff event. The currently anticipated guidelines of the permit are presented at http://www.waterboards.ca.gov/water_issues/programs/stormwater/constpermits.shtml.

The "Stormwater Quality Design Manual for the Sacramento and South Placer Regions" (SWDM), adopted by the City of Roseville, also specifies criteria for the design and analysis of LID measures for use in the City of Roseville. Computation methods are very similar for this manual as those presented in the Draft construction permit, however, the Draft construction permit computational methodology generates slightly lower volume reductions from some individual measures, and offers computational criteria for a greater diversity of LID measures, and therefore has been used for the LID calculations included with this project. The SVSP will have to meet the design guidelines specified in the SWDM, and it is very likely that the project

will also have to meet the guidelines of the Draft General Permit. The Appendix 4_1 Post-construction LID worksheets from the Draft General Permit #2 are used herein to estimate

IV.B.1 Volume Reductions from LID measures:

LID measures provide treatment opportunities at or near the source of the runoff, and can substantially reduce the amount of treatment required. The Specific Plan will incorporate the use of LID throughout the Plan development areas, and herein prescribes certain minimum Required Volume Reductions (RVR) of LID performance for each land use as shown in Table IV.B.1. The amount listed in TABLE IV.B.1 are minimum thresholds required for each outfall location throughout the project, and the achievement of higher RVR through the use of LID measures is encouraged if possible. The values indicated in TABLE IV.B.1 represent the maximum amount of LID which can be applied to all product types expected for each of the land use types shown below, as determined by the project working group.

TABLE IV.B.1 – LID Required Volume Reductions (RVR)

Land Use Type:	LID 85th Percentile Event Volume Reduction from all measures except Vegetated Swales
Low Density Residential (LDR)	80.5%
Medium Density Residential(MDR)	78.6%
High Density Residential (HDR)	70.8%
Commercial	74.2%
Park	100%
Public/Quasi Public	81.6%
Roadway	71.4%

Additional project design elements within the open space areas will also provide hydrograph modification benefits. The created wetland elements will provide additional floodplain storage capacity which is factored into the project hydrology analysis. The created wetlands also provide LID and treatment potential which has not been factored into the project mitigation, which include: added infiltration opportunities, evapotranspiration opportunities, nutrient uptake, biological filtering, and stream buffers.

Examples of LID Measures which may be used in this project are described in TABLE IV.B.2. This is only a partial list of the types of measures which may be selected.

TABLE IV.B.2 – Applicable LID Measures By Development Type:

LID Measure Descriptions	Benefits Description	Development Land Use Type which is applicable to LID Measure
Disconnected roof drains	Water running off of the impervious roof system is treated by biological filtration, and the runoff gains an opportunity to partially infiltrate.	Low Density Residential Medium Density Residential High Density Residential Commercial, Public/Quasi Public, Parks
* Pervious or partially paved driveways & Porous pavement areas, and soil confinement	Pavement alternatives offer the opportunity for partial or complete infiltration of runoff.	Low Density Residential Medium Density Residential High Density Residential Commercial, Public/Quasi Public, Park Roadway
Separated sidewalks & Pavement Disconnection and eliminated pavement	Runoff from the impervious sidewalk, driveway, and pavement areas can be treated and infiltrated in landscape areas before entering the gutter pan and storm drain systems. (including residential walkways) In some areas of the development, unnecessary pavement may also be eliminated for stormwater benefit.	Low Density Residential Medium Density Residential High Density Residential Commercial, Public/Quasi Public, Park Roadway
Tree Planting and Canopy Preservation	The creation and preservation of tree canopy reduces the rate and amount of total runoff which enters the storm drain systems.	Low Density Residential Medium Density Residential High Density Residential Commercial, Public/Quasi Public, Park Roadway
Soil amendments in landscaped areas and Storm water planters.	The addition of organic material to impervious soils can add voids which can absorb runoff preventing it from entering storm drain systems. In residential areas, this may include amending a landscape strip adjacent to the street or pavement areas where large amounts of runoff can be intercepte from the lots. In commercial areas this is likely to be limited to stormwater planter areas. At roadways this will be used where roadway flows are diverted into the landscape areas.	Low Density Residential Medium Density Residential High Density Residential Commercial, Public/Quasi Public, Park Roadway
Stream Buffer	Sheet flows can be discharged into the stream corridors (at the surface overbank) directly providing significant treatment and infiltration opportunity prior to entering the streams.	High Density Residential ** Commercial ** Park Public/Quasi Public **
Vegetated Swales	*** Discharge of runoff into vegetated swales provides additional treatment in the in the treatment train, and opportunities for additional infiltration of runoff waters	Required at all storm drain outlet locations.
Stormwater Retention	These measures remove stormwater from the system, and trap constituents at the stormwater retention location such that it is not discharged.	Not currently anticipated within this plan area, however, with appropriate supporting documentation could be used in individual projects to achieve the RVR criteria.

* The use of pervious pavement and other infiltration oriented paving systems are dependant on infiltration capacity of the underlying soils, and may not be used everywhere. Geotechnical investigations are necessary to support the use of these systems. ** Opportunities for the use of this measure and land use combination are extremely limited within the Specific Plan. *** Because infiltration potential of this measure is not directly computable without geotechnical investigations, this measure is not applicable in this plan towards the RVR criteria, however, this element is required at all storm drainage outfall locations to make up the shortfall of the RVR to the 100% criteria of the "Attachment F - New and Re-development Performance Standard".

TABLE IV.B.3 demonstrates an alternative for the quantity of individual LID measures needed to obtain each land uses RVR standard for this Specific Plan, using the Attachment F - New and Re-development Performance Standard worksheets (per http://www.waterboards.ca.gov/water_issues/programs/stormwater/constpermits.shtml). Individual projects are not required to use these alternative measures and quantities exactly, and can change the selected measures by providing computations for alternate selections meeting the RVR's established for this Specific Plan. It will be required with the submittal of design plans and specifications that calculations supporting the achievement of the minimum RVR will be submitted for each outfall of the Specific Plan. Attachment F worksheets supporting the RVR's for sample development areas of each type are provided in Appendix H.

TABLE IV.B.3 – Alternative LID Use to Achieve Minimum RVR

Land Use Type	Disconnected roof drains	Pervious or partially paved driveways & Porous pavement areas, and soil confinement	Separated sidewalks, Pavement Disconnection, and eliminated impervious paving areas (including sidewalks)	Tree Planting and Canopy Preservation ***	Soil amendments in landscaped areas & Planters (est. acreage of amendments per 100 acres of dev.)	Stream Buffer	Vegetated Swales	Required Volume Reduction (RVR) ****
LDR	* 95%	-	8%	3 per lot	1.5 ac.	-	**	80.5%
MDR	* 80%	-	8%	1 per lot	1.5 ac.	-	**	78.6%
HDR	* 50%	-	15%	1 per unit	1.5 ac.	-	**	70.8%
COM	* 50%	-	15%	20 per acre	2.0 ac.	-	**	74.2%
PARK	* 50%	-	20%	5 per acre	0.5 ac.	10%	**	100%
PQP	* 50%	-	20%	10 per acre	1.5 ac.	-	**	81.6%
ROAD	-	-	50%	10 per acre	1.0 ac.	-	**	71.4%

* Disconnected Roof Drains will likely be implemented 100% for developments that use this measure, however some development product types are incompatible with this measure, and post construction adjustments to these systems do sometimes occur. For these reasons, the project wide average assumed usage of this measure has been reduced to the amount shown. ** Vegetated Swales will be used at each outfall and the minimum design will be per the width and length needed to generate a RVR of 100% for each outfall location per Appendix 4 criteria. *** Tree planting is assumed to be 50% evergreen and 50% deciduous in this example. **** Extensive LID practices are proposed in lieu of of Structural BMP Treatment Requirement in City Standards, and Structural BMP's will not be provided at/near storm drain outfall locations with this project.

IV.B.2 End of Pipe Treatment:

The schematic Post-Project Drainage Systems Map is shown in Exhibits SD-1. The maps identify storm drain outfall locations. Treatment Facilities (BMPs) will be required upstream of discharge to Curry Creek or any other Regulated Water of the State such as wetlands. Based on the plan shown in SD-1, treatment consisting of a section of Vegetated Swale will be the most common form of outfall BMP. The minimum design length of Vegetated grassy swale will be computed as the minimum length needed to achieve an RVR of 100%. However, this design is only intended to be used where minimal length exists to construct a vegetated swale outlet. In the instance where the minimum length of swale is constructed, a supplemental treatment BMP will have to be used in combination with the swale to achieve 100% treatment per the City's requirements, assuming LID is ineffective.

The optimum design length of Vegetated grassy swale and the associated calculations are provided in TABLE IV.B.1. The design lengths represented in this table assume complete treatment via the Vegetated Grassy Swale, as if LID and source control measures were ineffective.

To comply with the requirements of the local Mosquito/Vector Abatement District, all BMP's will be required to be designed to discharge all waters within 96-hours of the completion of runoff from a storm event. All graded areas must drain so that no standing water could accumulate for more than 96-hours within water quality facilities.

TABLE IV.B.1 – BMP SIZING AT OUTFALLS

NON-REDUCED - SWQ VOLUME/FLOWRATE DESIGN - PRSCG FACTORS											
Outfall Location	Total Area	Total % Imperv	C Value	Volume Ft³	Reserve Volume Ft³	Total Volume Ft³	Design Flowrate cfs	Min. Swale Length ft	Min. Swale Width ft	Velocity (fps)	Slope ft/ft
AA	73.7	25.62%	0.20	38837	1942	40779	2.97	117	30.9	0.28	0.01
B	19.5	2.78%	0.06	3110	155	3265	0.24	98	2.1	0.23	0.01
BB	18.0	55.61%	0.38	17678	884	18562	1.35	114	13.9	0.27	0.01
CC	88.2	35.54%	0.26	58831	2942	61772	4.50	144	38.3	0.34	0.015
DD	3.8	50.00%	0.34	3346	167	3513	0.26	99	2.3	0.24	0.01
GG	6.5	42.06%	0.29	4960	248	5208	0.38	104	3.6	0.25	0.01
HB	101.7	36.08%	0.26	68620	3431	72051	5.25	145	44.7	0.34	0.015
HA	29.9	49.40%	0.34	26241	1312	27553	2.01	116	20.8	0.28	0.01
HH	18.1	56.35%	0.38	18075	904	18978	1.38	115	14.2	0.27	0.01
JJ	141.0	51.72%	0.35	129118	6456	135574	9.88	168	73.1	0.40	0.02
L	88.4	49.20%	0.33	77210	3861	81071	5.91	167	43.6	0.40	0.02
LL	81.4	14.40%	0.14	29337	1467	30804	2.24	116	23.3	0.28	0.01
MA	20.9	25.79%	0.20	11075	554	11629	0.85	112	8.6	0.27	0.01
MB	53.6	32.05%	0.24	33072	1654	34725	2.53	117	26.3	0.28	0.01
MC	3.7	49.11%	0.33	3201	160	3362	0.24	98	2.1	0.23	0.01
MD	11.8	46.29%	0.32	9729	486	10216	0.74	111	7.5	0.26	0.01
ME	8.3	42.50%	0.29	6347	317	6665	0.49	107	4.7	0.25	0.01
MF	49.0	37.70%	0.27	34181	1709	35890	2.62	117	27.2	0.28	0.01
MG	8.0	40.12%	0.28	5879	294	6173	0.45	106	4.4	0.25	0.01
MH	5.0	2.08%	0.06	727	36	764	0.06	72	0.0	0.17	0.01
MM	4.2	39.74%	0.28	3075	154	3228	0.24	98	2.0	0.23	0.01
NA	36.7	30.58%	0.23	21886	1094	22981	1.67	115	17.3	0.27	0.01
NB	4.8	15.41%	0.14	1818	91	1909	0.14	89	1.0	0.21	0.01
NC	11.7	45.05%	0.31	9479	474	9953	0.73	110	7.3	0.26	0.01
NE	1.9	49.20%	0.33	1656	83	1739	0.13	87	0.8	0.21	0.01
NF	5.6	49.75%	0.34	4936	247	5183	0.38	104	3.6	0.25	0.01
NG	4.5	49.99%	0.34	3959	198	4157	0.30	101	2.8	0.24	0.01
NH	7.4	49.56%	0.34	6534	327	6861	0.50	107	4.9	0.25	0.01
NI	2.7	40.97%	0.29	1976	99	2075	0.15	91	1.1	0.22	0.01
NJ	2.7	49.63%	0.34	2364	118	2482	0.18	93	1.4	0.22	0.01
NN	5.7	51.41%	0.35	5191	260	5450	0.40	104	3.8	0.25	0.01
OO	6.9	49.53%	0.34	6069	303	6372	0.46	106	4.5	0.25	0.01
P	37.3	7.15%	0.09	8933	447	9379	0.68	110	6.8	0.26	0.01
PP	19.6	57.00%	0.39	19761	988	20749	1.51	115	15.6	0.27	0.01
Q	63.3	50.05%	0.34	56173	2809	58981	4.30	144	36.6	0.34	0.015
S	41.5	5.01%	0.08	8348	417	8766	0.64	109	6.4	0.26	0.01
SS	7.6	39.61%	0.28	5532	277	5809	0.42	105	4.1	0.25	0.01
TA	38.0	30.01%	0.23	22369	1118	23487	1.71	115	17.7	0.27	0.01
TB	3.3	48.98%	0.33	2854	143	2997	0.22	96	1.9	0.23	0.01
TC	7.0	49.25%	0.33	6162	308	6470	0.47	106	4.6	0.25	0.01
TT	15.4	41.96%	0.29	11693	585	12278	0.89	112	9.1	0.27	0.01
U	45.2	46.64%	0.32	37641	1882	39523	2.88	117	30.0	0.28	0.01
W	62.8	53.27%	0.36	59158	2958	62116	4.53	144	38.5	0.34	0.015
X	66.4	37.57%	0.27	46167	2308	48476	3.53	144	30.0	0.34	0.015
Z	20.2	59.70%	0.41	21470	1073	22543	1.64	115	16.9	0.27	0.01

TABLE IV.B.2 – MINIMUM VEGETATED SWALE SIZING AT OUTFALLS

REDUCED for LID - SWQ VOLUME/FLOWRATE DESIGN - PRSCG FACTORS													
Outfall Location	Total Area ac	Imperv. Area ac	LID Mit. Imp. Area ac	Reduced % Imperv	C Value	Volume Ft ³	Reserve Volume Ft ³	Total Volume Ft ³	Design Flowrate cfs	Min. Swale Length ft	Min. Swale B. Width ft	Velocity < 1 fps Check	Design Slope ft/ft
AA	73.7	18.9	14.3	6.19%	0.09	16408	820	17229	1.26	50	30.9	0.12	0.010
B	19.5	0.5	0.1	2.11%	0.06	2857	143	3000	0.22	90	2.1	0.21	0.010
BB	18.0	10.0	7.1	16.31%	0.15	7001	350	7351	0.54	45	13.9	0.11	0.010
CC	88.2	31.4	23.9	8.50%	0.10	23229	1161	24391	1.78	57	38.3	0.14	0.015
DD	3.8	1.9	1.5	10.70%	0.11	1134	57	1190	0.09	33	2.3	0.08	0.010
GG	6.5	2.7	2.0	11.72%	0.12	2066	103	2170	0.16	43	3.6	0.10	0.010
HB	101.7	36.7	27.9	8.65%	0.10	27032	1352	28384	2.07	57	44.7	0.14	0.010
HA	29.9	14.8	11.6	10.54%	0.11	8910	445	9355	0.68	39	20.8	0.09	0.010
HH	18.1	10.2	7.2	16.54%	0.15	7125	356	7481	0.55	45	14.2	0.11	0.010
JJ	141.0	72.9	54.5	13.08%	0.13	47843	2392	50235	3.66	62	73.1	0.15	0.020
L	88.4	43.5	33.2	11.67%	0.12	27969	1398	29367	2.14	60	43.6	0.14	0.015
LL	81.4	11.7	8.0	4.60%	0.07	15762	788	16550	1.21	63	23.3	0.15	0.010
MA	20.9	5.4	4.3	5.05%	0.08	4224	211	4435	0.32	43	8.6	0.10	0.010
MB	53.6	17.2	12.9	7.95%	0.10	13588	679	14268	1.04	48	26.3	0.11	0.010
MC	3.7	1.8	1.4	10.42%	0.11	1086	54	1140	0.08	33	2.1	0.08	0.010
MD	11.8	5.4	4.3	9.92%	0.11	3383	169	3552	0.26	38	7.5	0.09	0.010
ME	8.3	3.5	2.8	9.18%	0.11	2270	114	2384	0.17	38	4.7	0.09	0.010
MF	49.0	18.5	14.7	7.76%	0.10	12267	613	12880	0.94	42	27.2	0.10	0.010
MG	8.0	3.2	2.6	7.41%	0.09	1958	98	2056	0.15	35	4.4	0.08	0.010
MH	5.0	0.1	0.0	1.95%	0.05	715	36	751	0.05	71	0.0	0.17	0.010
MM	4.2	1.7	1.4	7.31%	0.09	1025	51	1076	0.08	33	2.0	0.08	0.010
NA	36.7	11.2	8.2	8.22%	0.10	9475	474	9948	0.72	50	17.3	0.12	0.010
NB	4.8	0.7	0.5	4.25%	0.07	905	45	950	0.07	44	1.0	0.11	0.010
NC	11.7	5.3	4.2	9.47%	0.11	3284	164	3449	0.25	38	7.3	0.09	0.010
NE	1.9	0.9	0.7	10.50%	0.11	563	28	592	0.04	30	0.8	0.07	0.010
NF	5.6	2.8	2.2	10.64%	0.11	1675	84	1758	0.13	35	3.6	0.08	0.010
NG	4.5	2.2	1.8	10.70%	0.11	1341	67	1408	0.10	34	2.8	0.08	0.010
NH	7.4	3.7	2.9	10.58%	0.11	2218	111	2328	0.17	36	4.9	0.09	0.010
NI	2.7	1.1	0.9	8.69%	0.10	707	35	742	0.05	32	1.1	0.08	0.010
NJ	2.7	1.3	1.0	10.61%	0.11	802	40	842	0.06	32	1.4	0.08	0.010
NN	5.7	2.9	2.1	15.21%	0.14	2126	106	2233	0.16	43	3.8	0.10	0.010
OO	6.9	3.4	2.7	10.59%	0.11	2061	103	2164	0.16	36	4.5	0.09	0.010
P	37.3	2.7	1.7	2.72%	0.06	5893	295	6188	0.45	72	6.8	0.17	0.010
PP	19.6	11.1	7.9	16.69%	0.15	7740	387	8127	0.59	45	15.6	0.11	0.010
Q	63.3	31.7	24.4	11.48%	0.12	19826	991	20817	1.52	51	36.6	0.12	0.015
S	41.5	2.1	1.1	2.42%	0.06	6323	316	6639	0.48	83	6.4	0.20	0.010
SS	7.6	3.0	2.5	7.28%	0.09	1844	92	1937	0.14	35	4.1	0.08	0.010
TA	38.0	11.4	8.8	6.95%	0.09	8967	448	9416	0.69	46	17.7	0.11	0.010
TB	3.3	1.6	1.3	10.46%	0.11	972	49	1021	0.07	33	1.9	0.08	0.010
TC	7.0	3.5	2.7	10.45%	0.11	2088	104	2193	0.16	36	4.6	0.09	0.010
TT	15.4	6.5	5.1	9.12%	0.10	4213	211	4423	0.32	40	9.1	0.10	0.010
U	45.2	21.1	15.8	11.69%	0.12	14325	716	15041	1.10	45	30.0	0.11	0.010
W	62.8	33.4	25.5	12.56%	0.13	20769	1038	21808	1.59	51	38.5	0.12	0.015
X	66.4	24.9	17.3	11.42%	0.12	20728	1036	21764	1.59	64	30.0	0.15	0.010
Z	20.2	12.1	8.5	17.37%	0.16	8206	410	8616	0.63	44	16.9	0.10	0.010

* Vegetated Swales built to this minimum length complete the RVR to 100% for LID, but do not complete the total treatment requirements of the project, and additional BMP's will be required to obtain the treatment objectives.

IV.C Hydrograph Modification Benefits from LID:

The development of the Specific Plan has the potential to modify the hydrologic response for given storm recurrences within the Curry Creek Watershed. It is commonly understood that development generally increases runoff volume and peak flows by increasing the amount of impervious areas within the watershed and by reducing the amount of time over which runoff occurs.

Streams naturally migrate and evolve. Migration is the progression of the stream meanders in a downstream direction. Migration can naturally occur over short time periods or long geological periods, depending on stream factors such as soil types, etc... Evolution is the modification of the stream type or classification, into another stream type or classification. Evolution naturally occurs in streams over long geological periods. Modifications to the hydrologic response of a watershed has the potential in some watersheds to result in downstream modifications to the stream, referred to as an evolutionary response. An evolutionary response can result in considerable additional sediment load within a stream corridor, and ultimately can result in the degradation of water quality, environmental resources, recreational resources, and navigable resources.

The MS4 permit of the City of Roseville requires new projects to mitigate stormwater quality impacts to the "Maximum Extent Practicable". In response, this project has reviewed the potential for hydrograph modification, resulting from the proposed development measures and has incorporated mitigation measures which will reduce the potential as follows:

- The project will incorporate the use of extensive Low Impact Development measures to reduce runoff impacts at the source of runoff from impervious surfaces.
- The project has developed a peak flow mitigation plan within the creek corridors which will provide increased offsetting attenuation for all storm events from 10% of the peak 2-year to the 100-year event.

IV.C.2 Peak Flow Response:

In figure IV.C.1 we have plotted peak flow responses at the project exit to Curry Creek, for the Pre-Project and Post-Project Mitigated with LID scenarios. The response for the following events is compared in the graph: 10% of 2-year, 50% of 2-year, 2-year, 10-year, 100-year. The graph demonstrates that peak flow increases from the project area will not occur and are fully mitigated for the full range of events, by the proposed project mitigation plan.

FIGURE IV.C.1 – Peak Flowrate Comparisons Leaving Project

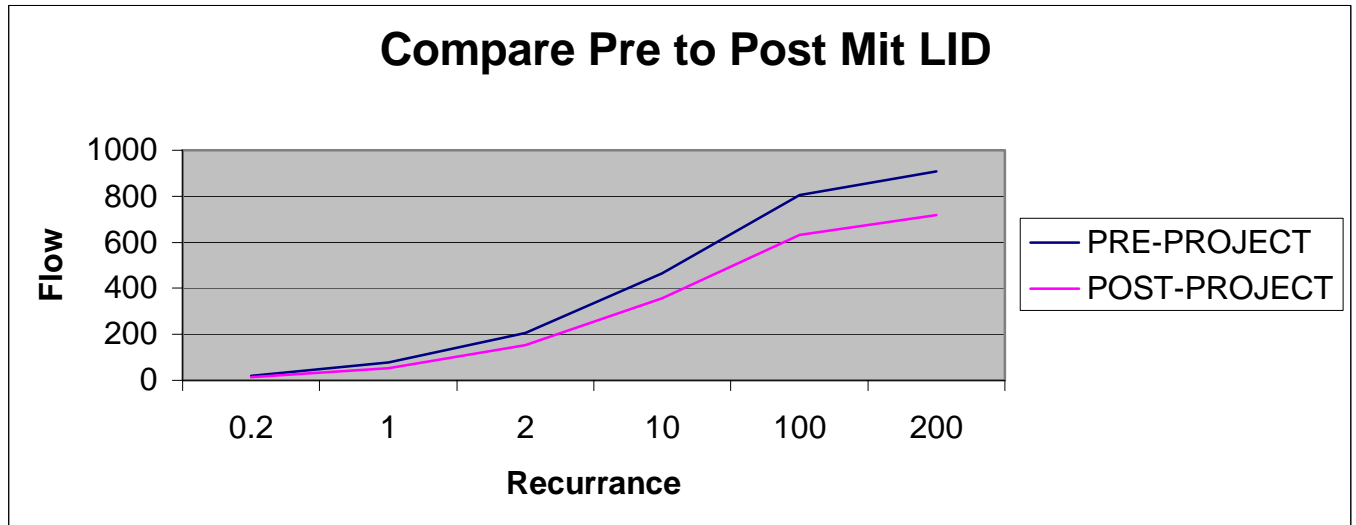
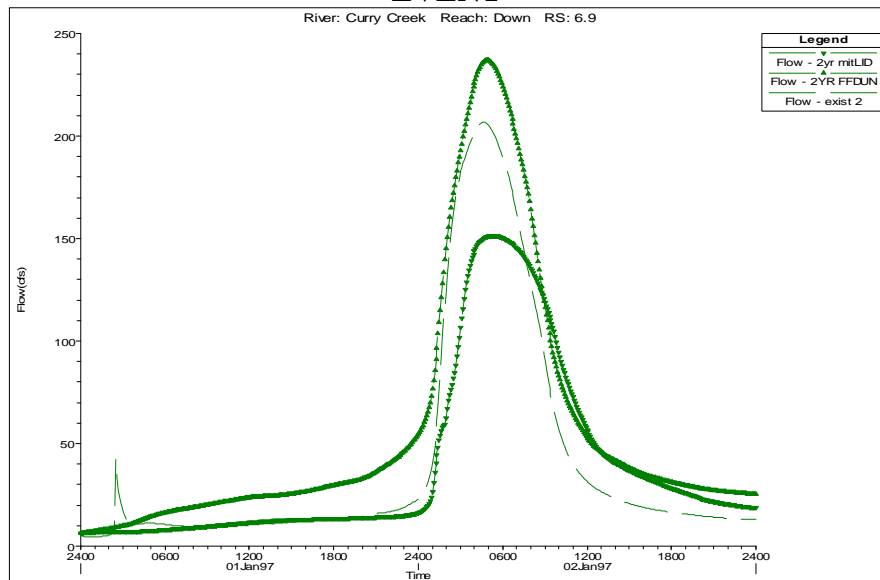


Figure IV.C.2 – plots the anticipated comparison of the resulting hydrographs at the project exit point for the 2-year design event. It is important to note that the mitigated with LID hydrograph is similar in overall timing and demonstrates a reduction in peak flow rate. The Future, Fully Developed, without mitigation hydrograph demonstrates an accelerated timing in the 2-year event, and a substantial increase in both peak flow and volume, which is summarized in TABLE IV.C.1.

FIGURE IV.C.2 – Comparison of Pre-project and Post-project Hydrographs 2-year EVENT



III.C.3 Hydrograph Characteristics:**TABLE IV.C.3: HYDROGRAPH CHARACTERISTICS AT PROJECT DISCHARGE**

Event	48-hour Runoff Volume (AF)			Peak Flow (CFS)		
	PRE-	POST MIT LID	UN-MIT	PRE-	POST MIT LID	UN-MIT
200-YEAR	648	597	712	907	720	882
100-YEAR	600	544	663	804	632	811
10-YEAR	411	374	437	466	357	472
2-YEAR	162	156	227	207	152	237
50% of 2-YEAR	82	86	118	77	54	91
10% of 2-YEAR	28	38	40	19	13	23

III.C.4 Hydrograph “Flow Duration”:

The duration of flow of certain flow rates over time will change the sediment load requirements of the channel. Traditionally, “Flow Duration analysis” involves preparing some continuous simulation models for the alternative project conditions, and running 30-years of gage records through the simulation, and compiling and comparing the results of flow exceedance verses time. Similarly we can use design events to chart the relative impact of the project. To do this, the hydrographs for the various project scenarios and design events are measured to determine the duration of exceedance of certain flow rates. For this analysis we selected flow rates of:

<u>Flow Rates for Flow Duration Analysis</u>	<u>Recurrence Events Studied</u>
Baseflow + 5 cfs	10% of the 2-year
Baseflow + 10 cfs	50% of the 2-year
Baseflow + 25 cfs	2-year
Baseflow + 50 cfs	10-year
Baseflow + 100 cfs	100-year
Baseflow + 150 cfs	200 -year
Baseflow + 200 cfs	
Baseflow + 300 cfs	
Baseflow + 400 cfs	
Baseflow + 500 cfs	
Baseflow + 600 cfs	
Baseflow + 700 cfs	
Baseflow + 800 cfs	

Data was assembled for comparison of the Pre-project conditions, Post-project conditions with mitigation and LID, and Post-project unmitigated conditions. Flows are compared at the exit point of the project downstream of the confluence of the Federico Tributary and the main branch of Curry Creek. Figure III.C.4 shows the comparison of these scenarios for flow duration where a log scale is used on the duration axis. The Duration Axis plots the total summation of duration for which flowrates are exceeded for the analyzed events for a normalized 100-year period.

Figure III.C.4 – Flow-Duration Relative Comparison (logrythmic)

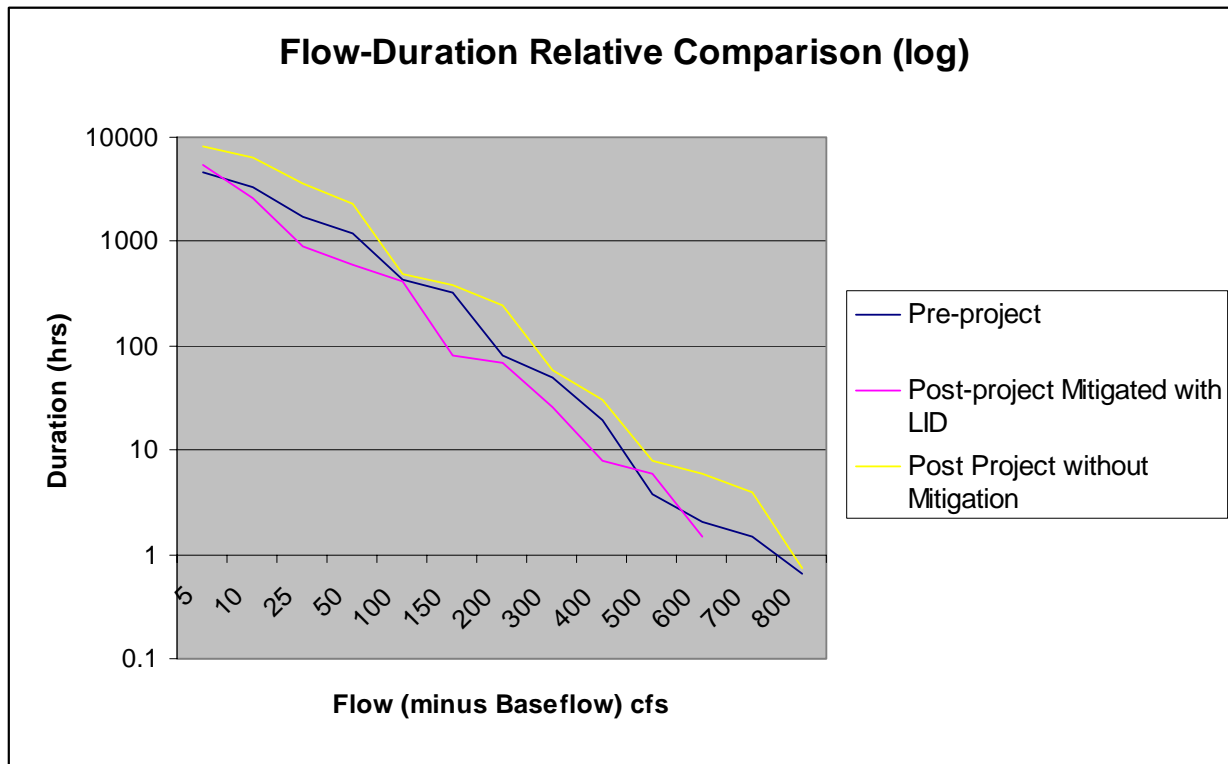
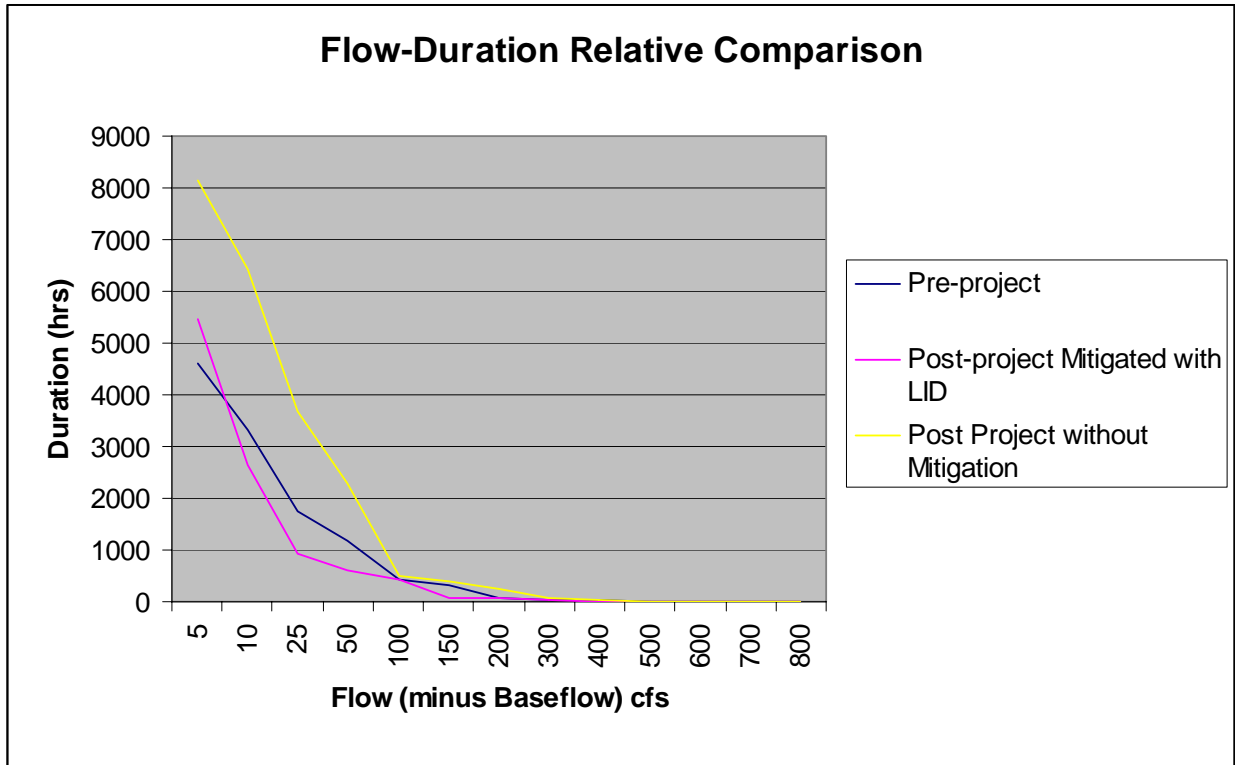


Figure III.C.4 shows that the project with LID and mitigation is expected to lessen or equal the flow duration amounts from the pre-project for all flows, while the project without mitigation would increase flow duration for all flows.

Figure III.C.5 shows the same correlation but on a non-logrythmic scale. On this figure it is easy to see the substantial relative impacts for the smaller and more frequently occurring flow rates, that the project without mitigation would have. The project with mitigation and LID demonstrates a much better relative correlation for the full array of flow rates expected at the exit point of the project.

Figure III.C.5 – Flow Duration Relative Comparison (non logrhythmic)



Note: the above charts plot relative impacts for the storm events analyzed in this document. Adding storm event frequencies to this analysis would make the curves smoother (less jagged), but would not alter the results of the relative comparisons demonstrated.

V. Volumetric Impacts:

The Sierra Vista Specific Plan drains via Curry Creek. Runoff from the project areas ultimately has to pass through the Natomas Cross Canal before entering the Sacramento River. The Cross Canal Watershed Study (CH2MHILL 1992-1994) identified that development within these watershed could make a flooding problem that exists within Sutter County, worse, by increasing runoff volumes. The City of Roseville has implemented a drainage fee to collect funds and to ultimately build a mitigation facility, currently planned at the Reason Farms site. The Pleasant Grove Watershed Mitigation Fee recently established the parameters shown in Table V.A for development in Type D soils.

TABLE V.A – VOLUMETRIC IMPACT RATES IN TYPE D SOILS

LAND USE TYPE	LAND USE DESCRIPTION	% IMPERVIOUS	TYPE "D" SOIL IMPACT RATE (AF/ACRE)	LAND USE AREA (ACRE)	LAND USE IMPACT (ACRE FT)
LDR	Low Density Residential	40	0.072	836.1	60.20
MDR	Medium Density Residential	50	0.126	279.8	35.25
HDR	High Density Residential	60	0.206	66.6	13.72
COMM	Commercial	70	0.233	228.8	53.31
Park	Park	5	-0.115	154.7	-17.79
PQP	Public/Quasi Public	50	0.126	103.4	13.03
ROAD	Roadways	85	0.313	124.4	38.94
UR	Urban Reserve	2	0	638.9	0.00
OS	Open Space	2	0	256.2	0.00
				TOTAL=	196.66

The total computed impact of this development is 196.7 acre feet of volumetric storage for a 8-day 100-year event. This impact will be mitigated at the City of Roseville's Reason Farms Facility, once it is constructed. The Pleasant Grove Mitigation Fee did consider the Sierra Vista Specific Plan as a possible participant in the Reason Farms Project (identified as an MOU area in that study), and sufficient storage volume was shown to be available for this project in the preliminary studies.

APPENDICES

APPENDIX A CREATED WETLANDS NARATIVE

**SIERRA VISTA SPECIFIC PLAN
ON-SITE WETLANDS CREATION COMPONENT OF
WETLAND MITIGATION PLAN**

**GIBSON AND SKORDAL
CIVIL ENGINEERING SOLUTIONS, INC.**

June 15, 2009

Mitigation Plan Overview. The wetland mitigation plan for the Sierra Vista Specific Plan will consist of several different components including off-site wetlands preservation, off-site wetlands restoration/creation and on-site wetlands creation. Mitigation for impacts to wetlands that are considered habitat for listed branchiopods will consist of off-site preservation of existing habitat and restoration of previously existing habitat at ratios determined through consultation with the U.S. Fish and Wildlife Service (the "Service"). Which vernal pools and seasonal wetland swales, or portions thereof, are considered to be habitat for listed branchiopods will be determined through consultation with the Service. All, or a portion thereof, of the impacts to those wetlands that are not considered to be habitat for listed branchiopods will be mitigated through creation of wetlands on-site within the proposed open space corridors. The on-site creation will not provide for creation of vernal pools not considered to be habitat for listed branchiopods. In the event that the Corps determines that mitigation for impacts to these vernal pools must be in-kind, that will be accomplished off-site.

On-site Mitigation Design. The following objectives and/or criteria were considered in designing the proposed on-site wetland mitigation plan:

- Maximize the area of wetlands to be created after consideration of physical and logistical constraints;
- Design wetlands that maximize watershed support functions such as flood attenuation functions, surface water storage functions, water quality improvement functions and habitat connectivity;
- Design wetlands that will provide a diversity of habitats including short and long term inundation seasonal wetlands, emergent marshes and riparian scrub.;
- Design wetlands that are hydrologically interconnected to the existing watercourses;
- Design wetlands that promote the long-term stability of the existing watercourses in consideration of the geofluvial morphology of these watercourses;
- Design wetlands that minimize the potential to exacerbate vector breeding conditions;
- Site wetlands so that they have adequate upland buffers separating them from the proposed development; and
- Site wetlands so that they are down gradient, and not within the proposed development's water quality treatment features such as bioswales.

Using these design criteria, Gibson & Skordal conceptually mapped the location and extent of wetlands to be created on-site. Following this conceptual mapping, it was reviewed by both MacKay & Somps and Civil Solutions. MacKay & Somps reviewed the plan with respect to its relation to the overall land plan. Civil Solutions reviewed the plan with respect to flood water attenuation, conveyance issues and quantified the hydrologic issues and benefits associated with the construction of the wetland features. Additionally, Civil Solutions evaluated preliminary stream stability guidance criteria to be used in their design. Based on their input, the plan was further refined. An overall plan drawing of the Specific Plan Area showing the conceptual layout of the wetlands to be created is attached. More detailed plan showing an example representative portion of the proposed wetland creation types is also attached.

A total of approximately 45 acres (\pm 2-3 acres) of wetlands will be constructed. The wetlands to be created will consist of a depression excavated in the overbank adjacent to existing watercourses but separated by a berm of native land. The wetlands to be created are situated along the inside edge of existing meanders and/or along relatively straight reaches. The depressional wetlands will be connected to the adjacent watercourses by openings through the berms of native land both on their up-gradient and down-gradient ends. The up-gradient openings (diversion weir) will have an invert elevation that is located above the active stream channel. Where the wetland to be created is not located down-gradient of a water quality treatment feature, the down-gradient openings will have an invert elevation that is 0.5-1.0 foot above the bottom elevation of the wetland. Where the wetland to be created is located down-gradient of a water quality treatment facility, the invert of the down-gradient opening will be at approximately the same elevation as the wetland. The openings will be protected from erosion by the use of a

vegetated/hydroseeded geotextile fabric rather than structural armoring. The interior slopes adjacent to the wetlands will be graded to approximately 5:1 or greater except where limited by proximity to the adjacent watercourse.

The wetlands will be constructed during the dry season when surface water is not present except possibly along certain reaches of Curry Creek. Where surface water is present throughout the summer along certain reaches of Curry Creek, the wetlands will be constructed “in the dry” by first completing the wetland grading for the wetland, then the down-gradient opening and finally the up-gradient opening. In constructing the wetlands, the first 4-6 inches of top soil will be salvaged and stockpiled. The wetland will then be excavated and graded to an elevation approximately 4-6 inches below design depth. The salvaged topsoil will then be placed to final grade. Once grading is completed, the slopes of the wetland will be hydro-seeded with a mixture of upland and wetland grasses and forbs. To minimize erosion, it may also be desirable to sprinkler irrigate the constructed wetlands and side slopes to promote establishment of a vegetative cover prior to the onset of the rainy season.

Related Issues. The following is a discussion of specific issues raised by the City of Roseville staff regarding the proposed on-site wetlands creation.

Vector Control:

The hydrology characteristics of the wetlands to be created are important considerations in designing wetlands that will not exacerbate vector control issues. Both presence or absence of ponding versus saturation as well as the timing of inundation are important factors to consider. The following design criteria were used to assure that the wetlands to be constructed on-site would minimize the potential for creating vector control problems.

- Depressional wetlands that will be subject to ponding will be situated so that they do not receive surface runoff from developed areas. These wetlands will experience ponding only from during storm events. This will assure that ponding only occurs during the winter and early spring and that ponding will not be present during the late spring and summer when vector breeding is problematic.
- Where constructed wetlands will be situated down gradient of surface water runoff from developed areas, they will be designed so that they are sloped with planed surfaces. While these wetlands will receive runoff from irrigation and other (treated) nuisance runoff during the spring and summer, they will not have depressions that will create ponding conditions. In these cases, the wetland hydrology will be dependent on soil saturation and shallow sheet flow conditions.

Channel Stability:

The wetlands will be designed to minimize impacts to the existing low flow stream systems. The natural banks will be left in tact, and a minimum berm width of 5 feet horizontal will be maintained between the stream bank and the created wetland excavation top of slope. Weirs intended to divert waters in flood events from the main low flow channel into the wetland features, will be constructed at an elevation above the ordinary (bank full) flow channel, and will be armored using a synthetic geotextile fabric and hydroseeded so that vegetation will establish across the weir. Additionally, low areas of the separation berms will be armored to prevent scour at these natural weir locations, where additional 1st breakouts of flood events into the floodplain and wetland areas could occur. The remainder of the berm will not be armored as high velocity breakout overflows will not likely occur at these locations until equilibrium with the overbank and main channel elevations has occurred.

At the downstream end of the wetlands, a return channel will be cut through the berm, or a pipe return will be constructed through the berm permitting flows to return to the main channel of the creek. Soft armoring of the return channels with a synthetic geotextile fabric will be provided to maintain grade control of the outlet of the wetland area, and stream bank at these locations. Where pipe returns are built, individual analysis of pipe velocity will dictate the need for armoring at the pipe exit into the creek. A cutoff wall will be provided at the pipe trench to prevent seepage erosion and piping failures at these locations.

The design of the wetlands is intended to provide minimal connectivity of the stream waters to the wetlands except during flood events, which maintains the active stream channel conditions intact. The active stream channel zone is the critical area of the stream with respect to fluvial geomorphology processes. Minimizing impacts to the flows, sediment load, and stream dynamics within this zone will minimize adverse changes to the main stem stream stability, from their current levels.

[The above section will be modified after the Initial review by the geomorphologist.]

Siltation within Constructed Wetlands:

The construction of the wetlands will add substantial conveyance area to the stream overbank flow paths. Additionally, within these wetland areas, dense vegetation is likely to grow. The combination of these items indicates that slower velocities in these zones of the creek are likely to occur in the built conditions. Slower velocity flow will have less energy and less sediment carrying capabilities. It is likely that sediment discharged by the stream into these areas, will be deposited within the wetland areas.

Hydraulic analysis of the wetland areas accounts for this potential in several ways. Increased friction coefficients ('n'-values) are used in these areas to account for the increased vegetation densities expected. Also, in the "Future-Fully Developed,

Unmitigated” flood scenarios (which are used to establish design floodplain parameters), the wetlands are assumed to be fully obstructed to their existing ground elevations, by sediment and vegetation.

In the hydrologic analysis of the project, it was determined that 20% of these wetland volumes could be filled before the project would no longer meet the peak flow mitigation requirements (reduced post project peak 100-year flow rate by 10% of the difference between the pre-project and post-project unmitigated flow rates for the 2-year, 10-year and 100-year design storm events). These streams do not currently have the amount of sediment loading within them to generate this volume of sediment deposition. It is also likely that sediment loading will be reduced in the ultimate buildout and vegetated conditions of the project. However construction activities are known to increase sediment loading in the creeks, and therefore the wetlands should be inspected following project construction to verify that sediment accumulation has not exceeded the 20% threshold.

Finally, the sediment accumulations pose a minimal threat to the long-term maintenance of the constructed wetlands. As is readily demonstrated by examining existing channels, swales, depressional seasonal wetlands and vernal pools within the Specific Plan Area, sediment accumulation under baseline conditions is minimal. Given this and the relatively low sediment loading, it is reasonable to assume that the amount of sediment deposition in the constructed wetlands will also be relatively minor, certainly not of a magnitude that would undermine the success of the constructed wetlands. Sediment loading is normally highest, during the first couple of years following wetland construction due to construction activities and associated ground disturbance in adjacent development areas. If this were to occur at a level likely to undermine the success of the constructed wetlands it would be detected during the initial mitigation construction monitoring. In which case, remedial measures would be implemented to correct any such problems at that time.

Long Term Maintenance:

Following the construction activities related to the development projects, sediment loading within the creek corridors would be expected to stabilize at current amounts or lower amounts. It is not expected that repairs from normal flood event depositions would ever exceed the 20% threshold necessary to maintain the peak flood mitigation objectives of the project. During project design, a detailed geomorphic analysis will be performed to provide analysis of specific velocities and soils conditions, and design guidance which will lead to a stable stream and floodplain, ready to accept the development flows, and existing flows in a more stable manner than currently exists.

All stream low flow channels naturally migrate and evolve over time. It is likely that these activities will continue within this project, and the project and open space designs are intended to accommodate this by setting back the development areas outside of the meander amplitude of the stream. These meandering activities of the stream may pose some risk to the success of the wetland creation areas, However, this risk is no greater

than other natural locations where wetlands develop adjacent to streams. [This discussion will be revised after we have the initial geomorphology investigations complete]

Pre-Project Stream Photos:



Curry Creek Main Reach – Upstream of Baseline Road, Looking Upstream



Curry Creek Main Reach – Upstream of Baseline Road, Looking Upstream



Curry Creek Main Branch Upstream of Baseline Road



Curry Creek Main Branch, at Culvert Plunge Pool,



Curry Creek Main Branch – Incised Bank Example



Curry Creek Main Branch – Straightened Reach.



Curry Creek Main Branch – Example of a Dry Season wet stretch of channel. Also demonstrates the up and down nature of the channel invert.

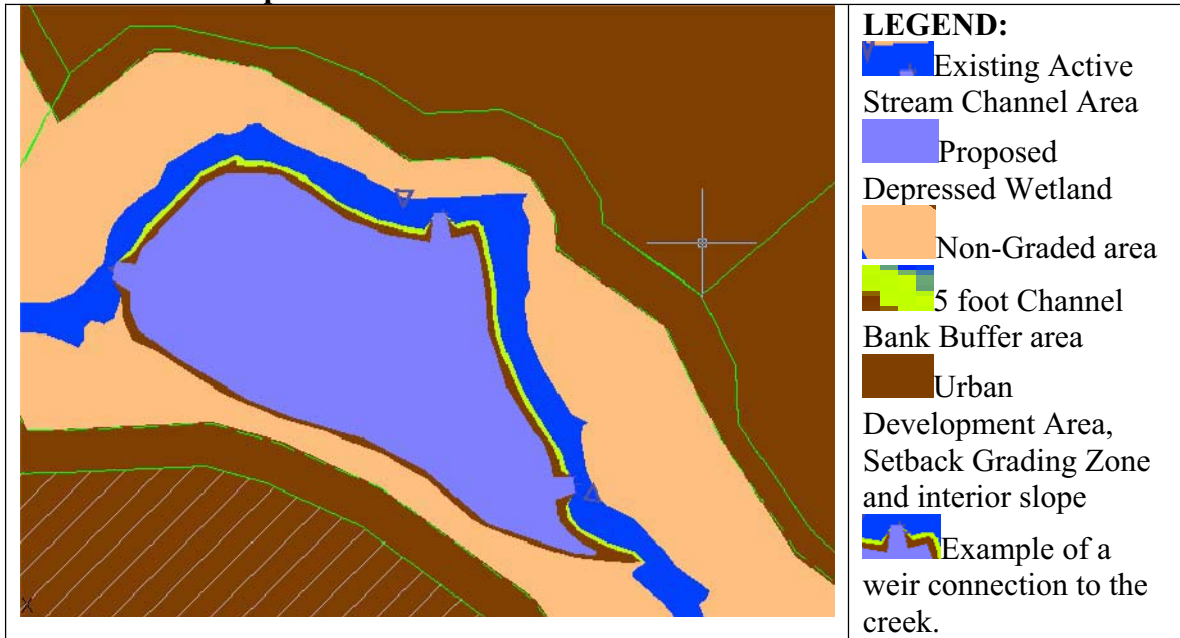


Curry Creek Main Branch – Example of Stream Bed Side materials at hardpan.



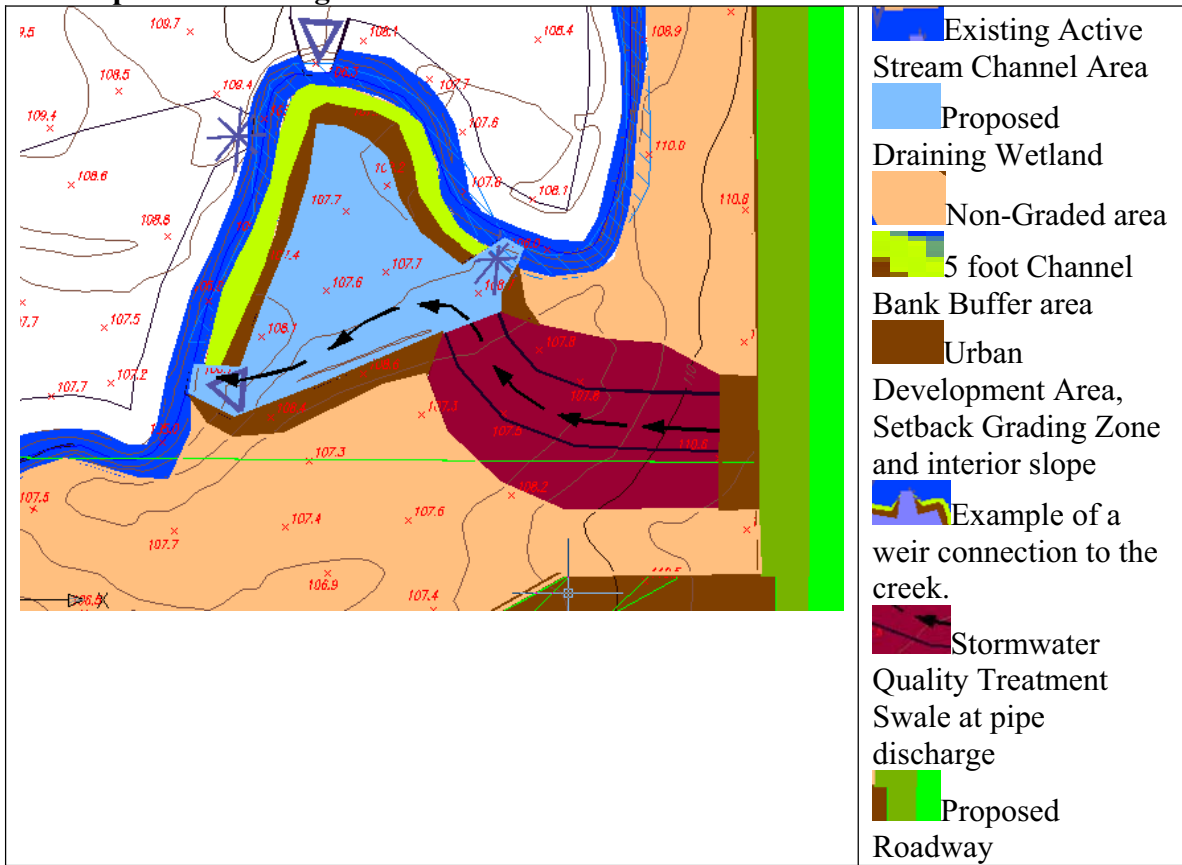
Westpark Outfall Channel at the Federico Tributary.

Plan View of a Depressed Wetland Creation Area:



The above example demonstrates the preliminary schematic example for the construction of a depressed wetland feature. The feature would not be directly connected to a development discharge and would not be anticipated to receive nuisance waters. Flooding of the depressed wetland would only occur from flood event waters, and would not be expected to be retained into the dry season.

Plan View Example of a Draining Wetland Creation Area, downstream of a Development Discharge Location:



The above example of a Draining Wetland Creation Feature demonstrates that wetland creation areas subject to nuisance waters from development discharge points will be constructed to include positive drainage through the wetland feature and into the creek to reduce vector potential. By draining through the wetland feature, additional opportunities for water quality enhancement, infiltration and evapotranspiration of the nuisance runoff will occur, and these flows will help to maintain soil saturation in support of the wetland vegetation.

APPENDIX B

HEC-1 PRE-PROJECT INPUT FILE

```

ID CURRY CREEK - Southwest Roseville Specific Plan
ID pre-Development Conditions Job# 2005-18
ID June 16, 2005
ID HEC-1 File SWRSPex1.in
ID Shed CU2A is subdivided into several small sheds
* ID CURRY CREEK - De La Sal University
* ID Pre-Development model; Fiddeyment Rd. to Cross Canal
* ID Civil Solutions
* ID Rev 8/11/03 GSU
* ID FILE NAME: CDSE.DAT Curry ck DelaSal-Existing, Modified sheds; CU2A, CU2B &
* CU3 to reflect detailed analysis within the DE LE SAL project boundaries.
* The current models do not reflect the existing agricultural modifications
* made to the drainage patterns.
* *****
* FILE NAME: CUDSE.DAT, CUrry ck DelaSal-Existing Rev 8/01/03 GSU
* Sheds CUS1 thru CUS17C are from the Placer Vineyards post-mitigated model
* (CUS-MIT.DAT) by Civil Solutions. Sheds CUS1 thru CUS14 have replaced Shed
* CU1 and sheds CUS15 thru CUS17C replace a portion of Shed CU2A in the West-
* park model (WPDET1.DAT) by Wood-Rodgers. The proposed improvements for
* Placer Vineyards and Westpark are included in this existing condition model.
* *****The following is from file: CUS-MIT.DAT *****
* ID Placer Vineyards - Curry Creek Tributary
* ID Pre-development model : on-site CUS - Curry Creek
* ID TSP - Civil Solutions
* ID Post-development Mitigated
* ID April 2003
* THIS FILE PREPARED USING THE PLACER COUNTY HEC-1 Pre-Processor
* A Civil Solutions Product
*
* CUS in Shed Names Ommited for some nodes
* ***** June 16,05 *****
* ***** Southwest Roseville Apecific Plan *****
* ***** Project No. 2005-18 *****
* ***** HEC-1 File Copied from Project 2003-24 File:CDSE.IN *****
* *****HEC-1 File:SWRSPex1.IN Existing conditions *****
*
* ***** April 28,06 Watt Ave Extension File Wattex1.in *****
* ***** new file copied from SWRSPex1.in *****
* ***** Sheds cus15, cu2a6, cu2a4, cu2a5, cu2b2 and cu2b1 are subdivided **
* !!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!
* !!!!!!!!!!! MAY 17, 06 PULS ROUTING USED FOR CHANNELS PER RAS !!!!!!!!!!!!!!!
* !!!!!!!!!!! NEW FILE : SVEXT1.IN COPIED FROM WATTEXL.IN !!!!!!!!!!!!!!!
*
* ##### MAY 8, 08 EXISTING CONDITION #####
* ##### SET UP FOR UNSTEADY RAS MODEL #####
* ##### NEW FILE : SVEXT2.IN COPIED FROM SVEXT1.IN #####
*
* #####
* ***** March 5, 09 Existing Model Updated Per New offsite drainage info
* ***** Updated CUS3G, CWP7 & CWP8 shed delineation per existing stormdrain
* ***** system from City of Roseville *****
* ***** NEW FILE : SVEXT3.IN COPIED FROM SVEXT2.IN *****
*
*
* *DISPLAY
*DIAGRAM
IT      5 01JAN97      0      600
IO      5      0      0
* The following sheds are from the PV model
* Shed CUS1 is part of shed CUS3G, by DNM 3/5/09
* KK CUS1
* ZW C=FLOW
* KM CUS1 43.7ac
* KM CUS1 58.789 AC
* BF -1
* BA.06828
* BA.09186
* PB
* * PI
* LU 0.1 0.07 2
* UK 200 0.005 0.4 100
* RD 1600 0.0050 0.040 TRAP 2.0 10
*
* Shed CUS2 is part of shed CUS3G, by DNM 3/5/09
* KK CUS2
* ZW C=FLOW
* KM CUS2 139.8ac
* KM CUS2 147.418 AC
* BF -1
* BA.21844
* BA.23034
* PB
* * PI

```

* LU 0.1 0.07 2
 * UK 200 0.005 0.4 100
 * RD 1800 0.0050 0.040 TRAP 2.0 10 YES

* Changed to 281.3 from 58.88 to include sheds CUS1 & CUS2
 * Changed by DNM 3/5/09

KK CUS3G
 ZW C=FLOW
 KM AREA EAST OF FIDDYMENT ROAD
 KM AREA= 281.3 AC

BF -1
 BA.43953

PB
 * PI
 LU 0.1 0.07 2 0.1 0.12 41
 UK 150 0.01 0.4 1
 UK 200 0.005 0.24 99
 RD 3000 .001 0.015 CIRC 3
 RD 2300 .0005 0.015 CIRC 5

* KKCCUS3G
 * ZW C=FLOW
 * KM COMBINE ALL FLOW FROM SHEDS EAST OF FIDDYMENT ROAD
 * HC 2

* ***** MAY 17,06 PULS ROUTING PER RAS *****

KK CUS2R
 ZW C=FLOW
 KM STORAGE ROUTING PER RAS
 RS 6 FLOW -1
 SV 0 4.57 8.15 17.05 24.42 30.95 37.10 40.86
 SQ 0 50 100 250 400 550 700 800

* KK CUS3 21

ZW C=FLOW
 * KO 3
 * KM CUS3 310.4ac
 KM CUS3 AREA (300.3AC-58.9=241.4) REVISED PER NEW TOPO ON JUNE 17,05

BF -1
 * BA.48500

BA.37719
 PB
 * PI
 LU 0.10 0.0713 2 0.10 0.1200 90
 UK 200 0.0050 0.400 96.78
 UK 150 0.0100 0.240 3.22
 RD 2500 0.0050 0.040 TRAP 2.0 10
 * RD 3300 0.0050 0.040 TRAP 2.0 10 YES
 RD 3300 0.0050 0.040 TRAP 2.0 10

* KK CUS3C
 ZW C=FLOW
 KM COMBINE FLOW
 HC 2
 * ***** MAY 17,06 PULS ROUTING PER RAS

* KK CUS3R
 ZW C=FLOW
 KM ROUTE FLOW THROUGH SHED CUS4
 RS 3 FLOW -1
 SV 0 2.12 3.63 10.37 18.04 20.35 22.37 23.70
 SQ 0 50 100 250 400 550 700 800

* KK CUS4
 ZW C=FLOW
 * KM CUS4 76.5ac
 KM CSU4 AREA (39.1 AC) REVISED PER NEW TOPO ON JUNE 17,05

BF -1
 * BA.11953

BA.06109
 PB
 * PI
 LU 0.10 0.0717 2
 UK 200 0.0050 0.400 100
 RD 400 0.0050 0.040 TRAP 2.0 10
 * RD 1000 0.0050 0.040 TRAP 2.0 10 YES
 RD 1000 0.0050 0.040 TRAP 2.0 10

* KK CUS5A
 ZW C=FLOW
 * KM CUS5A 31.6ac
 KM CUS5A 33.924 AC MAY 18,06
 BF -1

* BA.04938

BA.05301

PB

* PI

LU	0.10	0.0700	2	0.10	0.1200	90		
UK	200	0.0050	0.400	96.83				
UK	50	0.0100	0.240	3.17				
RD	300	0.0050	0.040		TRAP	2.0	10	
RD	1000	0.0050	0.040		TRAP	2.0	10	

*

KK CUS5

ZW C=FLOW

* KM CUS5 36.2ac

KM CUS5 AREA (34.9AC) REVISED PER NEW TOPO ON JUNE 17,05

BF -1

* BA.05656

BA.05453

PB

* PI

LU	0.10	0.0737	2	0.10	0.1200	90		
UK	200	0.0050	0.400	90.33				
UK	150	0.0100	0.240	9.67				
RD	2400	0.0050	0.040		TRAP	2.0	10	
RD	500	0.0050	0.040		TRAP	2.0	10	YES

*

KKCCUS5C

KM COMBINE FLOW FROM CUS4, CUS5A & CUS5

ZW C=FLOW

HC 2

*

KK CUS5C

ZW C=FLOW

HC 2

*

KK CUS5B

ZW C=FLOW

KM CUS5B 92.5ac

* KM CUS5B 93.889 AC

BF -1

BA.14453

* BA.14670

PB

* PI

LU	0.10	0.0700	2	0.10	0.1172	60.6		
UK	200	0.0050	0.400	12.43				
UK	50	0.0100	0.240	87.57				
RD	1800	0.0050	0.040		TRAP	2.0	10	
RD	1000	0.0050	0.040		TRAP	2.0	10	

*

KKCUS5BC

ZW C=FLOW

KM COMBINE FLOW

* HC 3

HC 2

*

*

* ***** MAY 17,06 PULS ROUTING PER RAS

KK SUS5R

ZW C=FLOW

KM ROUTE FLOW TO BASELINE ROAD

RS	5	FLOW	-1					
SV	0	2.21	5.37	27.75	41.15	42.72	43.82	44.50
SQ	0	50	100	250	400	550	700	800

*

KK CUS6

ZW C=FLOW

KM CUS6 118.6ac

KM CUS6 AREA (107.8 AC) REVISED PER NEW TOPO ON JUNE 17,05

BF -1

* BA.18531

BA.16844

PB

* PI

LU	0.10	0.0738	2	0.10	0.1200	90		
UK	200	0.0050	0.400	96.63				
UK	150	0.0100	0.240	3.37				
RD	2400	0.0050	0.040		TRAP	2.0	10	
* RD	1800	0.0050	0.040		TRAP	2.0	10	YES
RD	1800	0.0050	0.040		TRAP	2.0	10	

*

KK CUS6C

ZW C=FLOW

KM COMBINE FLOW UP/S OF BASELINE ROAD

HC 2

*

* ***** MAY 17,06 ROUTING REVISED
 * INSERT EXISTING PULS ROUTING UPSTREAM OF ROADWAY HERE!!!
 * KK CUS6D
 * ZW C=FLOW
 * KM PULS ROUTING UPSTREAM OF CULVERT
 * KO 1 2
 * RS 1 FLOW -1
 * SV 0 3.68 55.7 57.9 59.4 60.5 61.6 62.7 63.9
 * SQ 0 200 339 441 590 713 839 964 1180

*

* ***** MAY 17, 06 PULS ROUTING PER RAS

*

KKCUS5BR
 ZW C=FLOW
 KM ROUTE FLOW DN/S OF BASELINE ROAD THROUGH PLACER VINEYARDS
 RS 8 FLOW -1
 SV 0 6.0 10.54 21.68 30.28 37.68 44.58 48.92
 SQ 0 50 100 250 400 550 700 800

*

KKCUS11B
 ZW C=FLOW
 KM CUS11B 86.6ac
 BF -1
 BA.13531
 PB
 * PI
 LU 0.10 0.0748 2 0.10 0.1200 49.84
 UK 200 0.0050 0.400 23.78
 UK 50 0.0100 0.240 76.22
 RD 500 0.0050 0.040 TRAP 2.0 10
 * RD 150 0.0050 0.040 TRAP 2.0 10 YES
 RD 150 0.0050 0.040 TRAP 2.0 10

*

KKCUS12A
 ZW C=FLOW
 * KMCUS12A 165.2ac
 KM CUS12A AREA (158.00 AC) REVISED PER NEW TOPO ON JUNE 17,05
 BF -1
 * BA.25813
 BA.24688
 PB

*

* PI
 LU 0.10 0.0710 2 0.10 0.1200 90
 UK 200 0.0050 0.400 98
 UK 150 0.0100 0.240 2
 RD 3200 0.0050 0.040 TRAP 2.0 10
 RD 900 0.0050 0.040 TRAP 2.0 10

*

KKCS12AC
 KM COMBINE FLOW FROM CUS11B & CUS12A
 ZW C=FLOW
 HC 2

*

KKCS12ACC
 ZW C=FLOW
 HC 2

*

* ***** MAY 17, 06 PULS ROUTING PER RAS

*

KKCS12ACR
 ZW C=FLOW
 KM ROUTE FLOW TO BASELINE ROAD CULVERT
 RS 3 FLOW -1
 SV 0 1.82 4.14 10.69 18.67 33.44 46.30 48.62
 SQ 0 50 100 250 400 550 700 800

*

* ***** MAY 17, 06 ROUTING REVISED PER ABOVE

*

* INSERT UPSTREAM DETENTION BASIN
 * KKCUS12D
 * ZW C=FLOW
 * KM PULS ROUTING UPSTREAM OF CULVERT
 * KO 1 2
 * RS 1 FLOW -1
 * SV 0 20.6 23.5 26.1 30.2 34.1 37.5 58.5 67.7 80
 * SQ 0 202 230 252 282 305 326 461 800 12

*

KKCUS13A
 ZW C=FLOW
 * KM CUS13A 8.6ac

KM CUS13A AREA (12.3 AC) REVISED PER NEW TOPO ON JUNE 17,05

BF -1
* BA.01344

BA.01922

PB

* PI

LU	0.10	0.0700	2	0.10	0.1200	90			
UK	200	0.0050	0.400	95					
UK	150	0.0100	0.240	5					
RD	800	0.0050	0.040		TRAP	2.0	10		
RD	800	0.0050	0.040		TRAP	2.0	10		

*
* ***** MAY 17,06 MOVE KKS13ACC AFTER CUS14

* ZW C=FLOW

* KKS13ACC

* ZW C=FLOW

* HC 2

* D/S end CUI

*

KK CUS14

ZW C=FLOW

KMCUS14

BF -1

BA.09641

PB

* PI

LU	0.10	0.0760	2	0.10	0.1179	48.98			
UK	200	0.0050	0.400	17.02					
UK	50	0.0100	0.240	82.98					
RD	800	0.0050	0.040		TRAP	2.0	10		
* RD	900	0.0050	0.040		TRAP	2.0	10	YES	
RD	900	0.0050	0.040		TRAP	2.0	10		

KKCS13AC

KM COMBINE FLOW FROM CUS13A & CUS14

ZW C=FLOW

HC 2

*

KKS13ACC

ZW C=FLOW

HC 2

*

* ***** MAY 17, 06 PULS ROUTING PER RAS

KKS13ACR

ZW C=FLOW

KM ROUTE FLOW FROM BASELINE RD TO WATT AVE

RS	2	FLOW	-1						
SV	0	0.67	1.23	3.06	5.58	9.27	11.38	12.38	
SQ	0	50	100	250	400	550	700	800	

*

* ***** MAY 17, 06 ROUTING REVISED PER ABOVE

*

* INSERT DOWNSTREAM DETENTION BASIN!!!

* KKCUS16D

* ZW C=FLOW

* KM PULS ROUTING UPSTREAM OF CULVERT

* KO 1 2

* RS 1 FLOW -1

* From Channel

* SV	0	18.6	25.5	26.0	26.8	27.4	28.0	29.6	33.3	
* Modified for Detention Basin ??????										
* SV	0	17	24.6	28.5	31.5	35.0	38.0	41.0	44.0	50
* SQ	0	180	236	264	288	322	345	368	496	9

*

KKCUS16B

ZW C=FLOW

KMCUS16B

BF -1

BA.02156

PB

* PI

LU	0.10	0.0720	2	0.10	0.1158	80.47			
UK	200	0.0050	0.400	2.00					
UK	50	0.0100	0.240	98.00					
RD	900	0.0050	0.040		TRAP	2.0	10		
RD	900	0.0050	0.040		TRAP	2.0	10		

*

KKS16BCC

ZW C=FLOW

HC 2

*

* ***** APRIL 28, 06 WATT AVE EXTENSION *****

* ***** CUS15 IS DIVIDED INTO CUS15 & CUS15B *****

* KK CUS15
 * ZW C=FLOW
 * KM CUS15 AREA (124.2 AC) REVISED PER NEW TOPO ON JUNE 17,05
 * BF -1
 * BA.14891
 * BA.19406
 * PB
 * * PI
 * LU 0.10 0.0720 2 0.10 0.03 90
 * UK 200 0.0050 0.400 98.00
 * UK 50 0.0200 0.110 2.00
 * RD 2400 0.0050 0.040 TRAP 2.0 10
 * RD 1500 0.0050 0.040 TRAP 2.0 10 YES
 *

KK CUS15
 ZW C=FLOW
 KM CUS15 AREA =73.135 PER WATT AVE
 BF -1
 BA.11427
 PB

* PI
 LU 0.1 .072 2
 UK 300 0.01 0.40 100
 RD 2700 .008 0.04 TRAP 2 10
 *

KKCCUS15
 ZW C=FLOW
 KM COMBINE FLOW AT WATT AVE EXTENTION
 HC 2
 *

* KKRCUS15
 * ZW C=FLOW
 * KM ROUTE FLOW FROM WATT AVE TO DN/S
 * RD
 * RC 0.055 0.055 0.055 2600 0.002
 * RX 128 172 183 206 210 229 390 429
 * RY 86 85 84 80 80 84 85 86
 * ***** MAY 17, 06 PULS ROUTING PER RAS
 *

KKRCUS15
 ZW C=FLOW
 KM ROUTE FLOW DN/S
 RS 4 FLOW -1
 SV 0 2.17 3.69 9.34 14.20 18.27 21.98 24.30
 SQ 0 50 100 250 400 550 700 800
 *

KKCUS15B
 ZW C=FLOW
 KM CUS15B AREA=51.05 AC PER WATT AVE
 BF -1
 BA.07977
 PB

* PI
 LU 0.1 0.072 2
 UK 250 0.0167 0.4 100
 RD 1600 .0075 .040 TRAP 2 10
 *

KK ADD91
 ZW C=FLOW
 KM COMBINE FLOW
 HC 2
 *
 * ***** MAY 17, 06 PULS ROUTING PER RAS

KKADD91R
 ZW C=FLOW
 KM ROUTE FLOW TO CONFLUENCE WITH CURRY CK TRIB
 RS 2 FLOW -1
 SV 0 1.70 2.89 7.91 11.87 15.02 17.82 19.55
 SQ 0 50 100 250 400 550 700 800
 *

KKCUS17A
 ZW C=FLOW
 KMCUS17A
 BF -1
 BA.06000
 PB
 * PI
 LU 0.10 0.0700 2 0.10 0.1166 80.38
 UK 200 0.0050 0.400 2
 UK 50 0.0100 0.240 98
 RD 500 0.0050 0.040 TRAP 2.0 10
 RD 500 0.0050 0.040 TRAP 2.0 10
 *

* ***** MAY 17,06 MOVE S17ACC AFTER CUS17C

*

KKCUS17B

ZW C=FLOW

KMCUS17B AREA (43.8 AC) REVISED PER NEW TOPO ON JUNE 17,05

BF -1

* BA.10313

BA.06844

PB

* PI

LU	0.10	0.0741	2	0.10	0.1200	90			
UK	200	0.0050	0.400	97.00					
UK	150	0.0100	0.240	3.00					
RD	1300	0.0050	0.040		TRAP	2.0	10		
RD	2400	0.0050	0.040		TRAP	2.0	10	YES	

*

* KKS17ACC

* ZW C=FLOW

* HC 2

*

KKADD_92

ZW C=FLOW

HC 2

*

KKCUS17C

ZW C=FLOW

KMCUS17C AREA (76.4 AC) REVISED PER NEW TOPO ON JUNE 17,05

BF -1

* BA.15625

BA.11938

PB

* PI

LU	0.10	0.0761	2	0.10	0.1200	90			
UK	200	0.0050	0.400	99.00					
UK	50	0.0100	0.240	1.00					
RD	1300	0.0050	0.040		TRAP	2.0	10		
* RD	2400	0.0050	0.040		TRAP	2.0	10	YES	
RD	2400	0.0050	0.040		TRAP	2.0	10		

*

KKS17ACC

ZW C=FLOW

HC 2

*

KK S17BR

ZW C=FLOW

RD 2400 0.0050 0.040 TRAP 2.0 10

*

* END OF PLACER VINEYARDS SHEDS FOR CURRY CREEK * SHEDS FROM CUS-MIT.DAT

* *****

* The following data is from the Wood-Rodgers Westpark model WPDET1.DAT

* ID CROSS CANAL WATERSHED MODEL

* ID Existing (1989) Conditions

* ID Original by CH2M Hill 1991

* ID Modified by Placer County Flood Control District 1992

* ID - modified-puls routing changed to muskingum-cunge

* ID - storm centering PI and PB added

* ID 5-03 MODELS REVISED PER WESTPARK LANDUSE BY WOOD-RODGERS

* ID

* ID Landuse: Offsite-Existing; Westpark - Developed w/detention

* ID File Name: Wpdet1.dat

* ID

* *DIAGRAM

* IT 5 300

* IO 0

* CU1 removed & replaced by Placer Vineyards sheds CUS1 thru CUS14 * GSU 8-2003

* KK CU1 CURRY CREEK

* ZW C=FLOW

* BA 1.75

* PB

* * pi

* BF -1

* LU .1 .06 2

* UK 600 .015 .4 100

* RD 800 .03 .05 .10 TRAP 1 10

* RD 1500 .01 .05 .2 TRAP 5 10

* RD 7500 .005 .05 TRAP 5 10

*

* LH 0.05 0 1.5 1.4 0.1

* US3.0916 0.6

*

* ROUTE S17BR TO CU2A+

* KK RCH1 ROUTE FROM CU1 TO CU2A+

* ZW C=FLOW

```

* RS      1      FLOW      -1
* RD
* RC 0.055    0.055    0.055    14200    0.0014    1000
* RX      0      0      400      413      427      440      840      840
* RY 1020    1000      990    983.5    983.5      990    1000    1020

```

* Changed to 49.9 from 45.2 by DNM 3/5/09

* KK CWP7 (45.2 ac)

KK CWP7 (49.9 ac)

ZW C=FLOW

KM Curry Creek Basin CU2A (5-03)

BA.07797

PB

* pi

```

BF      -1
LU      .1      0.06      41      0.10      0.06      2
UK      150    0.020      .11      99
UK      300    0.020    0.350      1
RD      600    0.0020    0.024      .02      TRAP      1.0      0
RD      1800  0.0070    0.024      TRAP      4.0      1

```

* Possible Westpark Detention Basin Location

KK DB3

ZW C=FLOW

KM Det Basin C (1.6 acre surface area;1-18";1-24" outfalls)

```

* KO      1      2
RS      1      FLOW      -1
SV      0      1.5      3.1      4.6      6.3      7.9
SE      109    110      111      112      113      114
SQ      0      2      12      23      40      90

```

* remover RCH2 routing 8/11/03 gsu

* KK RCH2 ROUTE FROM WPA7 TO CU2A+

* ZW C=FLOW

```

* * RS      1      FLOW      -1
* * RD
* * RC 0.055    0.055    0.055    14000    0.0014    1000
* * RX      0      0      400      413      427      440      840      840
* * RY 1020    1000      990    983.5    983.5      990    1000    1020

```

* ***ON June 17,05 Shed CU2A is subdivided for SW Roseville Specific Plan *****

* ***** CU2A is subdivided into 8 Sheds *****

* ***** AUGUST 2, 05 *****

* ***** SHED CU2A1 IS SUBDIVIDED INTO CU2A1A, CU2A1B & CU2A1C *****

* KK CU2A1 (139.7 AC)

* ZW C=FLOW

KKCU2A1A

ZW C=FLOW

* KM Shed CU2A1 is the eastern portion of Shed CU2A

KM SHED AREA=42.4 AC

BF -1

* BA.21828

BA.06625

PB

* Pi

```

LU      0.10    .07      2
* UK      300    0.01    0.4      100
UK      280    0.010    0.4      100
RD      350    0.014    0.05      TRAP      5      10
* RD      4200  0.0054    0.05      TRAP      10      10
RD      1350  0.0096    0.055      TRAP      10      10

```

KK RR_1

ZW C=FLOW

KM ROUTE FLOW THROUGH SHED CU2A1B

```

RD      1300    .0046    0.055      TRAP      10      10
* RC 0.055    0.055    0.055    1300    0.0046
* RX      0      160      290      400      600      650      750      880
* RY      110    107      104      101      101      104      108      110

```

KKCU2A1B

ZW C=FLOW

KM SHED AREA=51.6

BF -1

BA.08063

PB

* PI

```

LU      0.10    0.07      2
UK      250    0.010    0.4      100
RD      350    0.011    0.05      TRAP      5      10
RD      1560  0.0071    0.055      TRAP      10      10

```

```

KK ADD_1
ZW C=FLOW
KM COMBINE FLOW FROM CU2A1A & CU2A1B
HC      2
*
KK RR_2
ZW C=FLOW
KM ROUTE FLOW THROUGH SHED CU2A1C
RD 1150 .0017 0.055          TRAP      10      10
* RC 0.055 0.055 0.055 1150 0.0017
* RX 0 160 290 400 600 650 750 880
* RY 110 107 104 101 101 104 108 110
*
KKCU2A1C
ZW C=FLOW
KM SHED AREA=46.0 AC
BF -1
BA.07188
PB
* PI
LU 0.10 0.07 2
UK 300 0.010 0.4 100
RD 650 0.011 0.05 TRAP 5 10
RD 2000 0.006 0.055 TRAP 10 10
*
* Changed 03/05/09 sicne label used already
KKCC2A1C
ZW C=FLOW
KM COMBINE FLOW WITH FROM(CU2A1A+CU2A1B)+CU2A1C WITH FLOW FROM C-WPY
* HC 2
HC 3
*
*
KKCU2A1R
ZW C=FLOW
KM ROUTE FLOW THROUGH SHED CU2A9
KM X-SECTION BASED ON NEW TOPO
RD
RC 0.055 0.055 0.055 1920 0.0047
RX 0 160 290 400 600 650 750 880
RY 110 107 104 101 101 104 108 110
*
* Changed to 63.5 from 64.9 by DNM 3/5/09
* KK CWP8 (64.9 ac)
KK CWP8 (63.5AC)
ZW C=FLOW
KM Basin CU2A (5-03)
BF -1
BA.09922
PB
* pi
BF -1
LU .1 0.07 40 .1 0.06 2
UK 150 0.0200 0.11 99
UK 300 0.0200 0.350 1
RD 610 0.0070 0.024 .02 TRAP 1.0 0
RD 2800 0.0020 0.024 TRAP 4.0 1
*
* Proposed Westpark Detention Basin Location
*
KK DB4
ZW C=FLOW
KM Det Basin B for WP8 (1.6 acre surface area;1-18";1-24" outfalls)
* KO 1 2
RS 1 FLOW -1
SV 0 1.5 3.1 4.6 6.3 7.9
SE 103 104 105 106 107 108
SQ 0 2 12 23 40 90
*
* insert combine segment for DB3 & DB4 prior to routing thru CU2A+
* KK CDB34
ZW C=FLOW
* HC 2
*
KK CWP8R
ZW C=FLOW
KM ROUTE FLOW THROUGH SHED CU2A8
RD
RC 0.055 0.055 0.055 900 0.0091
RX 0 75 155 240 360 500 620 800
RY 106 104 102 100 100 102 104 106
*
KK CU2A9 (81.1 AC)

```

```

ZW C=FLOW
KM PORTION OF SHED CU2A
BF -1
BA.12672
PB
* Pi
LU 0.1 0.07 2
UK 200 0.010 0.4 100
RD 1200 0.01 0.05 TRAP 5 10
RD 1050 0.003 0.055 TRAP 10 10
*
KKCU2A9C
ZW C=FLOW
KM COMBINE FLOW FROM C-WP8 & CU2A2 WITH FLOW FROM (CU2A1+C-WP7)
HC 2
*
KK CU2A8 (39.6 AC)
ZW C=FLOW
KM PORTION OF SHED CU2A
BF -1
BA.06188
PB
* Pi
LU 0.1 0.07 2
UK 200 0.010 0.4 100
RD 1100 0.01 0.05 TRAP 5 10
RD 400 0.009 0.055 TRAP 10 10
*
KKCU2A2C
ZW C=FLOW
KM COMBINE FLOW FROM C-WP8 & CU2A2 WITH FLOW FROM (CU2A1+C-WP7)
HC 3
*
KKCU2A2R
ZW C=FLOW
KM ROUTE FLOW THROUGH SHED CU2A3
* KK RCH3 ROUTE FROM A8 TO CU2A+
* ZW C=FLOW
* RS 1 FLOW -1
RD
* RC 0.055 0.055 0.055 14000 0.0014 1000
* RX 0 0 400 413 427 440 840 840
* RY 1020 1000 990 983.5 983.5 990 1000 1020
* ***** ROUTE FLOW THROUGH SHED CU2A3 *****
* ***** X-SECTION BASED ON NEW TOPO *****
RC 0.055 0.055 0.055 2950 0.0017
RX 0 200 330 360 520 560 680 950
RY 105 98 93 91 91 93 97 105
*
KK CU2A3 (156.4 AC)
ZW C=FLOW
KM PORTION OF SHED CU2A
BF -1
BA.24438
PB
* Pi
LU 0.1 0.07 2
UK 300 0.01 0.4 100
RD 1700 0.013 0.05 TRAP 5 10
RD 800 0.0030 0.055 TRAP 10 10
*
KKCU2A3C
ZW C=FLOW
KM COMBINE FLOW
HC 2
*
* ***** MAY 18, 06 PULS ROUTING PER RAS *****
KKCU2A3R
ZW C=FLOW
KM ROUTE FLOW THROUGH SHED CU2A7
RS 3 FLOW -1
SV 0 1.33 2.67 5.06 7.13 8.83 10.41 11.86
SQ 0 50 100 200 300 400 500 600
*
* KKCU2A3R
* ZW C=FLOW
* KM ROUTE FLOW THROUHG SHED CU2A4
* RD
* RC 0.055 0.055 0.055 1100 0.0045
* RX 0 290 360 460 620 690 780 890
* RY 92 90 88 85 85 88 90 92
*
KK CU2A7 (106.5 AC)

```

ZW C=FLOW
 KM PORTION OF SHED CU2A
 BF -1
 BA.16641
 PB
 * Pi
 LU 0.1 0.07 2
 UK 200 0.02 0.4 100
 RD 700 0.01 0.05 TRAP 5 10
 RD 1800 0.0072 0.05 TRAP 10 10
 *

KKCU2A7C
 ZW C=FLOW
 KM COMBINE FLOW
 HC 2
 *

* ***** MAY 18, 06 PULS ROUTING PER RAS

KKCU2A7R
 ZW C=FLOW
 KM ROUTE FLOW THROUGH SHED CU2A4 TO WATT AVE
 RS 3 FLOW -1
 SV 0 0.92 1.65 3.03 5.54 6.83 8.35 9.90
 SQ 0 50 100 200 300 400 500 600
 *

* KKCU2A7R
 * ZW C=FLOW
 * KM ROUTE FLOW THROUGH SHED CU2A4
 * RD
 * RC 0.055 0.055 0.055 1700 0.003
 * RX 0 290 360 460 620 690 780 890
 * RY 92 90 88 86 86 88 90 92
 *

* ***** APRIL 28,06 WATT AVE EXTENSION *****

* CU2A4 DIVIDED INTO 3, CU2A4, CU2A4B & CU2A4C *****

* KK CU2A4 (81.40 AC)

* ZW C=FLOW

KK CU2A4

ZW C=FLOW

KM AREA=47.861 AC PER WATT AVE

* KM PORTION OF SHED CU2A

BF -1

* BA.12719

BA.07478

PB

* Pi

LU 0.1 0.07 2
 UK 600 0.004 0.4 100
 RD 1000 0.0096 0.050 TRAP 5 10
 RD 700 0.0030 0.055 TRAP 10 10
 *

KKCCU2A4

ZW C=FLOW

KM COMBINE FLOW AT WATT AVE

HC 2

*

* KKCU2A4C

* ZW C=FLOW

* KM AREA=7.464 AC PER WATT AVE

* BF -1

* BA.01166

* PB

* * PI

* LU 0.1 0.07 2
 * UK 200 0.025 0.4 100
 * RD 850 0.015 0.040 TRAP 2 10
 *

* KKCC2A4C

* ZW C=FLOW

* HC 2

*

* ***** MAY 18, 06 PULS ROUTING PER RAS

KKROUTE1

ZW C=FLOW

KM ROUTE FLOW DN/S OF WATT AVE

RS 2 FLOW -1
 SV 0 0.94 1.87 4.07 5.36 6.69 7.89 9.03
 SQ 0 50 100 200 300 400 500 600
 *

* KKROUTE1

* ZW C=FLOW

* KM ROUTE FLOW FROM WATT AVE TO CURRY CK TRIB

* RD 600 .008 0.05 TRAP 5 5
 *

KKCU2A4C
 ZW C=FLOW
 KM AREA=7.464 AC PER WATT AVE
 BF -1
 BA.01166
 PB
 * PI
 LU 0.1 0.07 2
 UK 200 0.025 0.4 100
 RD 850 0.015 0.040 TRAP 2 10
 *

KK RTE1
 ZW C=FLOW
 KM ROUTE FLOW FROM CU2A4C THRO CU2A4B
 RD 800 .008 0.05 TRAP 5 5
 *

KKCU2A4B
 ZW C=FLOW
 KM AREA 26.094 AC WEST OF WATT AVE EXTENSION
 BF -1
 BA.04077
 LU 0.1 0.07 2
 UK 200 0.015 0.4 100
 RD 900 0.011 0.040 TRAP 2 10
 *

KKCC2A4B
 ZW C=FLOW
 HC 2
 *

KKADD_50
 ZW C=FLOW
 HC 2
 *

* ***** APRIL 28, 06 WATT AVE EXTENSION *****
 * ***** CU2A5 IS DIVIDED INTO 2, CU2A5 AND CU2A5B
 * KK CU2A5 (140.4 AC)
 * ZW C=FLOW
 * KM PORTION OF SHED CU2A

KK CU2A5
 ZW C=FLOW
 KM AREA = 131.754 AC PER WATT AVE
 BF -1
 * BA.21938
 BA.20587
 PB
 * Pi
 LU 0.1 0.07 2
 UK 600 0.0067 0.4 100
 RD 1000 0.008 0.050 TRAP 5 10
 RD 1550 0.0026 0.055 TRAP 10 10
 *

KKCU2A5B
 ZW C=FLOW
 KM AREA = 8.664 AC PER WATT AVE
 BF -1
 BA.01354
 PB
 * PI
 LU 0.1 0.07 2
 UK 200 0.025 0.4 100
 RD 700 0.008 0.040 TRAP 2 10
 *

KKCCU2A5
 ZW C=FLOW
 KM COMBINE FLOW
 HC 2
 *

KKCC2A5C
 ZW C=FLOW
 KM COMBINE FLOW
 HC 2
 *

* ***** MAY 17, 06 PULS ROUTING PER RAS

KKCU2A5R
 ZW C=FLOW
 KM ROUTE FLOW TO CONFLUENCE WITH CURRY CK MAIN CHANNEL
 RS 3 FLOW -1
 SV 0 1.50 2.54 5.31 7.70 9.75 11.62 13.34
 SQ 0 50 100 200 300 400 500 600
 *

* KKCU2A5R
 * ZW C=FLOW
 * KM ROUTE FLOW TO CURRY CK CONFLUENCE

* RD 2100 0.003 0.055 TRAP 10 10

*

* ***** APRIL 28,06 WATT AVE EXTENSION *****

* ***** CU2A6 IS DIVIDED INTO 2, CU2A6 AND CU2A6B

KKCU2A6B

ZW C=FLOW

KM AREA 3.383 AC PER WATT AVE

BF -1

BA.00529

PB

* PI

LU 0.1 0.07 2

UK 100 0.02 0.4 100

RD 300 0.025 0.05 TRAP 2 10

*

KKROUTE2

ZW C=FLOW

KM ROUTE FLOW THROUGH SHED CU2A6

RD 2200 .009 0.035 TRAP 5 10

*

* KK CU2A6 (58.8 AC)

* ZW C=FLOW

KK CU2A6

ZW C=FLOW

KM AREA =55.457 AC PER WATT AVE

* KM PORTION OF CU2A

BF -1

* BA.09188

BA.08665

PB

* Pi

LU 0.1 0.07 2

UK 300 0.027 0.4 100

RD 600 0.013 0.05 TRAP 5 10

RD 1600 0.0063 0.055 TRAP 10 10

*

* KKCU2A6C

* ZW C=FLOW

* KM COMBINE ALL FLOWS BEFORE ADDING TO CURRY CK

* HC 2

* HC 3

*

* ***** APRIL 28, 06 WATT AVE EXTENSION *****

* ***** THE UPPER PORTION OF CU2A IS CUT BT WATT AVE *****

* ** 2 SHEDS CU2A1 AND CU2A2 ARE UP/S OF WATT AVE *****

KK CU2A1

ZW C=FLOW

KM AREA = 9.6 AC UP/S OF WATT

BF -1

BA .0150

PB

* PI

LU 0.1 0.07 2

UK 200 0.01 0.4 100

RD 900 0.01 0.040 TRAP 2 10

*

KKCU2A5C

ZW C=FLOW

KM AREA = 58.6 AC

BF -1

BA.09156

PB

* PI

LU 0.1 0.07 2

UK 200 0.01 0.4 100

RD 1400 0.01 0.045 TRAP 2 10

*

KKCU2A6C

ZW C=FLOW

KM COMBINE FLOWS from cu2a6b, cu2a6, cu2a1 & cu2a5c

* HC 2

HC 4

*

KKadd100

ZW C=FLOW

KM COMBINE ALL FLOWS BEFORE ADDING TO CURRY CK

HC 2

*

KK COMB1

ZW C=FLOW

KM COMBINE FLOWS WITH CURRY CREEK

HC 2

*

KK CU2A2
 ZW C=FLOW
 KM AREA = 24.1 AC UP/S OF WATT
 BF -1
 BA.03766
 PB
 * PI
 LU 0.1 0.07 2
 UK 200 0.01 0.4 100
 RD 800 0.01 0.045 TRAP 2 10
 *

KKCU2A3A
 ZW C=FLOW
 KM AREA = 38.2 AC WEST OF WATT
 BF -1
 BA.05969
 PB
 * PI
 LU 0.1 0.07 2
 UK 200 0.01 0.4 100
 RD 1200 0.01 0.045 TRAP 2 10
 *

KKU2A3AC
 ZW C=FLOW
 KM COMBINE FLOW FROM CU2A2 & CU2A3A AT WEST PROPERTY LINE
 HC 2
 *

KKU2A3AR
 ZW C=FLOW
 KM ROUTE FLOW THROUGH SHED CU2A
 RD 2300 .001 0.055 TRAP 10 5
 *

* ***** JUNE 20,05 *****
 * ***** AREA OF CU2A IS SUBDIVIDED INTO 10 SUBSHEDS=804.2 AC *****
 * ***** THE NEW AREA OF SHED CU2A IS 1386.5-804.2=582.3 AC *****

KK CU2A CURRY CREEK
 ZW C=FLOW
 * KM CH2M Hill Original Basin (2.78 sq.mi.); Reduced for Westpark Prop.
 * Reduces the Wood-Rodger Basin (2.64 SM) by 374.2 Ac for Placer Vineyard sheds
 * USED THE COMPUTE AREA (2.64-.5847) BASED ON ABOVE DIFF
 * 1386.5 Ac
 * BA2.1664
 * BA.90984
 * AREA REVISED PER WATT AVE, NEW AREA=549.34 AC =0.85835
 KM AREA = 485.3 AC NOV 29, 06
 * BA.85835
 BA.75828
 PB

* pi
 BF -1
 LU .1 .06 2
 UK 600 .015 .4 100
 RD 800 .01 .05 .10 TRAP 1 10
 RD 5500 .005 .05 .2 TRAP 5 10
 RD 13500 .002 .05 TRAP 5 10
 *
 * LH 0.05 0 1.5 1.4 0.2
 * US3.0486 0.6
 *

KK CCU2A
 ZW C=FLOW
 KM COMBINED CU2A & RCH3 (2.64 SM + 0.1014 + 0.0706)+ S17BR
 * HC 2
 HC 3
 * HC 4
 *

* KK CWP1 (On-site area 91.2 ac)
 ZW C=FLOW
 KK CWP1 (92.6 AC 6-05)
 ZW C=FLOW
 KM AREA REVISED 6-05
 * KM Revised Date (5-03)
 * BA .1428
 BA.14469
 PB

* pi
 BF -1
 LU .1 0.07 2 .1 0.06 100
 UK 300 0.008 0.350 51
 UK 150 0.010 0.110 49
 RD 450 0.0070 0.018 .05 TRAP 0.0 1
 RD 2050 0.0020 0.015 TRAP 2.0 2
 *

```

KK    R3
ZW C=FLOW
KM main channel through to c-5
RD 1700 0.0030 0.035          TRAP    3.5    10
*
* KK CWP2 (On-site area 127.2 ac)
ZW C=FLOW
* KM Revise Date (5-03)
* BA .1988
KK CWP2 (126.0 AC)
ZW C=FLOW
KM AREA REVISED 6-05
BA.19688
PB
* pi
BF    -1
LU    .1    0.07    100    .1    0.06    2
UK    150  0.0080  0.110    27
UK    300  0.0100  0.350    73
RD    450  0.0070  0.018    .05   TRAP    0.0    1
RD    2000 0.0020  0.020          TRAP    2.0    2
*
KK    C5
ZW C=FLOW
KM node combines area 1 & area 2
HC    2
*
KK    R4
ZW C=FLOW
KM main channel from C-WP2
RD 1800 0.0020 0.035          TRAP    3.5    10
*
KK CWP4 (55.8 ac off-site(Fiddymt);232.1 ac)
ZW C=FLOW
KM Revise Date (5-03)
BA .3627
PB
* pi
BF    -1
LU    .1    0.07    2    .1    0.06    100
UK    300  0.0100  0.350    54
UK    200  0.0200  0.11    46
RD    450  0.0070  0.018    .05   TRAP    0.0    1
RD    2600 0.0020  0.020          TRAP    2.0    2
*
KK    R7
ZW C=FLOW
KM Main channel to C-6
RD 1935 0.0021 0.035          TRAP    3.5    10
*
* KK CWP3 (190.5 ac)
ZW C=FLOW
* KM Revise Date (5-03)
* BA .2977
KK CWP3 (192.0 AC)
ZW C=FLOW
KM AREA REVISED 6-05
BA 0.300
PB
* pi
BF    -1
LU    .1    0.07    2    .1    0.06    100
UK    300  0.0100  0.350    54
UK    200  0.0100  0.11    46
RD    2000 0.0020  0.020          TRAP    2.0    2
*
KK    C6
ZW C=FLOW
KM combines node C-5 and C-WP4
HC    3
*
* Revised Storage per Wood-Rodgers(10-01)
* Proposed Westpark Detention Basin Location
*
* KK DBC6
ZW C=FLOW
* KM Detention Storage (step boxes)(1-3x6;2-4x10 Box Culverts)
* KO    1    2
* RS    1    FLOW    -1
* SV    0    .7    3.5    10.3    22.8    42.5    69.7    88.0    106.0
* SE    86    88    89    90    91    92    93    93.5    94.0
* SQ    0    40    90    200    325    475    650    750    850.0
*

```

KK DBC6
 ZW C=FLOW
 KM Detention Storage (step boxes)(2-5'x 10';2-4'x 10' Box Culverts)
 * KO 1 2
 RS 1 FLOW -1
 SV 0 .7 3.5 10.3 22.8 35.0 42.5 69.7 88.0
 SE 86 88 89 90 91 91.5 92 93 93.5
 SQ 0 40 175 350 600 800 850 950 1000

*
 KK R8
 ZW C=FLOW
 KM main channel through area five
 RD 3700 0.0009 0.035 TRAP 3.5 10

*
 KK CWP6
 ZW C=FLOW
 KM Revise Date (5-03) (194.5 ac)
 BA .3039
 PB
 * pi
 BF -1
 LU .1 0.09 2 .1 0.06 100
 UK 300 0.0100 0.350 72
 UK 200 0.0080 0.11 28
 RD 450 0.0070 0.018 .05 TRAP 0.0 1
 RD 2000 0.0040 0.020 TRAP 2.0 2

*
 KK C8
 ZW C=FLOW
 KM node combines node c-7 and area 6
 HC 2
 *
 * Revised Storage per Wood-Rodgers(10-01)
 * Proposed Westpark Detention Basin Location

*
 * KK DBC8
 * ZW C=FLOW
 * KM Detention Storage (2-24" w/weir)
 * KO 1 2
 * RS 1 FLOW -1
 * SV 0 2.1 12.0 21.6 30.2 38.8 52.4 64.0 85.0 104
 * SE 80 82 84 85 85.5 86 86.5 86.7 87.5 88
 * SQ 0 20 50 60 70 200 650 900 900 9

*
 KK DBC8
 ZW C=FLOW
 KM Detention Storage (3-36" w/weir)
 * KO 1 2
 RS 1 FLOW -1
 SV 0 2.1 12.0 21.6 30.2 38.8 42.5 52.4 64.0
 SE 80 82 84 85 85.5 86 86.3 86.5 86.7
 SQ 0 30 120 165 200 500 900 950 1050

*
 KK R13
 ZW C=FLOW
 KM main channel EXIT FROM NODE C-8
 RD 2000 0.001 0.035 TRAP 5.0 5

*
 * KK CWP9 (241.9 off-site,assume future det)
 * ZW C=FLOW
 * KM Revise Date (5-03)
 * BA .3780

*
 KK CWP9 (218.2 AC)
 ZW C=FLOW
 KM AREA REVISED 6-05 area rewised to 191.524 ac
 * BA.31094
 BA.29926

PB
 * pi
 BF -1
 LU .1 0.07 2 .1 0.07 50
 UK 300 0.0080 0.40 92
 UK 400 0.0050 0.240 8
 RD 1450 0.0070 0.050 TRAP 0.0 1
 RD 3600 0.0020 0.050 TRAP 2.0 2

*
 KK R913
 ZW C=FLOW
 KM Route shed through WP-13
 RD 2000 0.0023 0.035 TRAP 10 10

*
 * KK CWP13 (62.0 ac)
 * ZW C=FLOW

```

* KM Revise Date (5-03)
* BA .0969
KK CWP13 (60.3 AC)
ZW C=FLOW
KM AEEA REVISED 6-05
BA.09422
PB
* pi
BF -1
LU .1 0.07 2 .1 0.07 100
UK 300 0.0080 0.350 67
UK 150 0.0080 0.11 33
RD 450 0.0070 0.018 .03 TRAP 0.0 1
RD 2400 0.0020 0.020 TRAP 2.0 2
*
KK C9
ZW C=FLOW
KM Combines Area WP9 & WP13
HC 2
*
KK C13
ZW C=FLOW
HC 2
*
* KK CWP14 (47.4 ac)
* ZW C=FLOW
* KM Revise Date (5-03)
* BA .0741
KK CWP14 (97.6AC)
ZW C=FLOW
KM AERA REVISED 6-05
BA.15250
PB
* pi
BF -1
LU .1 0.06 2 .1 0.06 100
UK 300 0.0080 0.350 98
UK 150 0.0100 0.110 2
RD 1600 0.0020 0.020 TRAP 2.0 2
*
KKCCWP14
ZW C=FLOW
KM Combines Area WP14, C-8 & C-9
HC 2
*
KK CU2B1 (54.0 AC)
ZW C=FLOW
KM 53.9 ACRES
KM AREA REVISED 6-05
BA.11500
PB
* pi
BF -1
LU .1 .06 2
UK 200 .005 .4 100
RD 1900 .008 .05 TRAP 2 10
*
KKCU2B1A (29.2AC)
ZW C=FLOW
KM 29.2 ACRES
KM AREA REVISED 6-05
BA.04563
PB
* pi
BF -1
LU .1 .06 2
UK 200 .005 .4 100
RD 1600 .008 .05 TRAP 2 10
*
KK C10
ZW C=FLOW
KM COMBINE FLOW AT WATT AVE & PLEASANT GROVE BLVD
HC 3
*
*
* KK RCH4 ROUTE FROM C-10 TO CU2A+
* ZW C=FLOW
* RS 1 FLOW -1
* RD
* REVISED ROUTING LENGTH FROM 8000' TO 6600' 8/12/03 - CS/gsu
* RC 0.055 0.055 0.055 6600 0.0014 1000
* REVISED CHANNEL SECTION TO MATCH AG DITCH PER TOPO 8/12/03 - CS/gsu
* RX 0 0 400 413 427 440 840 840

```

* RY 1020 1000 990 983.5 983.5 990 1000 1020
 * CU2B REPLACED WITH; CU2B1, CU2B2, & CU2B3 (74ac+243.9ac+ 221.1ac=539ac)
 * WESTPARK BASIN CU2B (0.932 sq mi) REVISED TO REFLECT CURRENT AG FIELDS OPER.

* KK CU2B CURRY CREEK
 * ZW C=FLOW
 * KM CH2M Hill Original Basin (2.71 sq.mi.); Reduced for Westpark Prop.

* BA .932
 * PB
 * * pi
 * BF -1
 * LU .1 .06 2
 * UK 600 .015 .4 100
 * RD 800 .01 .05 .10 TRAP 1 10
 * RD 5500 .005 .05 .2 TRAP 5 10
 * RD 13500 .002 .05 TRAP 5 10

* * LH 0.05 0 1.5 1.4 0.2
 * * US2.9025 0.6

* KK CU2B1 (54.0 AC)
 * ZW C=FLOW
 * KM 73.6 ACRES
 * KM AREA REVISED 6-05

* BA .1150
 * BA.08438
 * PB
 * * pi
 * BF -1
 * LU .1 .06 2
 * UK 200 .005 .4 100
 * RD 1900 .008 .05 TRAP 2 10

* KKCU2B1A (29.2AC)
 * ZW C=FLOW
 * KM 73.6 ACRES
 * KM AREA REVISED 6-05

* BA .1150
 * BA.04563
 * PB
 * * pi
 * BF -1
 * LU .1 .06 2
 * UK 200 .005 .4 100
 * RD 1600 .008 .05 TRAP 2 10

* KKCU2B1C
 * ZW C=FLOW
 * KM COMBINE FLOW AT WATT AVE & PLEASANT GROVE BLVD
 * HC 3

* *****
 * ***** 9-17-03 *****
 * ***** STORAGE ROUTING BASED ON HEC-RAS DATA *****
 * ***** OVERLAND FLOW = 600FT OR LESS *****
 * ***** %IMPERVIOUS = 2 *****
 * ***** CONSTTANT INFILT = 0.01 *****

KK RR1
 ZW C=FLOW
 KM ROUTE FLOW FROM C-10, 3500FT DNS TO CP-1
 RS 4 FLOW -1
 SV 0 4.6 7.4 10 15.7 20.6 24.5 37.6 45.5
 SQ 0 100 216 300 405 500 559 785 900

* KK CU2B2
 * ZW C=FLOW
 * KM 243.9 ACRES
 * KM AREA = 214.8 AC NOV 29, 06

* BA .3811
 * BA.33563
 * PB
 * * pi
 * BF -1
 * LU .1 .06 2
 * UK 600 .0050 .4 100
 * RD 2700 .0067 .05 .38 TRAP 10 2

* KK CU2B1 (54.0 AC)
 * ZW C=FLOW
 * KM 73.6 ACRES
 * KM AREA REVISED 6-05

* BA .1150
 * BA.08438
 * PB

```

* * pi
* BF -1
* LU .1 .07 2
* UK 200 .005 .4 100
* RD 1900 .008 .05 TRAP 1 10
*
KK CP1
ZW C=FLOW
KM COMBINE CU2B2 WITH FLOW AT CP-1
HC 2
* HC 3
*
KK RR2
ZW C=FLOW
KM ROUTE FLOW FROM CP-1 TO CU2A+
RS 4 FLOW -1
SV 0 16.6 25.9 34.1 51.7 57.3 71.2 91.8 95.4
SQ 0 150 245 320 467 500 634 872 900
*
KK CU2B3
ZW C=FLOW
KM 221.1 ACRES
BA .3455
PB
* * pi
BF -1
LU .1 .06 2
UK 600 .0050 .4 100
RD 6300 .0012 .05 TRAP 4 1.5
*
KK CU3B+
ZW C=FLOW
KM COMBINE CU3B & RR2
HC 2
*
KK CU2A+ COMBINE HYDROGRAPHS
ZW C=FLOW
* HC 6
HC 2
*
* REV RCH5 ROUTING TO REFLECT CU2C1, CU2C2 & CU2C3 SUB-SHEDS
* KK RCH5 ROUTE FROM CU2A TO CU2C+
* ZW C=FLOW
* RS 1 FLOW -1
* RD
* RC 0.045 0.045 0.045 8800 0.0014 1000
* RX 0 0 1000 1013 1027 1040 2040 2040
* RY 1020 1000 990 983.5 983.5 990 1000 1020
*
* REPLACE BASIN CU2C WITH CU2C1, CU2C2 & CU2C3 (186.4+146.6+246.6=579.6ac)
* KK CU2C CURRY CREEK (441.6 ac.)
* ZW C=FLOW
* BA 0.69
* PB
* * pi
* BF -1
* LU .1 .06 2
* UK 600 .015 .4 100
* RD 800 .01 .05 .10 TRAP 1 10
* RD 3500 .005 .05 .2 TRAP 5 10
* RD 7500 .002 .05 TRAP 5 10
*
* * LU 0.05 0 1.5 1.4 0.2
* * US 1.676 0.6
*
* REV SUB-SHEDS CU2C-1,2,&3
KK RR3
ZW C=FLOW
KM ROUTE FLOW FROM CU2A+ TO CU2C1, 2800 FT
RS 3 FLOW -1
SV 0 7.7 23.2 32.8 57.4 59.6 66.1 85.9 95.1
SQ 0 250 491 700 1369 1400 1566 2192 2500
*
KK CU2C1
ZW C=FLOW
KM 186.4 ACRE @ COUNTRY ACRE LN
BA .2913
PB
* * pi
BF -1
LU .1 .06 2
UK 600 .0050 .4 100
RD 3700 .0016 .1 .25 TRAP 10 20

```

```

*
KK CP2
ZW C=FLOW
KM COMBINE RUNOFF FROM CUUC1 WITH FLOW AT CP-2
HC 2
*
KK RR4
ZW C=FLOW
KM ROUTE FLOW FROM CP-2 TO CP-3, 2800 FT DNS
RS 4 FLOW -1
SV 0 20.7 34 44.3 64 70.7 81.8 112.3 125.6
SQ 0 250 500 700 1140 1300 1566 2185 2500
*
KK CU2C2
ZW C=FLOW
KM 146.6 ACRES COUNTRY ACRES LN TO "LANDING STRIP"
BA .229
PB
* pi
BF -1
LU .1 .06 2
UK 100 .0050 .4 100
RD 4200 .009 .05 TRAP 15 2.8
*
KK CP3
ZW C=FLOW
KM COMBINE RUNOFF FROM CU2C2 WITH FLOW AT CP-3
HC 2
*
KK RR5
ZW C=FLOW
KM ROUTE FLOW FROM CP-3 TO BREWER ROAD 4200 FT DNS
RS 3 FLOW -1
SV 0 7 11.1 12.2 15.8 17.2 19.8 27.2 30.1
SQ 0 260 513 720 1167 1350 1601 2230 2500
*
KK CU2C3
ZW C=FLOW
KM 216.6 ACRES "LANDING STRIP" TO BREWER RD
BA.33844
PB
* pi
BF -1
LU .1 .06 2
UK 600 .0050 .4 100
RD 1900 .0012 .1 .2 TRAP 300 1
RD 800 .0063 .05 .15 TRAP 2 2
* RD2600 .0040 .05 TRAP 15 2
*
KK CU2C+ COMBINE HYDROGRAPHS
ZW C=FLOW
HC 2
*
* KK RCH6 ROUTE FROM CU2C+ TO CU4+
* ZW C=FLOW
* RS 1 FLOW -1
* RD
* RC 0.045 0.045 0.045 12200 0.001 1000
* RX 0 0 1200 1210.5 1230.5 1241 2441 2441
* RY 1020 1000 995 988 988 995 1000 1020
*
KKCWP11A (48.0 ac)
ZW C=FLOW
KM Curry Creek Basin
BA .075
PB
* pi
BF -1
LU .1 0.06 2 .1 0.06 100
UK 300 0.010 0.350 60
UK 150 0.020 0.11 40
RD 500 0.0040 0.024 .08 TRAP 1 5
RD 900 0.0020 0.024 TRAP 2 10
*
KK DB5
ZW C=FLOW
KM Det Basin A for C-WP11A (1.0 acre surface area; 2-18" outfalls)
* KO 1 2
RS 1 FLOW -1
SV 0 .7 1.8 2.8 3.9 5.1 6.5
SE 87.3 88 89 90 91 92 92.5
SQ 0 2 7 15 24 55 90
*

```

KK R11 ROUTE FROM DB5 thru C-WP11

ZW C=FLOW

RS 1 FLOW -1

RD

RC	0.045	0.045	0.045	1800	0.0028	1000		
RX	0	0	1000	1013	1027	1040	2040	2040
RY	1020	1000	990	983.5	983.5	990	1000	1020

*

KK CWP11 (78.9 ac)

ZW C=FLOW

KM Open Space

BA .123

PB

* pi

BF -1

LU	.1	0.06	2	.1	0.06	100		
UK	300	0.010	0.350	95				
UK	200	0.010	0.11	5				
RD	1900	0.0036	0.050	.08	TRAP	3.5	10	
RD	1300	0.0028	0.050		TRAP	3.5	10	

*

KK CB11 COMBINE HYDROGRAPHS

ZW C=FLOW

HC 2

*

KK R1112

ZW C=FLOW

KM Route shed through WP-12

RD	1200	0.0036	0.035		TRAP	10	10	
----	------	--------	-------	--	------	----	----	--

*

KK CWP12

ZW C=FLOW

* KM (43.4 ac.)

BA0.0678

PB

* pi

BF -1

LU	0.10	0.06	100	0.10	0.06	2		
UK	600	0.0100	0.110	2				
UK	600	0.0100	0.400	98				
RD	1400	0.0085	0.035		TRAP	3.5	10	

*

KK C1112

ZW C=FLOW

KM combines WP11 and WP12 at Future Watt Ave. 170.3 ac=.266 sq mi

HC 2

*

* KK R23

* ZW C=FLOW

* KM chanelized flow Basin CC3 exit route

* RD	100	0.0020	0.024		TRAP	3.5	10	
------	-----	--------	-------	--	------	-----	----	--

* REPLACED RCH7 ROUTING & CURRY CK SHED CU3 - W/ SHEDS CU3 A THRU J

* CIVIL SOLUTIONS 8/03 gsu

* KK RCH7 ROUTE FROM C11-12 TO CU3+

* ZW C=FLOW

* RS 1 FLOW -1

RD

* RC	0.045	0.045	0.045	16000	0.0014	1000		
* RX	0	0	1000	1013	1027	1040	2040	2040
* RY	1020	1000	990	983.5	983.5	990	1000	1020

*

* KK CU3 CURRY CREEK

* ZW C=FLOW

* KM CH2M Hill Original Basin (3.79 sq.mi.); Reduced for Westpark Prop.

* BA 3.48

* PB

* * pi

* BF -1

* LU	.1	.06	2					
* UK	600	.015	.4	100				
* RD	800	.01	.05	.10	TRAP	1	10	
* RD	7500	.005	.05	.2	TRAP	5	10	
* RD	15000	.002	.05		TRAP	5	10	

*

* * LH 0.05 0 1.5 1.4 0.3

* * US2.6124 0.6

* BEGIN DeLaSal sheds; Exist'g terraced field grading & Ag ditches

*

* ***** STORAGE ROUTING BASED ON HEC-RAS DATA ***** 9-17-03

KK RCU3B ROUTE FROM C11-12 THRU CU3B

ZW C=FLOW

```

* RS 1 FLOW -1
* RD
* RC.045 0.045 0.045 5300 0.0022 80
* RX 745 753 1463 1467 1471 1485 1490 1500
* RY 78 76 76 77 77 74 74 78
KM ROUTE FLOW FROM C11-12 TO CU3B 5300 FT DNS
RS 6 FLOW -1
SV 0 2.9 14.3 20.3 27.9 30.8 32.9 41.7 47.9
SQ 0 30 62 90 159 190 214 298 350

```

```

*
KK CU3A
ZW C=FLOW
KM 247.3 acres
BA .3864
PB

```

```

* PI
BF -1
LU .1 .06 2
UK 600 .005 .4 100
RK 2000 .01 .05 .25 TRAP 10 10
RK 1500 .003 .05 TRAP 5 5

```

```

*
KK CU3AC
ZW C=FLOW
HC 2

```

```

* KK 3B2RR
* ZW C=FLOW
* KM ROUTE FLOW FROM WP to CULVERT 1

```

```

* KO 1
* RS 1 FLOW -1
* * POST PROJECT UNMIT AND MIT PER Exist CHANNEL
* * STATION 70 to 68
* SV 0 0.3 1.1 1.7 2.2 2.8
* SQ 0 56 160 218 397 600

```

```

*
KK CU3B
ZW C=FLOW
KM 307.4 ACRES
BA .4803
PB

```

```

* PI
BF -1
LU .1 .06 2
UK 600 .001 .4 100
RK 1800 .0013 .05 TRAP 200 1
RK 2600 .0022 .05 TRAP 5 3 YES

```

```

*
KK CU3C
ZW C=FLOW
KM 110.3 acres
BA .1723
PB

```

```

* PI
BF -1
LU .1 .06 2
UK 600 .001 .4 100
RK 2900 .0076 .05 TRAP 10 10

```

```

*
KK CC3B+
ZW C=FLOW
KM COMBINE RCU3B+CU3B&CU3A+CU3C;170.3 + 307.4+247.3 + 110.3 = 835.3=1.3052 sq mi
HC 2

```

```

*
KK RC3B+ ROUTE CC3B+ TO CU3D
ZW C=FLOW

```

```

* RS 1 FLOW -1
* RD
* RC.050 0.050 0.050 3800 0.0006
* RX 365 380 565 570 578 600 820 1056
* RY 69 65 65 61 61 64 64 65

```

```

*
KM ROUTE FLOW FROM CC3B+ TO CU3D 3800 FT DNS
RS 4 FLOW -1
SV 0 8.2 63.3 69.1 78.3 81.6 92.0 110.1 143.6
SQ 0 50 97 150 263 300 345 445 550

```

```

KK CU3D
ZW C=FLOW
KM 465.8 acres ROUTES & COMBINES 465.8+835.3=1,301.1 ac = 2.033 Sq. Mi.
BA .7278

```

```

PB
* PI
BF -1

```

LU	.1	.06	2						
UK	600	.001	.4	100					
RK	4400	.0006	.05	.4	TRAP	7	3		
RK	1500	.006	.06		TRAP	100	2	YES	

*
 KK RC3D+ ROUTE CU3D TO CU3E+
 ZW C=FLOW

* RS	1	FLOW	-1						
* RD									
* RC.050	0.050	0.050	3600	0.00086	60				
* RX	0	128	160	166	184	192	270	320	
* RY	59	59	58	57	57	58	59	59	

KM ROUTE FLOW CU3D TO CU3E+ 3600 FT DNS

RS	4	FLOW	-1						
SV	0	3.9	7.5	11.0	15.0	16.6	18.4	22.5	24.6
SQ	0	70	140	220	321	375	428	570	650

KK CU3E

ZW C=FLOW

KM 111.8 acres

BA .1747

PB

* PI

BF	-1								
LU	.1	.06	2						
UK	600	.001	.4	100					
RK	3600	.00086	.05		TRAP	18	8		

*
 KK CU3E+
 ZW C=FLOW
 KM COMBINE CU3E & RC3D+ 111.8+1301.1=1412.9 ac = 2.2077 Sq. Mi.
 HC 2

*
 KK RC3E+
 ZW C=FLOW
 KM ROUTE FLOW FROM CU3E+ TO CU3F+

RS	2	FLOW	-1						
SV	0	2.8	4.6	7.1	9.4	10.2	11.4	14.1	15.0
SQ	0	75	153	200	351	400	466	618	700

KK CU3F

ZW C=FLOW

KM 641.7ACRES

BA1.0026

PB

* PI

BF	-1								
LU	.1	.06	2						
UK	600	.001	.4	100					
RK	4100	.0062	.05	.5	TRAP	15	3		
RK	4400	.0013	.05		TRAP	12	2		

* RC3F+ ROUTE CU3F TO CU3E+

* RS	1	FLOW	-1						
* RD									
* RC.050	0.050	0.050	1500	0.0006					
* RX	43	65	85	93	230	268	446	473	
* RY	60	59	58	57	57	58	58	60	

*
 KK CC3F+
 ZW C=FLOW
 KM COMBINE RC3F+ & CU3E+ (602.7+ 1412.9=2015.6 ac = 3.149 Sq. Mi.)
 HC 2

*
 KK RC3F+
 ZW C=FLOW
 KM ROUTE CC3F+ TO CU3+ 2015.6 ac=3.149 Sq. Mi.

* RS	1	FLOW	-1						
* RD									
* RC.050	0.050	0.050	2700	.00019					
* RX	205	305	620	719	772	776	809	1207	
* RY	55	53	53	52	52	51	53	54	
RS	3	FLOW	-1						
SV	0	17.2	30.9	54.4	75.5	81.0	95.1	123.3	131.3
SQ	0	80	164	300	410	470	565	780	850

KK CU3G

ZW C=FLOW

KM 200.0 acres

BA .3125

PB

* PI

BF	-1								
LU	.1	.06	2						
UK	600	.001	.4	100					

```

RK 4500 .001 .05 TRAP 70 90
*
KK CU3HA
ZW C=FLOW
KM 57.4 acres
BA.08969
PB
* PI
BF -1
LU .1 .06 2 .1 .06 50
UK 600 .0050 .4 100
RK 1500 .001 .05 TRAP 10 2
*
KK CU3HB
ZW C=FLOW
KM 106.1 acres
BA .1658
PB
* PI
BF -1
LU .1 .06 2
UK 300 .001 .4 100
RK 4000 .0034 .05 TRAP 100 50 YES
*
KK CU3
ZW C=FLOW
KM COMBINE CU3H & CU3G & RC3F+ (106.1+200.0+2015.6=2321.7=3.627 Sq. Mi.
HC 3
*
KK CU3C+
ZW C=FLOW
KM COMBINE CU3 & CU2C+ (2321.7+
HC 2
KK RCH8 ROUTE FROM CU3 TO CU4+
ZW C=FLOW
* RS 1 FLOW -1
* RD
* RC.045 0.045 0.045 12200 0.001 1000
* RX 0 0 1200 1210.5 1230.5 1241 2441 2441
* RY1020 1000 995 988 988 995 1000 1020
* RS 14 FLOW -1
RS 10 FLOW -1
SV 0 51 115.5 194.1 281.7 287.9 335.4 390.5 400.2
SQ 0 400 621 1000 1387 1500 1900 2682 3200
* SQ 0 400 621 1000 1387 1500 1900 2682 2800
KK CU4 CURRY CREEK (2291 ac)
ZW C=FLOW
BA 3.58
PB
* pi
BF -1
LU .1 .06 2
UK 600 .015 .4 100
RD 800 .01 .05 .10 TRAP 1 10
RD 8500 .005 .05 .2 TRAP 5 10
RD 12000 .002 .05 TRAP 5 10
*
* LH 0.06 0 1.5 1.4 0.5
* US2.3818 0.6
*
KK CU5 CURRY CREEK
ZW C=FLOW
BA 0.28
PB
* pi
BF -1
LU .1 .06 2
UK 600 .015 .4 100
RD 500 .01 .05 .10 TRAP 1 10
RD 2500 .002 .05 TRAP 5 10
*
* LH 0.06 0 1.5 1.4 0.5
* US0.9782 0.6
*
KK CU5+ COMBINE HYDROGRAPHS
ZW C=FLOW
HC 3
*
KK RCH9 ROUTE FROM CU5 TO CU6
ZW C=FLOW
* RS 1 FLOW -1
RD
RC 0.04 0.04 0.04 6000 0.0016 1000

```

RX 0 0 1200 1210 1230 1240 2440 2440
RY 1020 1000 995 990 990 995 1000 1020

*
KK CU6 CURRY CREEK

ZW C=FLOW

BA 0.87

PB

* pi

BF -1

LU .1 .06 2

UK 600 .015 .4 100

RD 500 .01 .05 .10 TRAP 1 10

RD 1500 .005 .05 .2 TRAP 5 10

RD 6000 .002 .05 TRAP 5 10

*

* LH 0.06 0 1.5 1.4 0.5

* US1.2791 0.6

*

KK CU6+ COMBINE HYDROGRAPHS - END CURRY CREEK

ZW C=FLOW

HC 2

*

ZZ

**APPENDIX C
HEC-1 POST-PROJECT
UNMITIGATED INPUT FILE**

ID PLACER COUNTY WATERSHED MODEL, PLACER COUNTY, CA
 ID WATERSHED UPDATE MODELS - DRAFT ULT BUILDOUT
 ID DRAFT MODEL FOR HYD ROUTING - HEC1VOLS UTILITY
 ID CESI 1401/25/2129

ID
 IT 5 01JAN97 0 600
 IO 5 0
 IN 5 0 0

*DIAGRAM

*
 *

KK CUS3G
 KM AREA EAST OF FIDDYMENT ROAD AREA= 281.3 AC
 BA.43953

PB

* PI

BF	-1	-0.001	1.50					
LU	0.1	0.07	2	0.10	0.1200	41.369		
UK	150	0.01	0.4			1		
UK	200	0.0050	0.240			99		
RD	3000	0.0010	0.015				CIRC	3.0 0.0
RD	2300	0.0005	0.015				CIRC	5.0 0.0

ZW C=FLOW

*

KK SS3
 KM AREA=45.2 AC
 BA0.0707

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.982	0.10	0.1200	56.659		
UK	150	0.0100	0.400			5		
UK	96	0.0108	0.240			95		
RD	800	0.0010	0.015				CIRC	3.0 0.0
RD	2100	0.0010	0.015				CIRC	5.0 0.0

ZW C=FLOW

*

KK SS3B
 KM AREA= 16.1 AC
 BA0.0252

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	2.089	0.10	0.1200	84.855		
UK	300	0.0100	0.400			94		
UK	89	0.0122	0.240			6		
RD	400	0.0010	0.055				TRAP	2.0 10.0
RD	1200	0.0010	0.055				TRAP	10.0 5.0

ZW C=FLOW

*

KKYCUS3A
 KM COMBINE FLOW AT FIDDYMENT ROAD
 HC 3

ZW C=FLOW

*

*

KKUCUS3A
 KM ROUTE FLOW FROM FIDDYMENT ROAD TO UPLAND DIRVE PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING

RS	1	FLOW	-1					
SV	0.00	2.12	3.42	7.01	11.20	16.54	20.91	25.87
SQ	0.0	50.0	100.0	250.0	400.0	550.0	700.0	800.0

ZW C=FLOW

*

KK SS3C
 KM AREA=62.8 AC
 BA0.0981

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.997	0.10	0.1200	42.681		
UK	150	0.0100	0.400			7		
UK	130	0.0100	0.240			93		
RD	800	0.0010	0.015				CIRC	3.0 0.0
RD	1700	0.0010	0.015				CIRC	5.0 0.0

ZW C=FLOW

*

KK SS3D
 KM AREA= 66.4 AC
 BA0.1037

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.993	0.10	0.1200	65.734		

UK 150 0.0100 0.400 1
 UK 85 0.0130 0.240 99
 RD 850 0.0010 0.015 CIRC 3.0 0.0
 RD 1250 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*
 KK SS3E
 KM AREA= 15.1 AC
 BAO.0236
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.084
 UK 300 0.0100 0.400 100.00
 RD 300 0.0010 0.055 TRAP 2.0 10.0
 RD 1400 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*
 KK SS5
 KM AREA= 20.2 AC
 BAO.0316
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 70.000
 UK 81 0.0138 0.240 100.00
 RD 400 0.0010 0.055 TRAP 2.0 10.0
 RD 700 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*
 KKYCUS3B
 KM COMBINE FLOW AT UPLAND DRIVE
 HC 5
 ZW C=FLOW

*
 KKUCUS3B
 KM ROUTE FLOW THROUGH SHED CUS3E PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING
 RS 1 FLOW -1
 SV 0.00 2.09 3.84 7.89 11.29 14.29 17.00 18.70
 SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0

ZW C=FLOW

*
 KK SS3F
 KM AREA= 73.7 AC
 BAO.1151
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.999 0.10 0.1200 47.061
 UK 150 0.0100 0.400 6
 UK 115 0.0100 0.240 94
 RD 750 0.0010 0.015 CIRC 3.0 0.0
 RD 1200 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*
 KK CUS5A
 KM CUS5A 33.9 AC
 BAO.0530
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.000
 UK 200 0.0050 0.400 100.00
 RD 300 0.0050 0.040 TRAP 2.0 10.0
 RD 1000 0.0010 0.040 TRAP 2.0 10.0

ZW C=FLOW

*
 KK SS4A
 KM AREA= 24.4 AC
 BAO.0381
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.254
 UK 300 0.0100 0.400 100.00
 RD 400 0.0010 0.055 TRAP 10.0 5.0
 RD 1900 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*
 KK SS5C
 KM AREA= 18.0 AC
 BAO.0281

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 71.093
 UK 79 0.0142 0.240 100.00
 RD 1200 0.0010 0.015 CIRC 3.0 0.0
 ZW C=FLOW
 *
 KKYCUS3E
 KM COMBINE FLOWS
 HC 5
 ZW C=FLOW
 *
 *
 KKUCUS3E
 KM ROUTE FLOW THROUGH SHED CUS4A TO MARKET STREET PUL'S STOTAGE ROUTING - NOV 7- 06 RAS PER CH GRADING
 RS 1 FLOW -1
 SV 0.00 2.74 4.88 11.19 16.85 23.12 31.16 40.16
 SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
 ZW C=FLOW
 *
 KK SS4
 KM AREA= 82.0 AC
 BAO.1281
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 5.000 0.10 0.1200 57.303
 UK 150 0.0100 0.400 5
 UK 95 0.0110 0.240 95
 RD 550 0.0010 0.015 CIRC 3.0 0.0
 RD 2000 0.0010 0.015 CIRC 5.0 0.0
 ZW C=FLOW
 *
 KK SS5D
 KM AREA= 6.2 AC
 BAO.0097
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 5.000 0.10 0.1200 71.792
 UK 150 0.0100 0.400 8
 UK 84 0.0132 0.240 92
 RD 800 0.0010 0.015 CIRC 3.0 0.0
 RD 1000 0.0010 0.015 CIRC 5.0 0.0
 ZW C=FLOW
 *
 KK CUS6B
 KM AREA= 6.6 AC
 BAO.0103
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.971 0.10 0.1200 71.974
 UK 150 0.0100 0.400 29
 UK 400 0.0050 0.240 71
 RD 400 0.0016 0.015 CIRC 2.0 0.0
 RD 400 0.0010 0.015 CIRC 3.0 0.0
 ZW C=FLOW
 *
 KK SS6A
 KM AREA= 3.7 AC
 BAO.0058
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 40.000
 UK 130 0.0100 0.240 100.00
 RD 235 0.0100 0.015 CIRC 3.0 0.0
 ZW C=FLOW
 *
 KK SS6
 KM AREA= 7.8 AC
 BAO.0122
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.425 0.10 0.1200 85.000
 UK 150 0.0100 0.400 88
 UK 115 0.0100 0.240 12
 RD 400 0.0010 0.055 TRAP 10.0 5.0
 RD 500 0.0010 0.055 TRAP 10.0 5.0
 ZW C=FLOW

```

*
KKYCUS4B
KM COMBINE ALL FLOW AT MARKET STREET (EXCLUDING US ROUTING)
HC 5
ZW C=FLOW
*
KKYCUS4A
KM COMBINE ALL FLOW AT MARKET STREET (INCLUDING US ROUTING)
HC 2
ZW C=FLOW
*
*
KKUCUS4A
KM ROUTE FLOW THROUGH SHED CUS6 TO BASELINE ROAD PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING
RS 1 FLOW -1
SV 0.00 1.06 1.95 4.64 8.64 14.19 21.21 26.76
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KK CUS5B
KM CUS5B 94.1 AC
BA0.1470
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000
UK 200 0.0050 0.400 100.00
RD 1800 0.0050 0.040 TRAP 2.0 10.0
RD 1000 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*
KKYCCUS6
KM COMBINE FLOW AT BASELINE ROAD
HC 2
ZW C=FLOW
*
*
KKUUS5BR
KM ROUTE FLOW DN/S OF BASELINE ROAD THROUGH PLACER VINEYARDS
RS 1 FLOW -1
SV 0.00 5.99 10.55 21.67 30.29 37.67 44.57 48.92
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KKCUS11B
KM CUS11B 87.9ac
BA0.1374
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0748 2.000
UK 200 0.0050 0.400 100.00
RD 500 0.0050 0.040 TRAP 2.0 10.0
RD 150 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*
KKY12ACC
KM COMBINE FLOW AT WEST SIDE DRIVE
HC 2
ZW C=FLOW
*
*
KKU12ACR
KM ROUTE FLOW TO BASELINE ROAD CULVERT
RS 1 FLOW -1
SV 0.00 1.80 4.05 10.48 18.00 30.50 45.34 48.82
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KK CUS14
KM CUS14 AREA = 62.1 AC
BA0.0970
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0760 2.000
UK 200 0.0050 0.400 100.00
RD 800 0.0050 0.040 TRAP 2.0 10.0
RD 900 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*
KKYCUS14
KM COMBINE FLOW UP/S OF BASELINE ROAD CULVERT

```

HC 2
ZW C=FLOW
*
*

KKU RR_7
KM ROUTE FLOW THROUGH SHED CUS15C FROM BASELINE RD TO WATT AVE PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH
GRADING

RS	1	FLOW	-1					
SV	0.00	0.77	1.31	2.91	4.96	7.52	11.54	15.48
SQ	0.0	50.0	100.0	250.0	400.0	550.0	700.0	800.0

ZW C=FLOW
*

KKCUS16B
KM
BA0.0208
PB

* PI
BF -1 -0.001 1.50
LU 0.10 0.0720 2.000
UK 200 0.0050 0.400 100.00
RD 900 0.0050 0.040 TRAP 2.0 10.0
RD 900 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*

KKYUS16B
KM
HC 2
ZW C=FLOW
*

KK SS12A
KM AREA = 18.1 AC
BA0.0283
PB

* PI
BF -1 -0.001 1.50
LU 0.10 0.1200 70.939
UK 79 0.0142 0.240 100.00
RD 400 0.0016 0.015 CIRC 2.0 0.0
RD 900 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*

KK SS13A
KM AREA=141.0 AC
BA0.2204
PB

* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 5.000 0.10 0.1200 55.922
UK 150 0.0100 0.400 6
UK 99 0.0102 0.240 94
RD 800 0.0010 0.015 CIRC 3.0 0.0
RD 4400 0.0010 0.015 CIRC 5.0 0.0
ZW C=FLOW
*

KKYSS13A
KM 0
HC 2
ZW C=FLOW
*

KK SS15
KM AREA= 6.6 AC
BA0.0104
PB

* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.585 0.10 0.1200 84.484
UK 300 0.0100 0.400 69
UK 89 0.0122 0.240 31
RD 400 0.0010 0.055 TRAP 2.0 10.0
RD 600 0.0010 0.055 TRAP 10.0 10.0
ZW C=FLOW
*

KK YSS15
KM 0
HC 3
ZW C=FLOW
*

KKCUS17A
KM CUS17A AREA=38.2 AC
BA0.0597
PB

* PI
BF -1 -0.001 1.50

LU 0.10 0.0700 2.000
 UK 200 0.0050 0.400 100.00
 RD 500 0.0050 0.040 TRAP 2.0 10.0
 RD 500 0.0010 0.040 TRAP 2.0 10.0

ZW C=FLOW

*

KKV RR_9

KM ROUTE FLOW FROM BASELINE ROAD TO MAIN CHANNEL

RD 1300 0.0010 0.055 TRAP 5.0 5.0

ZW C=FLOW

*

KK SS15B

KM AREA= 5.7 AC

BA0.0089

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 2.724 0.10 0.1200 72.173
 UK 150 0.0100 0.400 1
 UK 78 0.0144 0.240 99
 RD 400 0.0016 0.015 CIRC 2.0 0.0
 RD 400 0.0010 0.015 CIRC 4.0 0.0 YES

ZW C=FLOW

*

KKYOMB_7

KM COMBINE FLOW DN/S OF WATT AVE

HC 2

ZW C=FLOW

*

*

KKU RR_8

KM ROUTE FLOW THROUGH SHED CUS15B PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING

RS 1 FLOW -1
 SV 0.00 2.17 3.69 9.34 14.20 18.27 21.98 24.30
 SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0

ZW C=FLOW

*

KK SS15A

KM AREA=74.8 AC

BA0.1168

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 4.996 0.10 0.1200 57.177
 UK 150 0.0100 0.400 13
 UK 94 0.0112 0.240 87
 RD 500 0.0010 0.015 CIRC 3.0 0.0
 RD 2600 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*

KKYSS15A

KM 0

HC 2

ZW C=FLOW

*

KK SS15F

KM AREA=19.0 AC

BA0.0297

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 2.125 0.10 0.1200 80.401
 UK 150 0.0100 0.400 96
 UK 121 0.0100 0.240 4
 RD 800 0.0010 0.055 TRAP 10.0 5.0
 RD 1200 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*

KK SS15C

KM AREA= 4.2 AC

BA0.0066

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 4.978
 UK 150 0.0100 0.400 100.00
 RD 200 0.0016 0.015 CIRC 2.0 0.0
 RD 200 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*

KK SS15D

KM AREA= 6.9 AC

BA0.0108

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.742 0.10 0.1200 40.000
UK 150 0.0100 0.400 1
UK 130 0.0100 0.240 99
RD 500 0.0010 0.015 CIRC 7.0 0.0
ZW C=FLOW
*
KK SS15E
KM AREA=19.6 AC
BA0.0306
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000 0.10 0.1200 70.638
UK 150 0.0100 0.400 1
UK 79 0.0142 0.240 99
RD 1000 0.0010 0.015 CIRC 6.0 0.0
ZW C=FLOW
*
KKYOMB_9
KM COMBINE FLOW AT EXISTING ROAD CULVERT
HC 5
ZW C=FLOW
*
KKU RR10
KM ROUTE FLOW THROUGH SHED CUS17F TO CNFLUENCE WITH CURRY CK TRIB PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH
GRADING
RS 1 FLOW -1
SV 0.00 1.70 2.89 7.91 11.87 15.02 17.82 19.55
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KK SS17F
KM AREA=18.0 AC
BA0.0281
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.016 0.10 0.1200 84.405
UK 150 0.0100 0.400 100.00 95
UK 290 0.0074 0.240 5
RD 1800 0.0010 0.055 TRAP 10.0 5.0
RD 600 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KK SS17B
KM AREA=33.9 AC
BA0.0529
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.998 0.10 0.1200 84.405
UK 150 0.0100 0.400 95
UK 290 0.0074 0.240 5
RD 1200 0.0010 0.020 TRAP 10.0 5.0
RD 600 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KK SS17D
KM AREA = 15.4
BA0.0240
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.501 0.10 0.1200 46.994
UK 150 0.0100 0.400 7
UK 115 0.0100 0.240 93
RD 500 0.0016 0.015 CIRC 2.0 0.0
RD 500 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KK SS17E
KM AREA=7.6 AC
BA0.0119
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.967 0.10 0.1200 46.994
UK 150 0.0100 0.400 100.00 7
RD 550 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*

```

KKYOMB10
KM COMBINE ALL FLOWS FROM CURRY CK AT CONFLUENCE W/ CURRY TRIB
HC 5
ZW C=FLOW
*
KKCUS17C
KM AREA = 10.7
BA0.0629
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.004
UK 150 0.0100 0.400 100.00
RD 2500 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KKYUS17C
KM 0
HC 2
ZW C=FLOW
*
KKCU2A1A
KM AREA = 49.0
BA0.0765
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 3.162 0.10 0.1200 46.652
UK 150 0.0100 0.400 28
UK 400 0.0050 0.240 72
RD 1000 0.0016 0.015 CIRC 2.0 0.0
RD 1000 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KKVR_201
KM ROUTE FLOW THROUGH SHED CU2A1D
RD 1000 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KK S2A1B
KM AREA = 13.0 AC
BA0.0203
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.953 0.10 0.1200 49.962
UK 150 0.0100 0.400 62
UK 178 0.0096 0.240 38
RD 700 0.0016 0.015 CIRC 2.0 0.0
RD 500 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KKYOM201
KM COMBINE FLOW FROM CU2A1A & CU2A1B
HC 2
ZW C=FLOW
*
KK CWP7
KM Curry Creek Basin CU2A (5-03)
BA0.0780
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 41.023
UK 300 0.0200 0.350 100.00
RD 600 0.0020 0.024 .0200 TRAP 1.0 0.0
RD 1800 0.0010 0.024 TRAP 4.0 1.0
ZW C=FLOW
*
*
* KKKU DB3
* KM " Det Basin C (1.6 acre surface area;1-18";1-24" outfalls)
* RS 1 FLOW -1
* SV 0.00 1.50 3.10 4.60 6.30 7.90
* SQ 0.0 2.0 12.0 23.0 40.0 90.0
* ZW C=FLOW
*
KKVR_203
KM ROUTE FLOW TO CURRY CK TRIB
RD 500 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KK YCWP7

```

KM 0
 HC 2
 ZW C=FLOW
 *
 KK S2A1C
 KM AREA = 20.0 AC
 BAO.0313
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.811 0.10 0.1200 45.362
 UK 150 0.0100 0.400 6
 UK 127 0.0100 0.240 94
 RD 300 0.0016 0.015 CIRC 2.0 0.0
 RD 1500 0.0010 0.015 CIRC 4.0 0.0
 ZW C=FLOW
 *

KKYOM202
 KM COMBINE FLOW WITH CU2A1C
 HC 2
 ZW C=FLOW
 *
 KKVR202A
 KM ROUTE FLOW THROUGH SHED CU2A1D
 RD 800 0.0010 0.020 TRAP 10.0 5.0
 ZW C=FLOW
 *

KK S2A1D
 KM AREA=39.5 AC
 BAO.0617
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.005
 UK 150 0.0100 0.400 100.00
 RD 2400 0.0010 0.020 TRAP 10.0 5.0
 ZW C=FLOW
 *

KKYM202B
 KM COMBINE FLOW WITH RR202A
 HC 2
 ZW C=FLOW
 *
 KKVR202B
 KM ROUTE FLOW TO MARKET STREET
 RD 700 0.0010 0.020 TRAP 10.0 5.0
 ZW C=FLOW
 *

KK S2A1E
 KM AREA = 3.7 AC
 BAO.0057
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 5.000 0.10 0.1200 40.000
 UK 150 0.0100 0.400 8
 UK 136 0.0100 0.240 92
 RD 350 0.0016 0.015 CIRC 2.0 0.0
 ZW C=FLOW
 *

KK S2A1F
 KM AREA=4.0 AC
 BAO.0062
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.011 0.10 0.1200 40.000
 UK 150 0.0100 0.400 99
 UK 160 0.0100 0.240 1
 RD 700 0.0010 0.020 TRAP 10.0 5.0
 ZW C=FLOW
 *

KKYM202C
 KM COMBINE
 HC 2
 ZW C=FLOW
 *
 KKYOM203
 KM COMBINE FLOW AT MARKET STREET
 HC 2
 ZW C=FLOW
 *
 KKVR202

KM ROUTE FLOW TO CURRY CK CHANNEL UP/S OF WEST SIDE DRIVE
RD 1100 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW

*
KK S2AB2
KM AREA = 5.3 AC
BAO.0083

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.895 0.10 0.1200 43.348
UK 150 0.0100 0.400 13
UK 132 0.0100 0.240 87
RD 450 0.0016 0.015 CIRC 2.0 0.0
ZW C=FLOW

*
KK S2AB3
KM AREA = 7.4 AC
BAO.0116

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.418 0.10 0.1200 40.056
UK 150 0.0100 0.400 2
UK 132 0.0100 0.240 98
RD 550 0.0016 0.015 CIRC 2.0 0.0
ZW C=FLOW

*
KK S2AB4
KM AREA = 7.3 AC
BAO.0114

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.053
UK 150 0.0100 0.400 100.00
RD 1100 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW

*
KKYCM2AB
KM COMBINE FLOW
HC 4
ZW C=FLOW

*
KK CWP8
KM Basin CU2A (5-03)
BAO.0992

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 40.000
UK 300 0.0200 0.350 100.00
RD 610 0.0070 0.024 .0200 TRAP 1.0 0.0
RD 2800 0.0010 0.024 TRAP 4.0 1.0
ZW C=FLOW

*
* KKKU DB4
* KM " Det Basin B for WP8 (1.6 acre surface area;1-18";1-24" outfalls)
* RS 1 FLOW -1
* SV 0.00 1.50 3.10 4.60 6.30 7.90
* SQ 0.0 2.0 12.0 23.0 40.0 90.0
* ZW C=FLOW

*
KK S2AC2
KM AREA = 3.3 AC
BAO.0052

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.132
UK 150 0.0100 0.400 100.00
RD 600 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW

*
KKYS2AC2
KM 0
HC 2
ZW C=FLOW

*
KK S2AC1
KM AREA = 1.9 AC
BAO.0029

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 3.286 0.10 0.1200 40.016
 UK 150 0.0100 0.400 3
 UK 130 0.0100 0.240 97
 RD 350 0.0016 0.015 CIRC 2.0 0.0
 ZW C=FLOW
 *
 KKYS2AC1
 KM 0
 HC 2
 ZW C=FLOW
 *
 KK CWP8A
 KM PORTION OF EXISTING SHED CU2A8- A=17.5 AC ASSUMED DEVELOPED MDResidential
 BAO.0274
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 50.000
 UK 150 0.0050 0.240 100.00
 RD 1100 0.0016 0.015 CIRC 6.0 0.0
 ZW C=FLOW
 *
 KK S2AC3
 KM AREA = 16.6 AC
 BAO.0259
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.955 0.10 0.1200 47.340
 UK 150 0.0100 0.400 19
 UK 132 0.0100 0.240 81
 RD 350 0.0016 0.015 CIRC 2.0 0.0
 RD 400 0.0010 0.015 CIRC 5.0 0.0
 ZW C=FLOW
 *
 KKYS2AC3
 KM 0
 HC 3
 ZW C=FLOW
 *
 KK YCOMB
 KM 0
 HC 2
 ZW C=FLOW
 *
 KK S2AC6
 KM AREA = 22.4
 BAO.0350
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.025 0.10 0.1200 82.339
 UK 150 0.0100 0.400 97
 UK 77 0.0146 0.240 3
 RD 2300 0.0010 0.055 TRAP 10.0 5.0
 ZW C=FLOW
 *
 KKYS2AC6
 KM 0
 HC 2
 ZW C=FLOW
 *
 KK S2AC4
 KM AREA = 10.1
 BAO.0157
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 40.000
 UK 130 0.0100 0.240 100.00
 RD 700 0.0016 0.015 CIRC 2.0 0.0
 ZW C=FLOW
 *
 KKYS2AC4
 KM 0
 HC 2
 ZW C=FLOW
 *
 KK S2A9
 KM AREA = 36.7 AC

BA0.0573

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.723	0.10	0.1200	55.939		
UK	150	0.0100	0.400	6				
UK	98	0.0104	0.240	94				
RD	1000	0.0016	0.015		CIRC	2.0	0.0	
RD	1550	0.0010	0.015		CIRC	5.0	0.0	

ZW C=FLOW

*

KK YS2A9

KM 0

HC 2

ZW C=FLOW

*

KK S2AB1

KM AREA = 53.6 AC

BA0.0523

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.440	0.10	0.1200	57.700		
UK	150	0.0100	0.400	29				
UK	127	0.0100	0.240	71				
RD	1200	0.0016	0.015		CIRC	2.0	0.0	
RD	300	0.0010	0.015		CIRC	4.0	0.0	

ZW C=FLOW

*

KK S2A1G

KM 0

BA0.0070

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	2.201	0.10	0.1200	82.720		
UK	600	0.0150	0.400	88				
UK	68	0.0173	0.240	12				
RD	500	0.0010	0.050		TRAP	2.0	2.0	

ZW C=FLOW

*

KKYS2A1G

KM 0

HC 2

ZW C=FLOW

*

KK S2AC5

KM AREA = 20.9

BA0.0571

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	2.823	0.10	0.1200	46.029		
UK	150	0.0100	0.400	40				
UK	121	0.0100	0.240	60				
RD	400	0.0016	0.015		CIRC	2.0	0.0	
RD	450	0.0010	0.015		CIRC	5.0	0.0	

ZW C=FLOW

*

KKYS2AC5

KM 0

HC 2

ZW C=FLOW

*

KK S2AC7

KM 0

BA0.0087

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.1200	42.931					
UK	124	0.0100	0.240	100.00				
RD	500	0.0010	0.050		TRAP	2.0	2.0	

ZW C=FLOW

*

KK S2A8

KM AREA = 88.4

BA0.1294

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.991	0.10	0.1200	46.529		
UK	150	0.0100	0.400	6				

UK 118 0.0100 0.240 94
 RD 600 0.0016 0.015 CIRC 2.0 0.0
 RD 3500 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*

KKYM203F

KM COMBINE FLOW

HC 2

ZW C=FLOW

*

KKYOM204

KM COMBINE TOTAL FLOW AT WEST SIDE DRIVE

HC 2

ZW C=FLOW

*

KKUR_207

KM ROUTE FLOW THROUGH SHED CU2A7B PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CHANNEL GRADING

RS 1 FLOW -1
 SV 0.00 2.28 4.19 7.75 10.94 13.89 16.81 21.00
 SQ 0.0 100.0 200.0 300.0 400.0 500.0 600.0

ZW C=FLOW

*

KK S2A7A

KM AREA = 37.3 AC

BA0.0583

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 4.911 0.10 0.1200 52.734
 UK 150 0.0100 0.400 14
 UK 112 0.0100 0.240 86
 RD 1100 0.0016 0.015 CIRC 2.0 0.0
 RD 1100 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*

KK S2A7

KM AREA = 17.7

BA0.0276

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 2.039 0.10 0.1200 83.349
 UK 150 0.0100 0.400 94
 UK 97 0.0106 0.240 6
 RD 400 0.0010 0.055 TRAP 10.0 5.0
 RD 1500 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*

KK S2A7B

KM AREA = 14.4 AC

BA0.0224

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 4.987 0.10 0.1200 40.000
 UK 150 0.0100 0.400 36
 UK 154 0.0100 0.240 64
 RD 550 0.0016 0.015 CIRC 2.0 0.0
 RD 450 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*

KK S2A7C

KM AREA = 51.9 AC

BA0.0812

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 5.000 0.10 0.1200 45.120
 UK 150 0.0100 0.400 5
 UK 132 0.0100 0.240 95
 RD 450 0.0016 0.015 CIRC 2.0 0.0
 RD 2000 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*

KKYOM205

KM COMBINE FLOW FROM CU2A7 & CU2A7A

HC 4

ZW C=FLOW

*

KKYOM206

KM COMBINE FLOW WITH IN THE CHANNEL

HC 2

ZW C=FLOW

*
*

KKUR_208

KM ROUTE FLOW THROUGH SHED CU2A4 TO WATT AVE PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CHANNEL GRADING

RS	1	FLOW	-1						
SV	0.00	1.47	2.86	6.41	11.52	17.48	24.82	36.80	
SQ	0.0		100.0	200.0	300.0	400.0	500.0	600.0	

ZW C=FLOW

*

KK S2A4A

KM AREA = 41.5 AC

BA0.0648

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0700	4.911	0.10	0.1200	56.440			
UK	150	0.0100	0.400	8					
UK	98	0.0104	0.240	92					
RD	1000	0.0016	0.015		CIRC	2.0	0.0		
RD	1400	0.0010	0.015		CIRC	4.0	0.0		

ZW C=FLOW

*

KK S2A4

KM AREA = 6.7 AC

BA0.0105

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0700	2.124	0.10	0.1200	84.481			
UK	150	0.0100	0.400	83					
UK	79	0.0142	0.240	17					
RD	1500	0.0010	0.055		TRAP	10.0	5.0		

ZW C=FLOW

*

KK S2A5

KM AREA = 108.8 AC ASSUMED OFFSITE DEVELOPED MDResidential

BA0.1700

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0700	4.944	0.10	0.1200	49.625			
UK	150	0.0100	0.400	7					
UK	109	0.0100	0.240	93					
RD	1500	0.0016	0.015		CIRC	2.0	0.0		
RD	1300	0.0010	0.015		CIRC	4.0	0.0		

ZW C=FLOW

*

KKYOM208

KM COMBINE FLOW FROM SHEDS CU2A5- CU2A4 & CU2A4A AT WATT AVE

HC 3

ZW C=FLOW

*

KKYOM110

KM COMBINE FLOW AT WATT AVE CULVERT

HC 2

ZW C=FLOW

*

KKUR_210

KM ROUTE FLOW THROUGH SHED CU2A6 TO CONFLUENCE WITH CURRY CK PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CHANNEL GRADING

RS	1	FLOW	-1						
SV	0.00	2.28	3.99	8.58	11.98	15.16	18.06	20.75	25.00
SQ	0.0	50.0	100.0	200.0	300.0	400.0	500.0	600.0	800.0

ZW C=FLOW

*

KK S2A6A

KM AREA = 29.9 AC

BA0.0468

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0700	4.828	0.10	0.1200	40.041			
UK	150	0.0100	0.400	4					
UK	132	0.0100	0.240	96					
RD	600	0.0016	0.015		CIRC	2.0	0.0		
RD	1000	0.0010	0.015		CIRC	4.0	0.0		

ZW C=FLOW

*

KK S2A6B

KM AREA = 10.3 AC

BA0.0161

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.622 0.10 0.1200 40.014
 UK 150 0.0100 0.400 6
 UK 134 0.0100 0.240 94
 RD 1000 0.0016 0.015 CIRC 2.0 0.0
 ZW C=FLOW

*
 KK S2A6
 KM AREA = 37.0 AC
 BAO.0578

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.031
 UK 150 0.0100 0.400 100.00
 RD 400 0.0010 0.055 TRAP 2.0 5.0
 RD 2500 0.0010 0.055 TRAP 10.0 5.0
 ZW C=FLOW

*
 KK S2A6C
 KM AREA = 38.0 AC
 BAO.0594

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.981 0.10 0.1200 47.276
 UK 150 0.0100 0.400 9
 UK 118 0.0100 0.240 91
 RD 650 0.0016 0.015 CIRC 2.0 0.0
 RD 1300 0.0010 0.015 CIRC 4.0 0.0
 ZW C=FLOW

*
 KKYOM211
 KM COMBINE ALL FLOWS FROM CURRY CK TRIB
 HC 5
 ZW C=FLOW

*
 KK S2A2
 KM AREA = 17.6 AC UP/S OF WATT
 BAO.0275

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.000
 UK 200 0.0100 0.400 100.00
 RD 800 0.0100 0.045 TRAP 2.0 10.0
 ZW C=FLOW

*
 KK S2A1
 KM AREA = 8.4 AC UP/S OF WATT
 BAO.0131

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.000
 UK 200 0.0100 0.400 100.00
 RD 900 0.0100 0.045 TRAP 2.0 10.0
 ZW C=FLOW

*
 KK YS2A1
 KM 0
 HC 2
 ZW C=FLOW

*
 KK S2A5C
 KM AREA = 69.3 AC
 BAO.1082

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.030
 UK 200 0.0100 0.400 100.00
 RD 1400 0.0100 0.045 TRAP 2.0 10.0
 ZW C=FLOW

*
 KKYM2A5C
 KM COMBINE FLOWS from cu2a1 & cu2a5c
 HC 2
 ZW C=FLOW

*
 KKYDD100

KM COMBINE ALL FLOWS BEFORE ADDING TO CURRY CK
 HC 2
 ZW C=FLOW
 *
 KKVR_211
 KM ROUTE FLOW TO COMBINATION OF ALL CURRY CREEK FLOW
 RD 1500 0.0010 0.055 TRAP 10.0 5.0
 ZW C=FLOW
 *
 KKYOM212
 KM COMBINE ALL FLOWS FROM CURRY CREEK AT PROJECT WEST BOUNDARY
 HC 2
 ZW C=FLOW
 *
 KKCU2A3A
 KM AREA = 38.2 AC WEST OF WATT
 BAO.0597
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.000
 UK 200 0.0100 0.400 100.00
 RD 1200 0.0100 0.045 TRAP 2.0 10.0
 ZW C=FLOW
 *
 KKY2A3AC
 KM COMBINE FLOW FROM CU2A2 & CU2A3A
 HC 2
 ZW C=FLOW
 *
 KKV2A3AR
 KM ROUTE FLOW THROUGH SHED CU2A
 RD 2300 0.0010 0.055 TRAP 10.0 5.0
 ZW C=FLOW
 *
 KK CU2A
 KM AREA = 485.3 AC NOV 29- 06
 BAO.7583
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000
 UK 600 0.0150 0.400 100.00
 RD 800 0.0100 0.050 .1000 TRAP 1.0 10.0
 RD 5500 0.0010 0.050 .2000 TRAP 5.0 10.0
 ZW C=FLOW
 *
 KKYCCU2A
 KM COMBINED CU2A & RCH3 (2.64 SM + 0.1014 + 0.0706)+ S17BR
 HC 3
 ZW C=FLOW
 *
 KK CWP1
 KM AREA REVISED 6-05
 BAO.1447
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000 0.10 0.0600 100.000
 UK 300 0.0080 0.350 51
 UK 150 0.0100 0.110 49
 RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
 RD 2050 0.0010 0.015 TRAP 2.0 2.0
 ZW C=FLOW
 *
 KKV R3
 KM main channel through to c-5
 RD 1700 0.0030 0.035 TRAP 3.5 10.0
 ZW C=FLOW
 *
 KK CWP2
 KM AREA REVISED 6-05
 BAO.1969
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 100.000 0.10 0.0600 2.000
 UK 150 0.0080 0.110 27
 UK 300 0.0100 0.350 73
 RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
 RD 2000 0.0010 0.020 TRAP 2.0 2.0
 ZW C=FLOW
 *

KKY C5
 KM node combines area 1 & area 2
 HC 2
 ZW C=FLOW
 *

KKV R4
 KM main channel from C-WP2
 RD 1800 0.0020 0.035 TRAP 3.5 10.0
 ZW C=FLOW
 *

KK CWP4
 KM Revise Date (5-03)
 BA0.3627
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000 0.10 0.0600 100.000
 UK 300 0.0100 0.350 54
 UK 200 0.0200 0.110 46
 RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
 RD 2600 0.0010 0.020 TRAP 2.0 2.0
 ZW C=FLOW
 *

KKV R7
 KM Main channel to C-6
 RD 1935 0.0021 0.035 TRAP 3.5 10.0
 ZW C=FLOW
 *

KK CWP3
 KM AREA REVISED 6-05
 BA0.3000
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.000 0.10 0.0600 100.000
 UK 300 0.0100 0.350 54
 UK 200 0.0100 0.110 46
 RD 2000 0.0020 0.020 TRAP 2.0 2.0
 ZW C=FLOW
 *

KKY C6
 KM combines node C-5 and C-WP4
 HC 3
 ZW C=FLOW
 *

KKU DBC6
 KM Detention Storage (step boxes)(2-5'x 10';2-4'x 10' Box Culverts)
 RS 1 FLOW -1
 SV 0.00 0.70 3.50 10.30 22.80 35.00 42.50 69.70 88.00
 SQ 0.0 40.0 175.0 350.0 600.0 800.0 850.0 950.0 1000.0
 ZW C=FLOW
 *

KKV R8
 KM main channel through area five
 RD 3700 0.0010 0.035 TRAP 3.5 10.0
 ZW C=FLOW
 *

KK CWP6
 KM Revise Date (5-03) (194.5 ac)
 BA0.3039
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0900 2.000 0.10 0.0600 100.000
 UK 300 0.0100 0.350 72
 UK 200 0.0080 0.110 28
 RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
 RD 2000 0.0010 0.020 TRAP 2.0 2.0
 ZW C=FLOW
 *

KKY C8
 KM node combines node c-7 and area 6
 HC 2
 ZW C=FLOW
 *

KKU DBC8
 KM " Detention Storage (3-36" w/weir)"
 RS 1 FLOW -1
 SV 0.00 2.10 12.00 21.60 30.20 38.80 42.50 52.40 64.00
 SQ 0.0 30.0 120.0 165.0 200.0 500.0 900.0 950.0 1050.0
 ZW C=FLOW

```

*
KKV R13
KM main channel EXIT FROM NODE C-8
RD 2000 0.0010 0.035 TRAP 5.0 5.0
ZW C=FLOW
*
KK SP9A
KM AREA = 19.5 AC
BA0.0305
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.948 0.10 0.1200 51.361
UK 150 0.0100 0.400 2
UK 100 0.0100 0.240 98
RD 1200 0.0016 0.015 CIRC 3.0 0.0
RD 650 0.0010 0.015 CIRC 5.0 0.0
ZW C=FLOW
*
KK CWP9
KM AREA REVISED 149.0 ac
BA0.2329
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000 0.10 0.0700 84.266
UK 300 0.0080 0.400 98
UK 400 0.0050 0.240 2
RD 1450 0.0070 0.050 TRAP 0.0 1.0
RD 3600 0.0010 0.050 TRAP 2.0 2.0
ZW C=FLOW
*
KKYCCWP9
KM COMBINE FLOW FROM CWP9A & CWP9 AT PLEASANT GROVE BLVD
HC 2
ZW C=FLOW
*
KKV R913
KM Route shed through WP-13
RD 2000 0.0023 0.035 TRAP 10.0 10.0
ZW C=FLOW
*
KK CWP13
KM AEEA REVISED 6-05
BA0.0942
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000 0.10 0.0700 100.000
UK 300 0.0080 0.350 67
UK 150 0.0080 0.110 33
RD 450 0.0070 0.018 .0300 TRAP 0.0 1.0
RD 2400 0.0010 0.020 TRAP 2.0 2.0
ZW C=FLOW
*
KKY C9
KM Combines Area WP9 & WP13
HC 2
ZW C=FLOW
*
KKY C13
KM 0
HC 2
ZW C=FLOW
*
KK CWP14
KM AERA REVISED 6-05
BA0.1525
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000 0.10 0.0600 100.000
UK 300 0.0080 0.350 98
UK 150 0.0100 0.110 2
RD 1600 0.0020 0.020 TRAP 2.0 2.0
ZW C=FLOW
*
KKYCWP14
KM COMBINE FLOW
HC 2
ZW C=FLOW
*
KK CU2B1

```

KM 53.9 ACRES AREA REVISED 6-05

BA0.1150

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0600 2.000

UK 200 0.0050 0.400 100.00

RD 1900 0.0080 0.050 TRAP 2.0 10.0

ZW C=FLOW

*

KKCU2B1A

KM 29.2 ACRES AREA REVISED 6-05

BA0.0456

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0600 2.000

UK 200 0.0050 0.400 100.00

RD 1600 0.0080 0.050 TRAP 2.0 10.0

ZW C=FLOW

*

KKY C10

KM COMBINE FLOW AT WATT AVE & PLEASANT GROVE BLVD

HC 3

ZW C=FLOW

*

KKU RR1

KM ROUTE FLOW FROM C-10- 3500FT DNS TO CP-1

RS 1 FLOW -1

SV 0.00 4.60 7.40 10.00 15.70 20.60 24.50 37.60 45.50

SQ 0.0 100.0 216.0 300.0 405.0 500.0 559.0 785.0 900.0

ZW C=FLOW

*

KK CU2B2

KM AREA = 214.8 AC NOV 29- 06

BA0.3356

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0600 2.000

UK 600 0.0050 0.400 100.00

RD 2700 0.0067 0.050 .3800 TRAP 10.0 2.0

ZW C=FLOW

*

KKY CP1

KM COMBINE CU2B2 WITH FLOW AT CP-1

HC 2

ZW C=FLOW

*

KKU RR2

KM ROUTE FLOW FROM CP-1 TO CU2A+

RS 1 FLOW -1

SV 0.00 16.60 25.90 34.10 51.70 57.30 71.20 91.80 95.40

SQ 0.0 150.0 245.0 320.0 467.0 500.0 634.0 872.0 900.0

ZW C=FLOW

*

KK CU2B3

KM 221.1 ACRES

BA0.3455

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0600 2.000

UK 600 0.0050 0.400 100.00

RD 6300 0.0012 0.050 TRAP 4.0 2.0

ZW C=FLOW

*

KKYCU3B+

KM COMBINE CU3B & RR2

HC 2

ZW C=FLOW

*

KKYCU2A+

KM 0

HC 2

ZW C=FLOW

*

KKU RR3

KM ROUTE FLOW FROM CU2A+ TO CU2C1- 2800 FT

RS 1 FLOW -1

```

SV 0.00 7.70 23.20 32.80 57.40 59.60 66.10 85.90 95.10
SQ 0.0 250.0 491.0 700.0 1369.0 1400.0 1566.0 2192.0 2500.0
ZW C=FLOW
*
KK CU2C1
KM 186.4 ACRE @ COUNTRY ACRE LN
BA0.2913
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0050 0.400 100.00
RD 3700 0.0016 0.100 .2500 TRAP 10.0 20.0
ZW C=FLOW
*
KKY CP2
KM COMBINE RUNOFF FROM CUUC1 WITH FLOW AT CP-2
HC 2
ZW C=FLOW
*
*
KKU RR4
KM ROUTE FLOW FROM CP-2 TO CP-3- 2800 FT DNS
RS 1 FLOW -1
SV 0.00 20.70 34.00 44.30 64.00 70.70 81.80 112.30 125.60
SQ 0.0 250.0 500.0 700.0 1140.0 1300.0 1566.0 2185.0 2500.0
ZW C=FLOW
*
KK CU2C2
KM " 146.6 ACRES COUNTRY ACRES LN TO "LANDING STRIP"
BA0.2290
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 100 0.0050 0.400 100.00
RD 4200 0.0090 0.050 TRAP 15.0 3.0
ZW C=FLOW
*
KKY CP3
KM COMBINE RUNOFF FROM CU2C2 WITH FLOW AT CP-3
HC 2
ZW C=FLOW
*
*
KKU RR5
KM ROUTE FLOW FROM CP-3 TO BREWER ROAD 4200 FT DNS
RS 1 FLOW -1
SV 0.00 7.00 11.10 12.20 15.80 17.20 19.80 27.20 30.10
SQ 0.0 260.0 513.0 720.0 1167.0 1350.0 1601.0 2230.0 2500.0
ZW C=FLOW
*
KK CU2C3
KM " 216.6 ACRES "LANDING STRIP" TO BREWER RD"
BA0.3384
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0050 0.400 100.00
RD 1900 0.0012 0.100 .2000 TRAP 300.0 1.0
RD 800 0.0010 0.050 .1500 TRAP 2.0 2.0
ZW C=FLOW
*
KKYCU2C+
KM 0
HC 2
ZW C=FLOW
*
KKCWP11A
KM Curry Creek Basin
BA0.0750
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000 0.10 0.0600 100.000
UK 300 0.0100 0.350 60
UK 150 0.0200 0.110 40
RD 500 0.0040 0.024 .0800 TRAP 1.0 5.0
RD 900 0.0010 0.024 TRAP 2.0 10.0
ZW C=FLOW
*
*

```

KKU DB5
 KM " Det Basin A for C-WP11A (1.0 acre surface area; 2-18" outfalls)"
 RS 1 FLOW -1
 SV 0.00 0.70 1.80 2.80 3.90 5.10 6.50
 SQ 0.0 2.0 7.0 15.0 24.0 55.0 90.0
 ZW C=FLOW
 *
 KK8 R11
 KM 0
 RD
 RC 0.045 0.045 0.045 1800 0.0028 1000
 RX 0 0 1000 1013 1027 1040 2040 2040 0 0
 RY1020.0 1000.0 990.0 984.0 984.0 990.0 1000.0 1020.0 0.0 0.0
 ZW C=FLOW
 *
 KK CWP11
 KM Open Space
 BA0.1230
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000 0.10 0.0600 100.000
 UK 300 0.0100 0.350 95
 UK 200 0.0100 0.110 5
 RD 1900 0.0036 0.050 .0800 TRAP 3.5 10.0
 RD 1300 0.0010 0.050 TRAP 3.5 10.0
 ZW C=FLOW
 *
 KKY CB11
 KM 0
 HC 2
 ZW C=FLOW
 *
 KKVR1112
 KM Route shed through WP-12
 RD 1200 0.0036 0.035 TRAP 10.0 10.0
 ZW C=FLOW
 *
 KK CWP12
 KM 0
 BA0.0678
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 100.000 0.10 0.0600 2.000
 UK 600 0.0100 0.110 2
 UK 600 0.0100 0.400 98
 RD 1400 0.0085 0.035 TRAP 3.5 10.0
 ZW C=FLOW
 *
 KKYC1112
 KM combines WP11 and WP12 at Future Watt Ave. 170.3 ac=.266 sq mi
 HC 2
 ZW C=FLOW
 *
 *
 KKU RCU3
 KM ROUTE FLOW FROM C11-12 TO CU3B 5300 FT DNS
 RS 1 FLOW -1
 SV 0.00 2.90 14.30 20.30 27.90 30.80 32.90 41.70 47.90
 SQ 0.0 30.0 62.0 90.0 159.0 190.0 214.0 298.0 350.0
 ZW C=FLOW
 *
 KK CU3A
 KM 247.3 acres
 BA0.3864
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000
 UK 600 0.0050 0.400 100.00
 RD 2000 0.0100 0.050 .2500 TRAP 10.0 10.0
 RD 1500 0.0010 0.050 TRAP 5.0 5.0
 ZW C=FLOW
 *
 KKYCU3AC
 KM 0
 HC 2
 ZW C=FLOW
 *
 KK CU3B
 KM 307.4 ACRES
 BA0.4803

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0010 0.400 100.00
RD 1800 0.0013 0.050 TRAP 200.0 1.0
RD 2600 0.0010 0.050 TRAP 5.0 3.0 YES
ZW C=FLOW
*
KK CU3C
KM 110.3 acres
BA0.1723
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0010 0.400 100.00
RD 2900 0.0076 0.050 TRAP 10.0 10.0
ZW C=FLOW
*
KKYCC3B+
KM COMBINE RCU3B+CU3B&CU3A+CU3C;170.3 + 307.4+247.3 + 110.3 = 835.3=1.3052 sq mi
HC 2
ZW C=FLOW
*
*
KKURC3B+
KM ROUTE FLOW FROM CC3B+ TO CU3D 3800 FT DNS
RS 1 FLOW -1
SV 0.00 8.20 63.30 69.10 78.30 81.60 92.00 110.10 143.60
SQ 0.0 50.0 97.0 150.0 263.0 300.0 345.0 445.0 550.0
ZW C=FLOW
*
KK CU3D
KM 465.8 acres ROUTES & COMBINES 465.8+835.3=1-301.1 ac = 2.033 Sq. Mi.
BA0.7278
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0010 0.400 100.00
RD 4400 0.0010 0.050 .4000 TRAP 7.0 3.0
RD 1500 0.0010 0.060 TRAP 100.0 2.0 YES
ZW C=FLOW
*
*
KKURC3D+
KM ROUTE FLOW CU3D TO CU3E+ 3600 FT DNS
RS 1 FLOW -1
SV 0.00 3.90 7.50 11.00 15.00 16.60 18.40 22.50 24.60
SQ 0.0 70.0 140.0 220.0 321.0 375.0 428.0 570.0 650.0
ZW C=FLOW
*
KK CU3E
KM 111.8 acres
BA0.1747
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0010 0.400 100.00
RD 3600 0.0010 0.050 TRAP 18.0 8.0
ZW C=FLOW
*
KKYCU3E+
KM COMBINE CU3E & RC3D+ 111.8+1301.1=1412.9 ac = 2.2077 Sq. Mi.
HC 2
ZW C=FLOW
*
*
KKURC3E+
KM ROUTE FLOW FROM CU3E+ TO CU3F+
RS 1 FLOW -1
SV 0.00 2.80 4.60 7.10 9.40 10.20 11.40 14.10 15.00
SQ 0.0 75.0 153.0 200.0 351.0 400.0 466.0 618.0 700.0
ZW C=FLOW
*
KK CU3F
KM 641.7ACRES
BA1.0026
PB
* PI
BF -1 -0.001 1.50

LU 0.10 0.0600 2.000
 UK 600 0.0010 0.400 100.00
 RD 4100 0.0062 0.050 .5000 TRAP 15.0 3.0
 RD 4400 0.0010 0.050 TRAP 12.0 2.0
 ZW C=FLOW
 *
 KKYCC3F+
 KM COMBINE RC3F+ & CU3E+ (602.7+ 1412.9=2015.6 ac = 3.149 Sq. Mi.)
 HC 2
 ZW C=FLOW
 *
 *
 KKURC3F+
 KM ROUTE CC3F+ TO CU3+ 2015.6 ac=3.149 Sq. Mi.
 RS 1 FLOW -1
 SV 0.00 17.20 30.90 54.40 75.50 81.00 95.10 123.30 131.30
 SQ 0.0 80.0 164.0 300.0 410.0 470.0 565.0 780.0 850.0
 ZW C=FLOW
 *
 KK CU3G
 KM 200.0 acres
 BA0.3125
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000
 UK 600 0.0010 0.400 100.00
 RD 4500 0.0010 0.050 TRAP 70.0 90.0
 ZW C=FLOW
 *
 KK CU3HA
 KM 57.4 acres
 BA0.0897
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000
 UK 600 0.0050 0.400 100.00
 RD 1500 0.0010 0.050 TRAP 10.0 2.0
 ZW C=FLOW
 *
 KK CU3HB
 KM 106.1 acres
 BA0.1658
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000
 UK 300 0.0010 0.400 100.00
 RD 4000 0.0034 0.050 TRAP 100.0 50.0 YES
 ZW C=FLOW
 *
 KKY CU3
 KM COMBINE CU3H & CU3G & RC3F+ (106.1+200.0+2015.6=2321.7=3.627 Sq. Mi.)
 HC 3
 ZW C=FLOW
 *
 KKYCU3C+
 KM COMBINE CU3 & CU2C+ (2321.7+
 HC 2
 ZW C=FLOW
 *
 *
 KKU RCH8
 KM 0
 RS 1 FLOW -1
 SV 0.00 51.00 115.50 194.10 281.70 287.90 335.40 390.50 400.20
 SQ 0.0 400.0 621.0 1000.0 1387.0 1500.0 1900.0 2682.0 3200.0
 ZW C=FLOW
 *
 KK CU4
 KM 0
 BA3.5800
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0600 2.000
 UK 600 0.0150 0.400 100.00
 RD 800 0.0100 0.050 .1000 TRAP 1.0 10.0
 RD 8500 0.0010 0.050 .2000 TRAP 5.0 10.0
 ZW C=FLOW
 *
 KK CU5

```

KM 0
BA0.2800
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0150 0.400 100.00
RD 500 0.0100 0.050 .1000 TRAP 1.0 10.0
RD 2500 0.0010 0.050 TRAP 5.0 10.0
ZW C=FLOW
*
KKY CU5+
KM 0
HC 3
ZW C=FLOW
*
KK8 RCH9
KM 0
RD
RC 0.040 0.040 0.040 6000 0.0016 1000
RX 0 0 1200 1210 1230 1240 2440 2440 0 0
RY1020.0 1000.0 995.0 990.0 990.0 995.0 1000.0 1020.0 0.0 0.0
ZW C=FLOW
*
KK CU6
KM 0
BA0.8700
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0150 0.400 100.00
RD 500 0.0100 0.050 .1000 TRAP 1.0 10.0
RD 1500 0.0010 0.050 .2000 TRAP 5.0 10.0
ZW C=FLOW
*
KKY CU6+
KM 0
HC 2
ZW C=FLOW
*
ZZ

```

**APPENDIX D
HEC-1 POST-PROJECT
MITIGATED SCENARIO
INPUT FILE**

ID PLACER COUNTY WATERSHED MODEL, PLACER COUNTY, CA
 ID WATERSHED UPDATE MODELS - DRAFT ULT BUILDOUT
 ID DRAFT MODEL FOR HYD ROUTING - HEC1VOLS UTILITY
 ID CESI 1401/25/2129

ID
 IT 5 01JAN97 0 600
 IO 5 0
 IN 5 0 0

*DIAGRAM

*
 *

KK CUS3G
 KM AREA EAST OF FIDDYMENT ROAD AREA= 281.3 AC
 BA.43953

PB

* PI

BF	-1	-0.001	1.50					
LU	0.1	0.07	2	0.10	0.1200	41		
UK	150	0.01	0.4	1				
UK	200	0.0050	0.240	99				
RD	3000	0.0010	0.015		CIRC	3.0	0.0	
RD	2300	0.0005	0.015		CIRC	5.0	0.0	

ZW C=FLOW

*

KK SS3
 KM AREA=45.2 AC
 BA0.0707

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.982	0.10	0.1200	56.659		
UK	150	0.0100	0.400	5				
UK	96	0.0108	0.240	95				
RD	800	0.0010	0.015		CIRC	3.0	0.0	
RD	2100	0.0010	0.015		CIRC	5.0	0.0	

ZW C=FLOW

*

KK SS3B
 KM AREA= 16.1 AC
 BA0.0252

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	2.089	0.10	0.1200	84.855		
UK	300	0.0100	0.400	94				
UK	89	0.0122	0.240	6				
RD	400	0.0010	0.055		TRAP	2.0	10.0	
RD	1200	0.0010	0.055		TRAP	10.0	5.0	

ZW C=FLOW

*

KKYCUS3A
 KM COMBINE FLOW AT FIDDYMENT ROAD
 HC 3

ZW C=FLOW

*

*

KKUCUS3A
 KM ROUTE FLOW FROM FIDDYMENT ROAD TO UPLAND DIRVE PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING

RS	1	FLOW	-1					
SV	0.00	2.12	3.42	7.01	11.20	16.54	20.91	25.87
SQ	0.0	50.0	100.0	250.0	400.0	550.0	700.0	800.0

ZW C=FLOW

*

KK SS3C
 KM AREA=62.8 AC
 BA0.0981

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.997	0.10	0.1200	42.681		
UK	150	0.0100	0.400	7				
UK	130	0.0100	0.240	93				
RD	800	0.0010	0.015		CIRC	3.0	0.0	
RD	1700	0.0010	0.015		CIRC	5.0	0.0	

ZW C=FLOW

*

KK SS3D
 KM AREA= 66.4 AC
 BA0.1037

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.993	0.10	0.1200	65.734		

UK 150 0.0100 0.400 1
 UK 85 0.0130 0.240 99
 RD 850 0.0010 0.015 CIRC 3.0 0.0
 RD 1250 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*
 KK SS3E
 KM AREA= 15.1 AC
 BAO.0236
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.084
 UK 300 0.0100 0.400 100.00
 RD 300 0.0010 0.055 TRAP 2.0 10.0
 RD 1400 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*
 KK SS5
 KM AREA= 20.2 AC
 BAO.0316
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 70.000
 UK 81 0.0138 0.240 100.00
 RD 400 0.0010 0.055 TRAP 2.0 10.0
 RD 700 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*
 KKUCUS3B
 KM COMBINE FLOW AT UPLAND DRIVE
 HC 5
 ZW C=FLOW

*
 KKUCUS3B
 KM ROUTE FLOW THROUGH SHED CUS3E PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING
 RS 1 FLOW -1
 SV 0.00 2.09 3.84 7.89 11.29 14.29 17.00 18.70
 SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0

ZW C=FLOW

*
 KK SS3F
 KM AREA= 73.7 AC
 BAO.1151
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.999 0.10 0.1200 47.061
 UK 150 0.0100 0.400 6
 UK 115 0.0100 0.240 94
 RD 750 0.0010 0.015 CIRC 3.0 0.0
 RD 1200 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*
 KK CUS5A
 KM CUS5A 33.9 AC
 BAO.0530
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.000
 UK 200 0.0050 0.400 100.00
 RD 300 0.0050 0.040 TRAP 2.0 10.0
 RD 1000 0.0010 0.040 TRAP 2.0 10.0

ZW C=FLOW

*
 KK SS4A
 KM AREA= 24.4 AC
 BAO.0381
 PB

* PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.254
 UK 300 0.0100 0.400 100.00
 RD 400 0.0010 0.055 TRAP 10.0 5.0
 RD 1900 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*
 KK SS5C
 KM AREA= 18.0 AC
 BAO.0281

PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 71.093
 UK 79 0.0142 0.240 100.00
 RD 1200 0.0010 0.015 CIRC 3.0 0.0
 ZW C=FLOW
 *
 KKYCUS3E
 KM COMBINE FLOWS
 HC 5
 ZW C=FLOW
 *
 *
 KKUCUS3E
 KM ROUTE FLOW THROUGH SHED CUS4A TO MARKET STREET PUL'S STOTAGE ROUTING - NOV 7- 06 RAS PER CH GRADING
 RS 1 FLOW -1
 SV 0.00 2.74 4.88 11.19 16.85 23.12 31.16 40.16
 SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
 ZW C=FLOW
 *
 KK SS4
 KM AREA= 82.0 AC
 BAO.1281
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 5.000 0.10 0.1200 57.303
 UK 150 0.0100 0.400 5
 UK 95 0.0110 0.240 95
 RD 550 0.0010 0.015 CIRC 3.0 0.0
 RD 2000 0.0010 0.015 CIRC 5.0 0.0
 ZW C=FLOW
 *
 KK SS5D
 KM AREA= 6.2 AC
 BAO.0097
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 5.000 0.10 0.1200 71.792
 UK 150 0.0100 0.400 8
 UK 84 0.0132 0.240 92
 RD 800 0.0010 0.015 CIRC 3.0 0.0
 RD 1000 0.0010 0.015 CIRC 5.0 0.0
 ZW C=FLOW
 *
 KK CUS6B
 KM AREA= 6.6 AC
 BAO.0103
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.971 0.10 0.1200 71.974
 UK 150 0.0100 0.400 29
 UK 400 0.0050 0.240 71
 RD 400 0.0016 0.015 CIRC 2.0 0.0
 RD 400 0.0010 0.015 CIRC 3.0 0.0
 ZW C=FLOW
 *
 KK SS6A
 KM AREA= 3.7 AC
 BAO.0058
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 40.000
 UK 130 0.0100 0.240 100.00
 RD 235 0.0100 0.015 CIRC 3.0 0.0
 ZW C=FLOW
 *
 KK SS6
 KM AREA= 7.8 AC
 BAO.0122
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.425 0.10 0.1200 85.000
 UK 150 0.0100 0.400 88
 UK 115 0.0100 0.240 12
 RD 400 0.0010 0.055 TRAP 10.0 5.0
 RD 500 0.0010 0.055 TRAP 10.0 5.0
 ZW C=FLOW

```

*
KKYCUS4B
KM COMBINE ALL FLOW AT MARKET STREET (EXCLUDING US ROUTING)
HC 5
ZW C=FLOW
*
KKYCUS4A
KM COMBINE ALL FLOW AT MARKET STREET (INCLUDING US ROUTING)
HC 2
ZW C=FLOW
*
*
KKUCUS4A
KM ROUTE FLOW THROUGH SHED CUS6 TO BASELINE ROAD PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING
RS 1 FLOW -1
SV 0.00 1.06 1.95 4.64 8.64 14.19 21.21 26.76
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KK CUS5B
KM CUS5B 94.1 AC
BA0.1470
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000
UK 200 0.0050 0.400 100.00
RD 1800 0.0050 0.040 TRAP 2.0 10.0
RD 1000 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*
KKYCCUS6
KM COMBINE FLOW AT BASELINE ROAD
HC 2
ZW C=FLOW
*
*
KKUUS5BR
KM ROUTE FLOW DN/S OF BASELINE ROAD THROUGH PLACER VINEYARDS
RS 1 FLOW -1
SV 0.00 5.99 10.55 21.67 30.29 37.67 44.57 48.92
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KKCUS11B
KM CUS11B 87.9ac
BA0.1374
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0748 2.000
UK 200 0.0050 0.400 100.00
RD 500 0.0050 0.040 TRAP 2.0 10.0
RD 150 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*
KKY12ACC
KM COMBINE FLOW AT WEST SIDE DRIVE
HC 2
ZW C=FLOW
*
*
KKU12ACR
KM ROUTE FLOW TO BASELINE ROAD CULVERT
RS 1 FLOW -1
SV 0.00 1.80 4.05 10.48 18.00 30.50 45.34 48.82
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KK CUS14
KM CUS14 AREA = 62.1 AC
BA0.0970
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0760 2.000
UK 200 0.0050 0.400 100.00
RD 800 0.0050 0.040 TRAP 2.0 10.0
RD 900 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*
KKYCUS14
KM COMBINE FLOW UP/S OF BASELINE ROAD CULVERT

```

HC 2
ZW C=FLOW
*
*

KKU RR_7
KM ROUTE FLOW THROUGH SHED CUS15C FROM BASELINE RD TO WATT AVE PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH
GRADING

RS	1	FLOW	-1					
SV	0.00	0.77	1.31	2.91	4.96	7.52	11.54	15.48
SQ	0.0	50.0	100.0	250.0	400.0	550.0	700.0	800.0

ZW C=FLOW
*

KKCUS16B
KM
BA0.0208
PB

* PI
BF -1 -0.001 1.50
LU 0.10 0.0720 2.000
UK 200 0.0050 0.400 100.00
RD 900 0.0050 0.040 TRAP 2.0 10.0
RD 900 0.0010 0.040 TRAP 2.0 10.0
ZW C=FLOW
*

KKYUS16B
KM
HC 2
ZW C=FLOW
*

KK SS12A
KM AREA = 18.1 AC
BA0.0283
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.1200 70.939
UK 79 0.0142 0.240 100.00
RD 400 0.0016 0.015 CIRC 2.0 0.0
RD 900 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*

KK SS13A
KM AREA=141.0 AC
BA0.2204
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 5.000 0.10 0.1200 55.922
UK 150 0.0100 0.400 6
UK 99 0.0102 0.240 94
RD 800 0.0010 0.015 CIRC 3.0 0.0
RD 4400 0.0010 0.015 CIRC 5.0 0.0
ZW C=FLOW
*

KKYSS13A
KM 0
HC 2
ZW C=FLOW
*

KK SS15
KM AREA= 6.6 AC
BA0.0104
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.585 0.10 0.1200 84.484
UK 300 0.0100 0.400 69
UK 89 0.0122 0.240 31
RD 400 0.0010 0.055 TRAP 2.0 10.0
RD 600 0.0010 0.055 TRAP 10.0 10.0
ZW C=FLOW
*

KK YSS15
KM 0
HC 3
ZW C=FLOW
*

KKCUS17A
KM CUS17A AREA=38.2 AC
BA0.0597
PB

* PI
BF -1 -0.001 1.50

LU 0.10 0.0700 2.000
 UK 200 0.0050 0.400 100.00
 RD 500 0.0050 0.040 TRAP 2.0 10.0
 RD 500 0.0010 0.040 TRAP 2.0 10.0

ZW C=FLOW

*

KKV RR_9

KM ROUTE FLOW FROM BASELINE ROAD TO MAIN CHANNEL

RD 1300 0.0010 0.055 TRAP 5.0 5.0

ZW C=FLOW

*

KK SS15B

KM AREA= 5.7 AC

BA0.0089

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 2.724 0.10 0.1200 72.173
 UK 150 0.0100 0.400 1
 UK 78 0.0144 0.240 99
 RD 400 0.0016 0.015 CIRC 2.0 0.0
 RD 400 0.0010 0.015 CIRC 4.0 0.0 YES

ZW C=FLOW

*

KKYOMB_7

KM COMBINE FLOW DN/S OF WATT AVE

HC 2

ZW C=FLOW

*

*

KKU RR_8

KM ROUTE FLOW THROUGH SHED CUS15B PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH GRADING

RS 1 FLOW -1
 SV 0.00 2.17 3.69 9.34 14.20 18.27 21.98 24.30
 SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0

ZW C=FLOW

*

KK SS15A

KM AREA=74.8 AC

BA0.1168

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 4.996 0.10 0.1200 57.177
 UK 150 0.0100 0.400 13
 UK 94 0.0112 0.240 87
 RD 500 0.0010 0.015 CIRC 3.0 0.0
 RD 2600 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*

KKYSS15A

KM 0

HC 2

ZW C=FLOW

*

KK SS15F

KM AREA=19.0 AC

BA0.0297

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 2.125 0.10 0.1200 80.401
 UK 150 0.0100 0.400 96
 UK 121 0.0100 0.240 4
 RD 800 0.0010 0.055 TRAP 10.0 5.0
 RD 1200 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*

KK SS15C

KM AREA= 4.2 AC

BA0.0066

PB

* PI

BF -1 -0.001 1.50
 LU 0.10 0.0700 4.978
 UK 150 0.0100 0.400 100.00
 RD 200 0.0016 0.015 CIRC 2.0 0.0
 RD 200 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*

KK SS15D

KM AREA= 6.9 AC

BA0.0108

```

PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.742 0.10 0.1200 40.000
UK 150 0.0100 0.400 1
UK 130 0.0100 0.240 99
RD 500 0.0010 0.015 CIRC 7.0 0.0
ZW C=FLOW
*
KK SS15E
KM AREA=19.6 AC
BA0.0306
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000 0.10 0.1200 70.638
UK 150 0.0100 0.400 1
UK 79 0.0142 0.240 99
RD 1000 0.0010 0.015 CIRC 6.0 0.0
ZW C=FLOW
*
KKYOMB_9
KM COMBINE FLOW AT EXISTING ROAD CULVERT
HC 5
ZW C=FLOW
*
KKU RR10
KM ROUTE FLOW THROUGH SHED CUS17F TO CNFLUENCE WITH CURRY CK TRIB PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CH
GRADING
RS 1 FLOW -1
SV 0.00 1.70 2.89 7.91 11.87 15.02 17.82 19.55
SQ 0.0 50.0 100.0 250.0 400.0 550.0 700.0 800.0
ZW C=FLOW
*
KK SS17F
KM AREA=18.0 AC
BA0.0281
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.016 0.10 0.1200 84.405
UK 150 0.0100 0.400 100.00 95
UK 290 0.0074 0.240 5
RD 1800 0.0010 0.055 TRAP 10.0 5.0
ZW C=FLOW
*
KK SS17B
KM AREA=33.9 AC
BA0.0529
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.998 0.10 0.1200 84.405
UK 150 0.0100 0.400 95
UK 290 0.0074 0.240 5
RD 1200 0.0010 0.020 TRAP 10.0 5.0
RD 600 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KK SS17D
KM AREA = 15.4
BA0.0240
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.501 0.10 0.1200 46.994
UK 150 0.0100 0.400 7
UK 115 0.0100 0.240 93
RD 500 0.0016 0.015 CIRC 2.0 0.0
RD 500 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KK SS17E
KM AREA=7.6 AC
BA0.0119
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.967 0.10 0.1200 46.994
UK 150 0.0100 0.400 100.00 7
RD 550 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*

```

```

KKYOMB10
KM COMBINE ALL FLOWS FROM CURRY CK AT CONFLUENCE W/ CURRY TRIB
HC 5
ZW C=FLOW
*
KKCUS17C
KM AREA = 10.7
BA0.0629
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.004
UK 150 0.0100 0.400 100.00
RD 2500 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KKYUS17C
KM 0
HC 2
ZW C=FLOW
*
KKCU2A1A
KM AREA = 49.0
BA0.0765
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 3.162 0.10 0.1200 46.652
UK 150 0.0100 0.400 28
UK 400 0.0050 0.240 72
RD 1000 0.0016 0.015 CIRC 2.0 0.0
RD 1000 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KKVR_201
KM ROUTE FLOW THROUGH SHED CU2A1D
RD 1000 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KK S2A1B
KM AREA = 13.0 AC
BA0.0203
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.953 0.10 0.1200 49.962
UK 150 0.0100 0.400 62
UK 178 0.0096 0.240 38
RD 700 0.0016 0.015 CIRC 2.0 0.0
RD 500 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KKYOM201
KM COMBINE FLOW FROM CU2A1A & CU2A1B
HC 2
ZW C=FLOW
*
KK CWP7
KM Curry Creek Basin CU2A (5-03)
BA0.0780
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 41.023
UK 300 0.0200 0.350 100.00
RD 600 0.0020 0.024 .0200 TRAP 1.0 0.0
RD 1800 0.0010 0.024 TRAP 4.0 1.0
ZW C=FLOW
*
*
KKU DB3
KM " Det Basin C (1.6 acre surface area;1-18";1-24" outfalls)
"
RS 1 FLOW -1
SV 0.00 1.50 3.10 4.60 6.30 7.90
SQ 0.0 2.0 12.0 23.0 40.0 90.0
ZW C=FLOW
*
KKVR_203
KM ROUTE FLOW TO CURRY CK TRIB
RD 500 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*

```

```

KK YCWP7
KM 0
HC 2
ZW C=FLOW
*
KK S2A1C
KM AREA = 20.0 AC
BA0.0313
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 4.811 0.10 0.1200 45.362
UK 150 0.0100 0.400 6
UK 127 0.0100 0.240 94
RD 300 0.0016 0.015 CIRC 2.0 0.0
RD 1500 0.0010 0.015 CIRC 4.0 0.0
ZW C=FLOW
*
KKYOM202
KM COMBINE FLOW WITH CU2A1C
HC 2
ZW C=FLOW
*
KKVR202A
KM ROUTE FLOW THROUGH SHED CU2A1D
RD 800 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KK S2A1D
KM AREA=39.5 AC
BA0.0617
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.005
UK 150 0.0100 0.400 100.00
RD 2400 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KKYM202B
KM COMBINE FLOW WITH RR202A
HC 2
ZW C=FLOW
*
KKVR202B
KM ROUTE FLOW TO MARKET STREET
RD 700 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KK S2A1E
KM AREA = 3.7 AC
BA0.0057
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 5.000 0.10 0.1200 40.000
UK 150 0.0100 0.400 8
UK 136 0.0100 0.240 92
RD 350 0.0016 0.015 CIRC 2.0 0.0
ZW C=FLOW
*
KK S2A1F
KM AREA=4.0 AC
BA0.0062
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.011 0.10 0.1200 40.000
UK 150 0.0100 0.400 99
UK 160 0.0100 0.240 1
RD 700 0.0010 0.020 TRAP 10.0 5.0
ZW C=FLOW
*
KKYM202C
KM COMBINE
HC 2
ZW C=FLOW
*
KKYOM203
KM COMBINE FLOW AT MARKET STREET
HC 2
ZW C=FLOW
*

```

KKVRCM20

KM ROUTE FLOW TO CURRY CK CHANNEL UP/S OF WEST SIDE DRIVE
RD 1100 0.0010 0.020 TRAP 10.0 5.0

ZW C=FLOW

*

KK S2AB2

KM AREA = 5.3 AC

BA0.0083

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 4.895 0.10 0.1200 43.348

UK 150 0.0100 0.400 13

UK 132 0.0100 0.240 87

RD 450 0.0016 0.015 CIRC 2.0 0.0

ZW C=FLOW

*

KK S2AB3

KM AREA = 7.4 AC

BA0.0116

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 4.418 0.10 0.1200 40.056

UK 150 0.0100 0.400 2

UK 132 0.0100 0.240 98

RD 550 0.0016 0.015 CIRC 2.0 0.0

ZW C=FLOW

*

KK S2AB4

KM AREA = 7.3 AC

BA0.0114

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.053

UK 150 0.0100 0.400 100.00

RD 1100 0.0010 0.020 TRAP 10.0 5.0

ZW C=FLOW

*

KKYCM2AB

KM COMBINE FLOW

HC 4

ZW C=FLOW

*

KK CWP8

KM Basin CU2A (5-03)

BA0.0992

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0600 40.000

UK 300 0.0200 0.350 100.00

RD 610 0.0070 0.024 .0200 TRAP 1.0 0.0

RD 2800 0.0010 0.024 TRAP 4.0 1.0

ZW C=FLOW

*

*

KKU DB4

KM " Det Basin B for WP8 (1.6 acre surface area;1-18";1-24" outfalls)

"

RS 1 FLOW -1

SV 0.00 1.50 3.10 4.60 6.30 7.90

SQ 0.0 2.0 12.0 23.0 40.0 90.0

ZW C=FLOW

*

KK S2AC2

KM AREA = 3.3 AC

BA0.0052

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.132

UK 150 0.0100 0.400 100.00

RD 600 0.0010 0.020 TRAP 10.0 5.0

ZW C=FLOW

*

KKYS2AC2

KM 0

HC 2

ZW C=FLOW

*

KK S2AC1

KM AREA = 1.9 AC

BA0.0029

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	3.286	0.10	0.1200	40.016		
UK	150	0.0100	0.400	3				
UK	130	0.0100	0.240	97				
RD	350	0.0016	0.015		CIRC	2.0	0.0	

ZW C=FLOW

*

KKYS2AC1

KM 0

HC 2

ZW C=FLOW

*

KK CWP8A

KM PORTION OF EXISTING SHED CU2A8- A=17.5 AC ASSUMED DEVELOPED MDResidential

BA0.0274

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.1200	50.000					
UK	150	0.0050	0.240	100.00				
RD	1100	0.0016	0.015		CIRC	6.0	0.0	

ZW C=FLOW

*

KK S2AC3

KM AREA = 16.6 AC

BA0.0259

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.955	0.10	0.1200	47.340		
UK	150	0.0100	0.400	19				
UK	132	0.0100	0.240	81				
RD	350	0.0016	0.015		CIRC	2.0	0.0	
RD	400	0.0010	0.015		CIRC	5.0	0.0	

ZW C=FLOW

*

KKYS2AC3

KM 0

HC 3

ZW C=FLOW

*

KK YCOMB

KM 0

HC 2

ZW C=FLOW

*

KK S2AC6

KM AREA = 22.4

BA0.0350

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	2.025	0.10	0.1200	82.339		
UK	150	0.0100	0.400	97				
UK	77	0.0146	0.240	3				
RD	2300	0.0010	0.055		TRAP	10.0	5.0	

ZW C=FLOW

*

KKYS2AC6

KM 0

HC 2

ZW C=FLOW

*

KK S2AC4

KM AREA = 10.1

BA0.0157

PB

* PI

BF	-1	-0.001	1.50					
LU	0.10	0.1200	40.000					
UK	130	0.0100	0.240	100.00				
RD	700	0.0016	0.015		CIRC	2.0	0.0	

ZW C=FLOW

*

KKYS2AC4

KM 0

HC 2

ZW C=FLOW

*

KK S2A9
 KM AREA = 36.7 AC
 BAO.0573
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.723 0.10 0.1200 55.939
 UK 150 0.0100 0.400 6
 UK 98 0.0104 0.240 94
 RD 1000 0.0016 0.015 CIRC 2.0 0.0
 RD 1550 0.0010 0.015 CIRC 5.0 0.0
 ZW C=FLOW
 *

KK YS2A9
 KM 0
 HC 2
 ZW C=FLOW
 *

KK S2AB1
 KM AREA = 53.6 AC
 BAO.0523
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 4.440 0.10 0.1200 57.700
 UK 150 0.0100 0.400 29
 UK 127 0.0100 0.240 71
 RD 1200 0.0016 0.015 CIRC 2.0 0.0
 RD 300 0.0010 0.015 CIRC 4.0 0.0
 ZW C=FLOW
 *

KK S2A1G
 KM 0
 BAO.0070
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.201 0.10 0.1200 82.720
 UK 600 0.0150 0.400 88
 UK 68 0.0173 0.240 12
 RD 500 0.0010 0.050 TRAP 2.0 2.0
 ZW C=FLOW
 *

KKYS2A1G
 KM 0
 HC 2
 ZW C=FLOW
 *

KK S2AC5
 KM AREA = 20.9
 BAO.0571
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.0700 2.823 0.10 0.1200 46.029
 UK 150 0.0100 0.400 40
 UK 121 0.0100 0.240 60
 RD 400 0.0016 0.015 CIRC 2.0 0.0
 RD 450 0.0010 0.015 CIRC 5.0 0.0
 ZW C=FLOW
 *

KKYS2AC5
 KM 0
 HC 2
 ZW C=FLOW
 *

KK S2AC7
 KM 0
 BAO.0087
 PB
 * PI
 BF -1 -0.001 1.50
 LU 0.10 0.1200 42.931
 UK 124 0.0100 0.240 100.00
 RD 500 0.0010 0.050 TRAP 2.0 2.0
 ZW C=FLOW
 *

KK S2A8
 KM AREA = 88.4
 BAO.1294
 PB
 * PI
 BF -1 -0.001 1.50

LU	0.10	0.0700	4.991	0.10	0.1200	46.529		
UK	150	0.0100	0.400		6			
UK	118	0.0100	0.240		94			
RD	600	0.0016	0.015			CIRC	2.0	0.0
RD	3500	0.0010	0.015			CIRC	5.0	0.0

ZW C=FLOW
*

KKYM203F
KM COMBINE FLOW
HC 2

ZW C=FLOW
*

KKYOM204
KM COMBINE TOTAL FLOW AT WEST SIDE DRIVE
HC 2

ZW C=FLOW
*

KKUR_207
KM ROUTE FLOW THROUGH SHED CU2A7B PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CHANNEL GRADING
RS 1 FLOW -1

SV	0.00	2.28	4.19	7.75	10.94	13.89	16.81	21.00
SQ	0.0		100.0	200.0	300.0	400.0	500.0	600.0

ZW C=FLOW
*

KK S2A7A
KM AREA = 37.3 AC
BA0.0583

PB
* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.911	0.10	0.1200	52.734		
UK	150	0.0100	0.400		14			
UK	112	0.0100	0.240		86			
RD	1100	0.0016	0.015			CIRC	2.0	0.0
RD	1100	0.0010	0.015			CIRC	4.0	0.0

ZW C=FLOW
*

KK S2A7
KM AREA = 17.7
BA0.0276

PB
* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	2.039	0.10	0.1200	83.349		
UK	150	0.0100	0.400		94			
UK	97	0.0106	0.240		6			
RD	400	0.0010	0.055			TRAP	10.0	5.0
RD	1500	0.0010	0.055			TRAP	10.0	5.0

ZW C=FLOW
*

KK S2A7B
KM AREA = 14.4 AC
BA0.0224

PB
* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	4.987	0.10	0.1200	40.000		
UK	150	0.0100	0.400		36			
UK	154	0.0100	0.240		64			
RD	550	0.0016	0.015			CIRC	2.0	0.0
RD	450	0.0010	0.015			CIRC	4.0	0.0

ZW C=FLOW
*

KK S2A7C
KM AREA = 51.9 AC
BA0.0812

PB
* PI

BF	-1	-0.001	1.50					
LU	0.10	0.0700	5.000	0.10	0.1200	45.120		
UK	150	0.0100	0.400		5			
UK	132	0.0100	0.240		95			
RD	450	0.0016	0.015			CIRC	2.0	0.0
RD	2000	0.0010	0.015			CIRC	4.0	0.0

ZW C=FLOW
*

KKYOM205
KM COMBINE FLOW FROM CU2A7 & CU2A7A
HC 4

ZW C=FLOW
*

KKYOM206

KM COMBINE FLOW WITH IN THE CHANNEL

HC 2
ZW C=FLOW

*
*

KKUR_208

KM ROUTE FLOW THROUGH SHED CU2A4 TO WATT AVE PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CHANNEL GRADING

RS 1 FLOW -1
SV 0.00 1.47 2.86 6.41 11.52 17.48 24.82 36.80
SQ 0.0 100.0 200.0 300.0 400.0 500.0 600.0

ZW C=FLOW

*
*

KK S2A4A

KM AREA = 41.5 AC

BA0.0648

PB

* PI

BF -1 -0.001 1.50
LU 0.10 0.0700 4.911 0.10 0.1200 56.440
UK 150 0.0100 0.400 8
UK 98 0.0104 0.240 92
RD 1000 0.0016 0.015 CIRC 2.0 0.0
RD 1400 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*
*

KK S2A4

KM AREA = 6.7 AC

BA0.0105

PB

* PI

BF -1 -0.001 1.50
LU 0.10 0.0700 2.124 0.10 0.1200 84.481
UK 150 0.0100 0.400 83
UK 79 0.0142 0.240 17
RD 1500 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*
*

KK S2A5

KM AREA = 108.8 AC ASSUMED OFFSITE DEVELOPED MDResidential

BA0.1700

PB

* PI

BF -1 -0.001 1.50
LU 0.10 0.0700 4.944 0.10 0.1200 49.625
UK 150 0.0100 0.400 7
UK 109 0.0100 0.240 93
RD 1500 0.0016 0.015 CIRC 2.0 0.0
RD 1300 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*
*

KKYOM208

KM COMBINE FLOW FROM SHEDS CU2A5- CU2A4 & CU2A4A AT WATT AVE

HC 3

ZW C=FLOW

*
*

KKYOM110

KM COMBINE FLOW AT WATT AVE CULVERT

HC 2

ZW C=FLOW

*
*

KKUR_210

KM ROUTE FLOW THROUGH SHED CU2A6 TO CONFLUENCE WITH CURRY CK PUL'S STORAGE ROUTING - NOV 7- 06 RAS PER CHANNEL GRADING

RS 1 FLOW -1
SV 0.00 2.28 3.99 8.58 11.98 15.16 18.06 20.75 25.00
SQ 0.0 50.0 100.0 200.0 300.0 400.0 500.0 600.0 800.0

ZW C=FLOW

*
*

KK S2A6A

KM AREA = 29.9 AC

BA0.0468

PB

* PI

BF -1 -0.001 1.50
LU 0.10 0.0700 4.828 0.10 0.1200 40.041
UK 150 0.0100 0.400 4
UK 132 0.0100 0.240 96
RD 600 0.0016 0.015 CIRC 2.0 0.0
RD 1000 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*
*

KK S2A6B

KM AREA = 10.3 AC

BA0.0161

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 4.622 0.10 0.1200 40.014

UK 150 0.0100 0.400 6

UK 134 0.0100 0.240 94

RD 1000 0.0016 0.015 CIRC 2.0 0.0

ZW C=FLOW

*

KK S2A6

KM AREA = 37.0 AC

BA0.0578

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.031

UK 150 0.0100 0.400 100.00

RD 400 0.0010 0.055 TRAP 2.0 5.0

RD 2500 0.0010 0.055 TRAP 10.0 5.0

ZW C=FLOW

*

KK S2A6C

KM AREA = 38.0 AC

BA0.0594

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 4.981 0.10 0.1200 47.276

UK 150 0.0100 0.400 9

UK 118 0.0100 0.240 91

RD 650 0.0016 0.015 CIRC 2.0 0.0

RD 1300 0.0010 0.015 CIRC 4.0 0.0

ZW C=FLOW

*

KKYOM211

KM COMBINE ALL FLOWS FROM CURRY CK TRIB

HC 5

ZW C=FLOW

*

KK S2A2

KM AREA = 17.6 AC UP/S OF WATT

BA0.0275

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.000

UK 200 0.0100 0.400 100.00

RD 800 0.0100 0.045 TRAP 2.0 10.0

ZW C=FLOW

*

KK S2A1

KM AREA = 8.4 AC UP/S OF WATT

BA0.0131

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.000

UK 200 0.0100 0.400 100.00

RD 900 0.0100 0.045 TRAP 2.0 10.0

ZW C=FLOW

*

KK YS2A1

KM 0

HC 2

ZW C=FLOW

*

KK S2A5C

KM AREA = 69.3 AC

BA0.1082

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.030

UK 200 0.0100 0.400 100.00

RD 1400 0.0100 0.045 TRAP 2.0 10.0

ZW C=FLOW

*

KKYM2A5C

KM COMBINE FLOWS from cu2a1 & cu2a5c

HC 2

ZW C=FLOW

```

*
KKYDD100
KM COMBINE ALL FLOWS BEFORE ADDING TO CURRY CK
HC 2
ZW C=FLOW
*
KKVR_211
KM ROUTE FLOW TO COMBINATION OF ALL CURRY CREEK FLOW
RD 1500 0.0010 0.055 TRAP 10.0 5.0
ZW C=FLOW
*
KKYOM212
KM COMBINE ALL FLOWS FROM CURRY CREEK AT PROJECT WEST BOUNDARY
HC 2
ZW C=FLOW
*
KKCU2A3A
KM AREA = 38.2 AC WEST OF WATT
BAO.0597
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000
UK 200 0.0100 0.400 100.00
RD 1200 0.0100 0.045 TRAP 2.0 10.0
ZW C=FLOW
*
KKY2A3AC
KM COMBINE FLOW FROM CU2A2 & CU2A3A
HC 2
ZW C=FLOW
*
KKV2A3AR
KM ROUTE FLOW THROUGH SHED CU2A
RD 2300 0.0010 0.055 TRAP 10.0 5.0
ZW C=FLOW
*
KK CU2A
KM AREA = 485.3 AC NOV 29- 06
BAO.7583
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0150 0.400 100.00
RD 800 0.0100 0.050 .1000 TRAP 1.0 10.0
RD 5500 0.0010 0.050 .2000 TRAP 5.0 10.0
ZW C=FLOW
*
KKYCCU2A
KM COMBINED CU2A & RCH3 (2.64 SM + 0.1014 + 0.0706)+ S17BR
HC 3
ZW C=FLOW
*
KK CWP1
KM AREA REVISED 6-05
BAO.1447
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000 0.10 0.0600 100.000
UK 300 0.0080 0.350 51
UK 150 0.0100 0.110 49
RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
RD 2050 0.0010 0.015 TRAP 2.0 2.0
ZW C=FLOW
*
KKV R3
KM main channel through to c-5
RD 1700 0.0030 0.035 TRAP 3.5 10.0
ZW C=FLOW
*
KK CWP2
KM AREA REVISED 6-05
BAO.1969
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 100.000 0.10 0.0600 2.000
UK 150 0.0080 0.110 27
UK 300 0.0100 0.350 73
RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
RD 2000 0.0010 0.020 TRAP 2.0 2.0

```

```

ZW C=FLOW
*
KKY C5
KM node combines area 1 & area 2
HC 2
ZW C=FLOW
*
KKV R4
KM main channel from C-WP2
RD 1800 0.0020 0.035 TRAP 3.5 10.0
ZW C=FLOW
*
KK CWP4
KM Revise Date (5-03)
BA0.3627
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000 0.10 0.0600 100.000
UK 300 0.0100 0.350 54
UK 200 0.0200 0.110 46
RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
RD 2600 0.0010 0.020 TRAP 2.0 2.0
ZW C=FLOW
*
KKV R7
KM Main channel to C-6
RD 1935 0.0021 0.035 TRAP 3.5 10.0
ZW C=FLOW
*
KK CWP3
KM AREA REVISED 6-05
BA0.3000
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0700 2.000 0.10 0.0600 100.000
UK 300 0.0100 0.350 54
UK 200 0.0100 0.110 46
RD 2000 0.0020 0.020 TRAP 2.0 2.0
ZW C=FLOW
*
KKY C6
KM combines node C-5 and C-WP4
HC 3
ZW C=FLOW
*
KKU DBC6
KM Detention Storage (step boxes)(2-5'x 10';2-4'x 10' Box Culverts)
RS 1 FLOW -1
SV 0.00 0.70 3.50 10.30 22.80 35.00 42.50 69.70 88.00
SQ 0.0 40.0 175.0 350.0 600.0 800.0 850.0 950.0 1000.0
ZW C=FLOW
*
KKV R8
KM main channel through area five
RD 3700 0.0010 0.035 TRAP 3.5 10.0
ZW C=FLOW
*
KK CWP6
KM Revise Date (5-03) (194.5 ac)
BA0.3039
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0900 2.000 0.10 0.0600 100.000
UK 300 0.0100 0.350 72
UK 200 0.0080 0.110 28
RD 450 0.0070 0.018 .0500 TRAP 0.0 1.0
RD 2000 0.0010 0.020 TRAP 2.0 2.0
ZW C=FLOW
*
KKY C8
KM node combines node c-7 and area 6
HC 2
ZW C=FLOW
*
KKU DBC8
KM " Detention Storage (3-36" w/weir)"
RS 1 FLOW -1
SV 0.00 2.10 12.00 21.60 30.20 38.80 42.50 52.40 64.00

```

SQ 0.0 30.0 120.0 165.0 200.0 500.0 900.0 950.0 1050.0

ZW C=FLOW

*

KKV R13

KM main channel EXIT FROM NODE C-8

RD 2000 0.0010 0.035 TRAP 5.0 5.0

ZW C=FLOW

*

KK SP9A

KM AREA = 19.5 AC

BA0.0305

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 4.948 0.10 0.1200 51.361

UK 150 0.0100 0.400 2

UK 100 0.0100 0.240 98

RD 1200 0.0016 0.015 CIRC 3.0 0.0

RD 650 0.0010 0.015 CIRC 5.0 0.0

ZW C=FLOW

*

KK CWP9

KM AREA REVISED 149.0 ac

BA0.2329

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.000 0.10 0.0700 84.266

UK 300 0.0080 0.400 98

UK 400 0.0050 0.240 2

RD 1450 0.0070 0.050 TRAP 0.0 1.0

RD 3600 0.0010 0.050 TRAP 2.0 2.0

ZW C=FLOW

*

KKYCCWP9

KM COMBINE FLOW FROM CWP9A & CWP9 AT PLEASANT GROVE BLVD

HC 2

ZW C=FLOW

*

KKV R913

KM Route shed through WP-13

RD 2000 0.0023 0.035 TRAP 10.0 10.0

ZW C=FLOW

*

KK CWP13

KM AEEA REVISED 6-05

BA0.0942

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0700 2.000 0.10 0.0700 100.000

UK 300 0.0080 0.350 67

UK 150 0.0080 0.110 33

RD 450 0.0070 0.018 .0300 TRAP 0.0 1.0

RD 2400 0.0010 0.020 TRAP 2.0 2.0

ZW C=FLOW

*

KKY C9

KM Combines Area WP9 & WP13

HC 2

ZW C=FLOW

*

KKY C13

KM 0

HC 2

ZW C=FLOW

*

KK CWP14

KM AERA REVISED 6-05

BA0.1525

PB

* PI

BF -1 -0.001 1.50

LU 0.10 0.0600 2.000 0.10 0.0600 100.000

UK 300 0.0080 0.350 98

UK 150 0.0100 0.110 2

RD 1600 0.0020 0.020 TRAP 2.0 2.0

ZW C=FLOW

*

KKYCWP14

KM COMBINE FLOW

HC 2

ZW C=FLOW

```

*
KK CU2B1
KM 53.9 ACRES AREA REVISED 6-05
BA0.1150
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 200 0.0050 0.400 100.00
RD 1900 0.0080 0.050 TRAP 2.0 10.0
ZW C=FLOW
*
KKCU2B1A
KM 29.2 ACRES AREA REVISED 6-05
BA0.0456
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 200 0.0050 0.400 100.00
RD 1600 0.0080 0.050 TRAP 2.0 10.0
ZW C=FLOW
*
KKY C10
KM COMBINE FLOW AT WATT AVE & PLEASANT GROVE BLVD
HC 3
ZW C=FLOW
*
*
KKU RR1
KM ROUTE FLOW FROM C-10- 3500FT DNS TO CP-1
RS 1 FLOW -1
SV 0.00 4.60 7.40 10.00 15.70 20.60 24.50 37.60 45.50
SQ 0.0 100.0 216.0 300.0 405.0 500.0 559.0 785.0 900.0
ZW C=FLOW
*
KK CU2B2
KM AREA = 214.8 AC NOV 29- 06
BA0.3356
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0050 0.400 100.00
RD 2700 0.0067 0.050 .3800 TRAP 10.0 2.0
ZW C=FLOW
*
KKY CP1
KM COMBINE CU2B2 WITH FLOW AT CP-1
HC 2
ZW C=FLOW
*
*
KKU RR2
KM ROUTE FLOW FROM CP-1 TO CU2A+
RS 1 FLOW -1
SV 0.00 16.60 25.90 34.10 51.70 57.30 71.20 91.80 95.40
SQ 0.0 150.0 245.0 320.0 467.0 500.0 634.0 872.0 900.0
ZW C=FLOW
*
KK CU2B3
KM 221.1 ACRES
BA0.3455
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0050 0.400 100.00
RD 6300 0.0012 0.050 TRAP 4.0 2.0
ZW C=FLOW
*
KKYCU3B+
KM COMBINE CU3B & RR2
HC 2
ZW C=FLOW
*
KKYCU2A+
KM 0
HC 2
ZW C=FLOW
*
*
KKU RR3

```

```

KM  ROUTE FLOW FROM CU2A+ TO CU2C1- 2800 FT
RS   1      FLOW      -1
SV  0.00   7.70   23.20   32.80   57.40   59.60   66.10   85.90   95.10
SQ  0.0   250.0   491.0   700.0  1369.0  1400.0  1566.0  2192.0  2500.0
ZW C=FLOW
*
KK CU2C1
KM  186.4 ACRE @ COUNTRY ACRE LN
BA0.2913
PB
* PI
BF   -1   -0.001   1.50
LU  0.10  0.0600  2.000
UK   600  0.0050  0.400  100.00
RD  3700  0.0016  0.100  .2500   TRAP   10.0   20.0
ZW C=FLOW
*
KKY CP2
KM  COMBINE RUNOFF FROM CUUC1 WITH FLOW AT CP-2
HC   2
ZW C=FLOW
*
*
KKU RR4
KM  ROUTE FLOW FROM CP-2 TO CP-3- 2800 FT DNS
RS   1      FLOW      -1
SV  0.00   20.70   34.00   44.30   64.00   70.70   81.80  112.30  125.60
SQ  0.0   250.0   500.0   700.0  1140.0  1300.0  1566.0  2185.0  2500.0
ZW C=FLOW
*
KK CU2C2
KM  " 146.6 ACRES COUNTRY ACRES LN TO "LANDING STRIP""
BA0.2290
PB
* PI
BF   -1   -0.001   1.50
LU  0.10  0.0600  2.000
UK   100  0.0050  0.400  100.00
RD  4200  0.0090  0.050           TRAP   15.0   3.0
ZW C=FLOW
*
KKY CP3
KM  COMBINE RUNOFF FROM CU2C2 WITH FLOW AT CP-3
HC   2
ZW C=FLOW
*
*
KKU RR5
KM  ROUTE FLOW FROM CP-3 TO BREWER ROAD 4200 FT DNS
RS   1      FLOW      -1
SV  0.00   7.00   11.10   12.20   15.80   17.20   19.80   27.20   30.10
SQ  0.0   260.0   513.0   720.0  1167.0  1350.0  1601.0  2230.0  2500.0
ZW C=FLOW
*
KK CU2C3
KM  " 216.6 ACRES "LANDING STRIP" TO BREWER RD"
BA0.3384
PB
* PI
BF   -1   -0.001   1.50
LU  0.10  0.0600  2.000
UK   600  0.0050  0.400  100.00
RD  1900  0.0012  0.100  .2000   TRAP   300.0   1.0
RD   800  0.0010  0.050  .1500   TRAP    2.0   2.0
ZW C=FLOW
*
KKYCU2C+
KM  0
HC   2
ZW C=FLOW
*
KKCWP11A
KM  Curry Creek Basin
BA0.0750
PB
* PI
BF   -1   -0.001   1.50
LU  0.10  0.0600  2.000   0.10  0.0600  100.000
UK   300  0.0100  0.350    60
UK   150  0.0200  0.110    40
RD   500  0.0040  0.024  .0800   TRAP    1.0   5.0
RD   900  0.0010  0.024           TRAP    2.0  10.0
ZW C=FLOW

```

```

*
*
KKU DB5
KM " Det Basin A for C-WP11A (1.0 acre surface area; 2-18" outfalls)"
RS 1 FLOW -1
SV 0.00 0.70 1.80 2.80 3.90 5.10 6.50
SQ 0.0 2.0 7.0 15.0 24.0 55.0 90.0
ZW C=FLOW
*
KK8 R11
KM 0
RD
RC 0.045 0.045 0.045 1800 0.0028 1000
RX 0 0 1000 1013 1027 1040 2040 2040 0 0
RY1020.0 1000.0 990.0 984.0 984.0 990.0 1000.0 1020.0 0.0 0.0
ZW C=FLOW
*
KK CWP11
KM Open Space
BA0.1230
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000 0.10 0.0600 100.000
UK 300 0.0100 0.350 95
UK 200 0.0100 0.110 5
RD 1900 0.0036 0.050 .0800 TRAP 3.5 10.0
RD 1300 0.0010 0.050 TRAP 3.5 10.0
ZW C=FLOW
*
KKY CB11
KM 0
HC 2
ZW C=FLOW
*
KKVR1112
KM Route shed through WP-12
RD 1200 0.0036 0.035 TRAP 10.0 10.0
ZW C=FLOW
*
KK CWP12
KM 0
BA0.0678
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 100.000 0.10 0.0600 2.000
UK 600 0.0100 0.110 2
UK 600 0.0100 0.400 98
RD 1400 0.0085 0.035 TRAP 3.5 10.0
ZW C=FLOW
*
KKYC1112
KM combines WP11 and WP12 at Future Watt Ave. 170.3 ac=.266 sq mi
HC 2
ZW C=FLOW
*
*
KKU RCU3
KM ROUTE FLOW FROM C11-12 TO CU3B 5300 FT DNS
RS 1 FLOW -1
SV 0.00 2.90 14.30 20.30 27.90 30.80 32.90 41.70 47.90
SQ 0.0 30.0 62.0 90.0 159.0 190.0 214.0 298.0 350.0
ZW C=FLOW
*
KK CU3A
KM 247.3 acres
BA0.3864
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0050 0.400 100.00
RD 2000 0.0100 0.050 .2500 TRAP 10.0 10.0
RD 1500 0.0010 0.050 TRAP 5.0 5.0
ZW C=FLOW
*
KKYCU3AC
KM 0
HC 2
ZW C=FLOW
*
KK CU3B

```

KM 307.4 ACRES

BA0.4803

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0600	2.000						
UK	600	0.0010	0.400	100.00					
RD	1800	0.0013	0.050		TRAP	200.0	1.0		
RD	2600	0.0010	0.050		TRAP	5.0	3.0	YES	

ZW C=FLOW

*

KK CU3C

KM 110.3 acres

BA0.1723

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0600	2.000						
UK	600	0.0010	0.400	100.00					
RD	2900	0.0076	0.050		TRAP	10.0	10.0		

ZW C=FLOW

*

KKYCC3B+

KM COMBINE RCU3B+CU3B&CU3A+CU3C;170.3 + 307.4+247.3 + 110.3 = 835.3=1.3052 sq mi

HC 2

ZW C=FLOW

*

*

KKURC3B+

KM ROUTE FLOW FROM CC3B+ TO CU3D 3800 FT DNS

RS	1	FLOW	-1						
SV	0.00	8.20	63.30	69.10	78.30	81.60	92.00	110.10	143.60
SQ	0.0	50.0	97.0	150.0	263.0	300.0	345.0	445.0	550.0

ZW C=FLOW

*

KK CU3D

KM 465.8 acres ROUTES & COMBINES 465.8+835.3=1-301.1 ac = 2.033 Sq. Mi.

BA0.7278

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0600	2.000						
UK	600	0.0010	0.400	100.00					
RD	4400	0.0010	0.050	.4000	TRAP	7.0	3.0		
RD	1500	0.0010	0.060		TRAP	100.0	2.0	YES	

ZW C=FLOW

*

*

KKURC3D+

KM ROUTE FLOW CU3D TO CU3E+ 3600 FT DNS

RS	1	FLOW	-1						
SV	0.00	3.90	7.50	11.00	15.00	16.60	18.40	22.50	24.60
SQ	0.0	70.0	140.0	220.0	321.0	375.0	428.0	570.0	650.0

ZW C=FLOW

*

KK CU3E

KM 111.8 acres

BA0.1747

PB

* PI

BF	-1	-0.001	1.50						
LU	0.10	0.0600	2.000						
UK	600	0.0010	0.400	100.00					
RD	3600	0.0010	0.050		TRAP	18.0	8.0		

ZW C=FLOW

*

KKYCU3E+

KM COMBINE CU3E & RC3D+ 111.8+1301.1=1412.9 ac = 2.2077 Sq. Mi.

HC 2

ZW C=FLOW

*

*

KKURC3E+

KM ROUTE FLOW FROM CU3E+ TO CU3F+

RS	1	FLOW	-1						
SV	0.00	2.80	4.60	7.10	9.40	10.20	11.40	14.10	15.00
SQ	0.0	75.0	153.0	200.0	351.0	400.0	466.0	618.0	700.0

ZW C=FLOW

*

KK CU3F

KM 641.7ACRES

BA1.0026

PB

```

* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0010 0.400 100.00
RD 4100 0.0062 0.050 .5000 TRAP 15.0 3.0
RD 4400 0.0010 0.050 TRAP 12.0 2.0
ZW C=FLOW
*
KKYCC3F+
KM COMBINE RC3F+ & CU3E+ (602.7+ 1412.9=2015.6 ac = 3.149 Sq. Mi.)
HC 2
ZW C=FLOW
*
*
KKURC3F+
KM ROUTE CC3F+ TO CU3+ 2015.6 ac=3.149 Sq. Mi.
RS 1 FLOW -1
SV 0.00 17.20 30.90 54.40 75.50 81.00 95.10 123.30 131.30
SQ 0.0 80.0 164.0 300.0 410.0 470.0 565.0 780.0 850.0
ZW C=FLOW
*
KK CU3G
KM 200.0 acres
BA0.3125
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0010 0.400 100.00
RD 4500 0.0010 0.050 TRAP 70.0 90.0
ZW C=FLOW
*
KK CU3HA
KM 57.4 acres
BA0.0897
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0050 0.400 100.00
RD 1500 0.0010 0.050 TRAP 10.0 2.0
ZW C=FLOW
*
KK CU3HB
KM 106.1 acres
BA0.1658
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 300 0.0010 0.400 100.00
RD 4000 0.0034 0.050 TRAP 100.0 50.0 YES
ZW C=FLOW
*
KKY CU3
KM COMBINE CU3H & CU3G & RC3F+ (106.1+200.0+2015.6=2321.7=3.627 Sq. Mi.)
HC 3
ZW C=FLOW
*
KKYCU3C+
KM COMBINE CU3 & CU2C+ (2321.7+
HC 2
ZW C=FLOW
*
*
KKU RCH8
KM 0
RS 1 FLOW -1
SV 0.00 51.00 115.50 194.10 281.70 287.90 335.40 390.50 400.20
SQ 0.0 400.0 621.0 1000.0 1387.0 1500.0 1900.0 2682.0 3200.0
ZW C=FLOW
*
KK CU4
KM 0
BA3.5800
PB
* PI
BF -1 -0.001 1.50
LU 0.10 0.0600 2.000
UK 600 0.0150 0.400 100.00
RD 800 0.0100 0.050 .1000 TRAP 1.0 10.0
RD 8500 0.0010 0.050 .2000 TRAP 5.0 10.0
ZW C=FLOW

```

```

*
KK  CU5
KM  0
BA0.2800
PB
* PI
BF  -1  -0.001  1.50
LU  0.10  0.0600  2.000
UK  600  0.0150  0.400  100.00
RD  500  0.0100  0.050  .1000  TRAP  1.0  10.0
RD  2500  0.0010  0.050  TRAP  5.0  10.0
ZW C=FLOW
*
KKY CU5+
KM  0
HC  3
ZW C=FLOW
*
KK8 RCH9
KM  0
RD
RC  0.040  0.040  0.040  6000  0.0016  1000
RX  0  0  1200  1210  1230  1240  2440  2440  0  0
RY1020.0  1000.0  995.0  990.0  990.0  995.0  1000.0  1020.0  0.0  0.0
ZW C=FLOW
*
KK  CU6
KM  0
BA0.8700
PB
* PI
BF  -1  -0.001  1.50
LU  0.10  0.0600  2.000
UK  600  0.0150  0.400  100.00
RD  500  0.0100  0.050  .1000  TRAP  1.0  10.0
RD  1500  0.0010  0.050  .2000  TRAP  5.0  10.0
ZW C=FLOW
*
KKY CU6+
KM  0
HC  2
ZW C=FLOW
*
ZZ

```

**APPENDIX E
HEC-RAS PRE-PROJECT
SUMMARY TABLE**

HEC-RAS Profile: Max WS

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	1.44	Max WS	exist 10	146.35	112.07	114.24		114.25	0.002007	0.77	198.09	304.87	0.14
Main Reach	1.44	Max WS	exist 100	285.15	112.07	114.54		114.56	0.002486	1.04	310.30	428.77	0.17
Main Reach	1.43666*	Max WS	exist 10	146.34	111.85	114.13		114.14	0.000019	0.89	188.39	249.30	0.15
Main Reach	1.43666*	Max WS	exist 100	285.08	111.85	114.40		114.43	0.000035	1.39	264.09	319.84	0.21
Main Reach	1.43333*	Max WS	exist 10	143.04	111.63	114.04		114.06	0.001716	0.97	207.16	359.94	0.14
Main Reach	1.43333*	Max WS	exist 100	284.43	111.63	114.26		114.28	0.002979	1.40	288.27	385.13	0.19
Main Reach	1.43	Max WS	exist 10	146.87	111.41	113.43		113.46	0.010060	1.98	130.83	303.56	0.33
Main Reach	1.43	Max WS	exist 100	284.08	111.41	113.72		113.75	0.007689	2.01	220.90	330.54	0.30
Main Reach	1.42	Max WS	exist 10	142.39	109.58	112.15		112.16	0.001016	1.05	164.45	174.19	0.18
Main Reach	1.42	Max WS	exist 100	278.53	109.58	112.73		112.75	0.000823	1.28	269.04	187.88	0.18
Main Reach	1.41	Max WS	exist 10	140.46	109.58	111.98		112.00	0.000691	1.22	161.61	166.65	0.17
Main Reach	1.41	Max WS	exist 100	277.32	109.58	112.60		112.61	0.000570	1.36	342.29	314.30	0.16
Main Reach	1.40	Max WS	exist 10	155.18	108.42	111.82		111.84	0.000425	1.29	236.97	195.91	0.14
Main Reach	1.40	Max WS	exist 100	304.64	108.42	112.44		112.46	0.000496	1.60	376.03	265.17	0.16
Main Reach	1.39	Max WS	exist 10	169.81	107.49	111.26		111.38	0.003054	2.88	73.31	119.61	0.35
Main Reach	1.39	Max WS	exist 100	330.85	107.49	111.77		111.92	0.003669	3.51	162.80	230.05	0.40
Main Reach	1.38	Max WS	exist 10	191.26	107.00	110.14		110.18	0.002217	2.50	178.95	270.95	0.30
Main Reach	1.38	Max WS	exist 100	368.24	107.00	110.64		110.67	0.001671	2.50	335.83	349.63	0.27
Main Reach	1.37	Max WS	exist 10	209.51	106.00	109.39		109.44	0.001604	2.12	154.76	154.58	0.26
Main Reach	1.37	Max WS	exist 100	400.06	106.00	109.94		110.01	0.001827	2.65	250.67	194.20	0.29
Main Reach	1.36	Max WS	exist 10	228.81	105.84	108.48		108.55	0.002642	2.90	157.70	192.17	0.34
Main Reach	1.36	Max WS	exist 100	433.90	105.84	108.98		109.05	0.002638	3.30	259.98	219.90	0.35
Main Reach	1.35	Max WS	exist 10	240.32	104.83	107.91		107.96	0.001908	2.18	178.63	201.45	0.28
Main Reach	1.35	Max WS	exist 100	453.87	104.83	108.36		108.43	0.002176	2.69	299.24	305.41	0.31
Main Reach	1.34	Max WS	exist 10	253.38	104.46	107.23		107.28	0.002432	2.59	206.63	287.68	0.32
Main Reach	1.34	Max WS	exist 100	476.55	104.46	107.69		107.74	0.002159	2.79	347.12	322.63	0.31
Main Reach	1.33	Max WS	exist 10	252.39	101.96	105.94		106.09	0.003667	3.57	123.67	172.64	0.40
Main Reach	1.33	Max WS	exist 100	475.31	101.96	106.49		106.63	0.003535	4.01	245.70	271.06	0.41
Main Reach	1.32	Max WS	exist 10	257.18	101.80	105.28		105.31	0.001287	2.08	260.28	302.06	0.24
Main Reach	1.32	Max WS	exist 100	486.15	101.80	105.89		105.92	0.000954	2.08	450.70	316.72	0.21
Main Reach	1.31	Max WS	exist 10	262.52	100.91	104.67		104.79	0.002643	3.23	138.04	131.67	0.35
Main Reach	1.31	Max WS	exist 100	472.32	100.91	105.33		105.44	0.002437	3.59	307.18	378.14	0.35
Main Reach	1.30	Max WS	exist 10	267.85	100.63	104.06		104.09	0.001625	2.06	223.70	202.77	0.26
Main Reach	1.30	Max WS	exist 100	407.91	100.63	104.94		104.96	0.000613	1.61	439.05	284.89	0.17
Main Reach	1.29	Max WS	exist 10	267.15	100.36	103.62		103.70	0.001970	2.70	176.00	199.84	0.30
Main Reach	1.29	Max WS	exist 100	402.46	100.36	104.83		104.85	0.000406	1.59	470.71	373.80	0.15
Main Reach	1.28	Max WS	exist 10	207.82	100.00	103.31		103.33	0.000570	1.35	266.11	277.13	0.16
Main Reach	1.28	Max WS	exist 100	401.92	100.00	104.76		104.77	0.000146	0.95	754.35	394.52	0.09
Main Reach	1.27	Max WS	exist 10	202.45	98.88	103.23		103.24	0.000060	0.53	619.30	299.99	0.05
Main Reach	1.27	Max WS	exist 100	405.84	98.88	104.74		104.74	0.000043	0.59	1116.80	358.95	0.05
Main Reach	1.26	Max WS	exist 10	200.32	98.56	103.21		103.22	0.000093	0.69	550.07	377.53	0.07
Main Reach	1.26	Max WS	exist 100	405.13	98.56	104.73		104.73	0.000045	0.62	1162.76	433.26	0.05
Main Reach	1.255	Max WS	exist 10	209.11	97.97	103.19		103.19	0.000030	0.46	924.95	470.63	0.04
Main Reach	1.255	Max WS	exist 100	424.29	97.97	104.72		104.72	0.000020	0.46	1669.87	503.37	0.03
Main Reach	1.22	Max WS	exist 10	226.40	97.51	103.19		103.19	0.000020	0.38	897.73	389.22	0.03
Main Reach	1.22	Max WS	exist 100	458.22	97.51	104.71		104.71	0.000019	0.46	1603.59	546.34	0.03
Main Reach	1.215			Culvert									
Main Reach	1.21	Max WS	exist 10	226.40	96.51	99.85		100.04	0.006647	3.48	64.97	43.52	0.50
Main Reach	1.21	Max WS	exist 100	458.07	96.51	100.37		100.76	0.013238	5.05	90.73	58.58	0.71
Main Reach	1.20	Max WS	exist 10	226.38	96.05	99.46		99.60	0.006865	3.54	102.63	171.78	0.50
Main Reach	1.20	Max WS	exist 100	456.63	96.05	99.95		100.10	0.005785	3.91	197.25	210.30	0.48
Main Reach	1.19	Max WS	exist 10	226.82	95.07	98.36		98.40	0.001317	1.70	171.80	235.83	0.23
Main Reach	1.19	Max WS	exist 100	452.57	95.07	99.05		99.09	0.001012	1.87	365.51	320.07	0.21
Main Reach	1.18	Max WS	exist 10	227.45	94.10	97.66		97.76	0.002855	3.28	122.23	114.46	0.36
Main Reach	1.18	Max WS	exist 100	452.43	94.10	98.34		98.48	0.003118	4.00	215.27	158.77	0.39
Main Reach	1.17	Max WS	exist 10	228.25	93.80	97.01		97.05	0.000816	1.64	169.13	137.09	0.19
Main Reach	1.17	Max WS	exist 100	452.60	93.80	97.53		97.61	0.001363	2.43	251.51	179.34	0.25

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	1.16	Max WS	exist 10	228.64	93.25	96.46		96.53	0.002175	2.52	167.08	281.53	0.30
Main Reach	1.16	Max WS	exist 100	447.99	93.25	96.93		97.00	0.002051	2.80	314.20	337.36	0.31
Main Reach	1.15	Max WS	exist 10	229.29	92.65	96.02		96.04	0.000429	1.32	359.15	424.67	0.14
Main Reach	1.15	Max WS	exist 100	446.58	92.65	96.50		96.52	0.000493	1.58	566.71	438.99	0.16
Main Reach	1.14	Max WS	exist 10	230.56	92.20	94.73		94.80	0.003981	2.83	140.89	185.59	0.39
Main Reach	1.14	Max WS	exist 100	447.14	92.20	95.19		95.28	0.003827	3.29	232.98	212.65	0.40
Main Reach	1.13	Max WS	exist 10	231.11	91.95	93.93		93.94	0.000490	0.72	319.71	324.63	0.13
Main Reach	1.13	Max WS	exist 100	442.10	91.95	94.44		94.45	0.000443	0.89	500.26	540.00	0.13
Main Reach	1.12	Max WS	exist 10	231.34	90.70	93.31		93.40	0.003495	3.05	134.77	162.33	0.38
Main Reach	1.12	Max WS	exist 100	439.07	90.70	93.93		94.01	0.002843	3.29	254.38	228.07	0.36
Main Reach	1.11	Max WS	exist 10	231.41	90.02	92.80		92.81	0.000403	0.97	337.98	311.24	0.13
Main Reach	1.11	Max WS	exist 100	436.82	90.02	93.51		93.53	0.000323	1.09	572.19	340.77	0.12
Main Reach	1.10	Max WS	exist 10	231.28	88.90	92.21		92.32	0.003090	2.64	89.26	69.23	0.35
Main Reach	1.10	Max WS	exist 100	428.03	88.90	92.97		93.11	0.002724	3.16	167.25	137.53	0.35
Main Reach	1.09	Max WS	exist 10	246.17	87.51	91.67		91.68	0.000212	1.00	417.54	305.34	0.10
Main Reach	1.09	Max WS	exist 100	448.47	87.51	92.51		92.52	0.000184	1.10	705.22	358.37	0.10
Main Reach	1.08	Max WS	exist 10	243.35	87.08	91.39		91.45	0.001370	2.29	185.08	196.40	0.25
Main Reach	1.08	Max WS	exist 100	449.05	87.08	92.36		92.40	0.000694	2.01	431.69	293.71	0.19
Main Reach	1.07	Max WS	exist 10	219.94	87.00	91.01		91.06	0.001135	1.88	166.13	182.83	0.22
Main Reach	1.07	Max WS	exist 100	452.16	87.00	92.19		92.22	0.000512	1.68	482.05	340.85	0.16
Main Reach	1.06	Max WS	exist 10	210.54	86.38	90.84		90.84	0.000074	0.60	525.32	358.13	0.06
Main Reach	1.06	Max WS	exist 100	453.06	86.38	92.11		92.11	0.000063	0.70	1026.53	436.91	0.06
Main Reach	1.05	Max WS	exist 10	210.36	85.43	90.83		90.83	0.000046	0.53	622.31	341.05	0.05
Main Reach	1.05	Max WS	exist 100	453.66	85.43	92.09		92.10	0.000050	0.67	1129.06	458.18	0.05
Main Reach	1.04	Max WS	exist 10	210.41	84.89	90.82		90.82	0.000001	0.07	4683.00	1990.79	0.01
Main Reach	1.04	Max WS	exist 100	454.34	84.89	92.09		92.09	0.000001	0.10	7234.61	2043.52	0.01
Main Reach	1.03	Max WS	exist 10	210.43	84.80	90.82		90.82	0.000001	0.05	5078.08	1780.56	0.01
Main Reach	1.03	Max WS	exist 100	454.61	84.80	92.09		92.09	0.000001	0.08	7389.98	1868.37	0.01
Main Reach	1.025			Culvert									
Main Reach	1.02	Max WS	exist 10	209.42	84.82	88.69		88.69	0.000033	0.34	1086.01	770.73	0.04
Main Reach	1.02	Max WS	exist 100	423.76	84.82	89.42		89.42	0.000035	0.41	1653.57	832.18	0.04
Main Reach	1.01	Max WS	exist 10	210.09	84.33	88.10		88.20	0.002019	2.60	91.74	156.82	0.30
Main Reach	1.01	Max WS	exist 100	425.24	84.33	88.76		88.89	0.002232	3.22	209.92	200.56	0.32
Main Reach	1.00	Max WS	exist 10	210.90	83.82	87.58		87.76	0.005107	3.59	78.59	92.63	0.45
Main Reach	1.00	Max WS	exist 100	425.93	83.82	88.36		88.49	0.003844	3.60	214.90	251.27	0.41
Main Reach	0.95	Max WS	exist 10	212.10	83.21	86.87		87.00	0.002588	2.92	72.54	30.43	0.33
Main Reach	0.95	Max WS	exist 100	427.80	83.21	87.84		87.94	0.001719	2.97	258.41	265.24	0.29
Main Reach	0.90	Max WS	exist 10	213.66	82.09	86.32		86.40	0.001938	2.39	89.34	41.37	0.29
Main Reach	0.90	Max WS	exist 100	431.17	82.09	87.19		87.36	0.002739	3.37	130.77	77.09	0.36
Main Reach	0.85	Max WS	exist 10	213.60	81.05	86.02		86.07	0.000513	1.79	161.89	79.13	0.16
Main Reach	0.85	Max WS	exist 100	429.10	81.05	86.77		86.86	0.000939	2.73	225.31	91.24	0.22
Main Reach	0.80	Max WS	exist 10	214.57	81.05	85.84		85.90	0.001043	2.10	159.79	204.56	0.22
Main Reach	0.80	Max WS	exist 100	429.89	81.05	86.58		86.64	0.000942	2.32	345.47	273.69	0.22
Main Reach	0.75	Max WS	exist 10	214.70	80.50	85.56		85.64	0.001423	2.38	109.91	115.76	0.25
Main Reach	0.75	Max WS	exist 100	429.00	80.50	86.26		86.38	0.001777	3.09	225.88	205.41	0.29
Main Reach	0.70	Max WS	exist 10	214.76	80.36	85.28		85.36	0.001393	2.32	126.09	181.41	0.25
Main Reach	0.70	Max WS	exist 100	428.06	80.36	85.95		86.04	0.001512	2.80	257.60	213.44	0.27
Main Reach	0.65	Max WS	exist 10	214.89	80.36	85.09		85.13	0.000805	1.82	192.44	206.17	0.19
Main Reach	0.65	Max WS	exist 100	427.61	80.36	85.74		85.78	0.000917	2.23	330.56	224.51	0.21
Main Reach	0.6	Max WS	exist 10	215.09	80.20	84.92		84.96	0.000627	1.67	199.05	232.97	0.17
Main Reach	0.6	Max WS	exist 100	427.80	80.20	85.53		85.58	0.000783	2.11	358.88	281.66	0.20
Main Reach	0.55	Max WS	exist 10	215.32	80.20	84.77		84.81	0.000585	1.56	178.89	178.68	0.17
Main Reach	0.55	Max WS	exist 100	428.45	80.20	85.30		85.37	0.000953	2.24	293.88	243.00	0.22
Main Reach	0.5	Max WS	exist 10	215.49	79.65	84.05		84.24	0.004211	3.45	70.66	197.95	0.41
Main Reach	0.5	Max WS	exist 100	427.45	79.65	84.65		84.81	0.003654	3.79	201.51	238.63	0.40
Main Reach	0.45	Max WS	exist 10	215.53	79.65	83.50		83.57	0.001473	2.29	151.11	202.68	0.25
Main Reach	0.45	Max WS	exist 100	424.98	79.65	84.14		84.20	0.001416	2.61	298.12	256.89	0.26

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	0.40	Max WS	exist 10	215.63	79.65	83.34		83.36	0.000541	1.30	217.17	283.58	0.15
Main Reach	0.40	Max WS	exist 100	424.16	79.65	83.97		84.00	0.000570	1.58	426.73	376.88	0.16
Main Reach	0.35	Max WS	exist 10	215.77	79.23	83.27		83.29	0.000300	1.07	229.63	207.79	0.12
Main Reach	0.35	Max WS	exist 100	424.37	79.23	83.88		83.91	0.000458	1.53	362.65	228.52	0.15
Main Reach	0.30	Max WS	exist 10	215.77	79.23	83.24		83.26	0.000329	1.05	207.70	114.85	0.12
Main Reach	0.30	Max WS	exist 100	424.22	79.23	83.83		83.87	0.000572	1.61	294.75	182.55	0.17
Main Reach	0.25	Max WS	exist 10	216.30	79.23	82.98		83.09	0.002102	2.68	80.78	32.97	0.30
Main Reach	0.25	Max WS	exist 100	424.24	79.23	83.56		83.68	0.002255	3.19	235.54	285.27	0.32
Main Reach	0.20	Max WS	exist 10	216.71	79.02	82.83		82.89	0.001238	2.15	179.88	276.03	0.24
Main Reach	0.20	Max WS	exist 100	424.69	79.02	83.41		83.45	0.001091	2.31	364.14	329.91	0.23
Main Reach	0.15	Max WS	exist 10	217.42	78.73	82.65		82.68	0.000685	1.67	222.80	237.18	0.18
Main Reach	0.15	Max WS	exist 100	426.10	78.73	83.22		83.26	0.000815	2.06	358.51	242.37	0.20
Main Reach	0.10	Max WS	exist 10	217.80	78.71	82.55		82.59	0.000985	1.94	197.30	243.90	0.21
Main Reach	0.10	Max WS	exist 100	426.83	78.71	83.11		83.15	0.001065	2.29	334.09	248.35	0.23
Main Reach	0.05	Max WS	exist 10	218.57	78.19	82.22		82.31	0.001713	2.54	143.29	336.37	0.27
Main Reach	0.05	Max WS	exist 100	428.65	78.19	82.85		82.91	0.001173	2.42	358.11	340.44	0.23
Main Reach	0.04	Max WS	exist 10	219.42	77.40	81.38		81.58	0.004725	3.68	68.62	83.03	0.44
Main Reach	0.04	Max WS	exist 100	430.49	77.40	82.06		82.30	0.004698	4.40	158.95	178.01	0.46
Main Reach	0.03	Max WS	exist 10	220.10	76.67	80.80		80.86	0.001933	2.06	106.88	62.42	0.28
Main Reach	0.03	Max WS	exist 100	431.68	76.67	81.36		81.47	0.002700	2.80	188.22	204.04	0.34
Main Reach	0.02	Max WS	exist 10	197.58	76.20	80.25		80.34	0.002074	2.55	127.66	285.12	0.30
Main Reach	0.02	Max WS	exist 100	432.95	76.20	80.94		80.99	0.001445	2.53	356.11	382.15	0.26
Main Reach	0.01	Max WS	exist 10	191.07	75.74	79.96		79.99	0.000924	1.51	196.72	237.64	0.19
Main Reach	0.01	Max WS	exist 100	435.05	75.74	80.72		80.74	0.000692	1.62	470.44	420.36	0.18
Main Reach	0.009	Max WS	exist 10	191.47	75.08	79.89		79.90	0.000428	1.37	273.55	260.91	0.14
Main Reach	0.009	Max WS	exist 100	436.26	75.08	80.66		80.67	0.000420	1.58	570.21	472.16	0.14
Main Reach	0.008	Max WS	exist 10	193.26	73.85	79.76		79.80	0.000458	1.59	125.29	46.21	0.15
Main Reach	0.008	Max WS	exist 100	300.53	73.85	80.54		80.58	0.000426	1.70	306.25	364.64	0.15
Main Reach	0.007	Max WS	exist 10	192.90	73.00	79.71		79.72	0.000196	1.15	209.47	87.32	0.10
Main Reach	0.007	Max WS	exist 100	300.51	73.00	80.49		80.51	0.000212	1.30	347.93	248.33	0.11
Federico Trib	1.6	Max WS	exist 10	27.65	117.18	117.59		117.60	0.005080	0.82	36.66	163.31	0.32
Federico Trib	1.6	Max WS	exist 100	55.41	117.18	117.72		117.73	0.005146	0.99	59.87	200.49	0.34
Federico Trib	1.58333*	Max WS	exist 10	27.64	116.48	117.00		117.02	0.005759	1.02	29.87	135.98	0.35
Federico Trib	1.58333*	Max WS	exist 100	55.32	116.48	117.15		117.17	0.005294	1.16	52.99	175.86	0.35
Federico Trib	1.56666*	Max WS	exist 10	27.63	115.79	116.36		116.38	0.006078	1.12	24.74	86.52	0.37
Federico Trib	1.56666*	Max WS	exist 100	55.30	115.79	116.54		116.56	0.005880	1.31	42.14	112.92	0.38
Federico Trib	1.55*	Max WS	exist 10	27.62	115.09	115.71		115.73	0.005959	1.17	23.71	76.70	0.37
Federico Trib	1.55*	Max WS	exist 100	55.27	115.09	115.90		115.93	0.005759	1.37	40.40	100.12	0.38
Federico Trib	1.53333*	Max WS	exist 10	27.61	114.39	115.07		115.09	0.005753	1.22	22.67	66.86	0.37
Federico Trib	1.53333*	Max WS	exist 100	55.26	114.39	115.28		115.31	0.005385	1.41	39.11	87.80	0.37
Federico Trib	1.51666*	Max WS	exist 10	27.60	113.70	114.46		114.49	0.005298	1.27	21.81	57.06	0.36
Federico Trib	1.51666*	Max WS	exist 100	55.24	113.70	114.68		114.72	0.005463	1.53	36.08	78.33	0.38
Federico Trib	1.5	Max WS	exist 10	27.60	113.00	113.87		113.90	0.005552	1.41	19.59	45.21	0.38
Federico Trib	1.5	Max WS	exist 100	55.16	113.00	114.10		114.14	0.005141	1.68	38.63	136.44	0.38
Federico Trib	1.48333*	Max WS	exist 10	27.59	112.67	113.33		113.35	0.004656	1.25	22.06	53.33	0.34
Federico Trib	1.48333*	Max WS	exist 100	55.13	112.67	113.57		113.60	0.004808	1.51	36.40	67.63	0.36
Federico Trib	1.46666*	Max WS	exist 10	27.57	112.33	112.87		112.89	0.004040	1.11	24.79	64.22	0.32
Federico Trib	1.46666*	Max WS	exist 100	55.09	112.33	113.09		113.12	0.004201	1.37	40.33	79.08	0.34
Federico Trib	1.45*	Max WS	exist 10	27.56	112.00	112.47		112.48	0.003688	1.01	27.29	76.32	0.30
Federico Trib	1.45*	Max WS	exist 100	55.04	112.00	112.66		112.69	0.003863	1.26	43.80	91.41	0.32
Federico Trib	1.43333*	Max WS	exist 10	27.55	111.67	112.08		112.09	0.003662	0.95	29.01	88.54	0.29
Federico Trib	1.43333*	Max WS	exist 100	55.00	111.67	112.26		112.28	0.003789	1.19	46.33	103.81	0.31
Federico Trib	1.41666*	Max WS	exist 10	27.53	111.33	111.70		111.71	0.003532	0.89	30.97	101.58	0.28
Federico Trib	1.41666*	Max WS	exist 100	54.93	111.33	111.86		111.88	0.003615	1.11	49.28	117.14	0.30
Federico Trib	1.4	Max WS	exist 10	27.53	111.00	111.34		111.35	0.003306	0.83	33.22	115.27	0.27
Federico Trib	1.4	Max WS	exist 100	54.92	111.00	111.51		111.52	0.003096	1.01	54.22	132.40	0.28
Federico Trib	1.38*	Max WS	exist 10	34.20	110.60	110.98		110.99	0.003318	0.89	38.52	120.77	0.28
Federico Trib	1.38*	Max WS	exist 100	67.35	110.60	111.16		111.18	0.003155	1.08	62.17	139.23	0.29

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	1.36*	Max WS	exist 10	40.95	110.20	110.62		110.63	0.003339	0.94	43.55	125.86	0.28
Federico Trib	1.36*	Max WS	exist 100	79.90	110.20	110.81		110.83	0.003203	1.14	69.80	145.57	0.29
Federico Trib	1.34*	Max WS	exist 10	47.70	109.80	110.25		110.26	0.003465	1.00	47.84	130.22	0.29
Federico Trib	1.34*	Max WS	exist 100	92.45	109.80	110.45		110.48	0.003216	1.20	77.34	151.64	0.29
Federico Trib	1.32*	Max WS	exist 10	54.44	109.40	109.90		109.92	0.002830	0.97	56.26	137.59	0.27
Federico Trib	1.32*	Max WS	exist 100	105.00	109.40	110.07		110.09	0.003749	1.31	80.33	154.52	0.32
Federico Trib	1.3	Max WS	exist 10	61.10	109.00	109.44		109.47	0.005466	1.25	48.80	194.55	0.36
Federico Trib	1.3	Max WS	exist 100	117.42	109.00	109.61		109.64	0.004557	1.38	97.33	240.20	0.35
Federico Trib	1.28*	Max WS	exist 10	61.07	108.40	108.90		108.92	0.005208	1.29	47.29	118.53	0.36
Federico Trib	1.28*	Max WS	exist 100	117.29	108.40	109.11		109.15	0.004981	1.55	78.47	172.62	0.37
Federico Trib	1.26*	Max WS	exist 10	61.02	107.80	108.36		108.39	0.005248	1.36	44.75	103.96	0.37
Federico Trib	1.26*	Max WS	exist 100	117.14	107.80	108.61		108.65	0.004742	1.59	73.56	125.49	0.37
Federico Trib	1.24*	Max WS	exist 10	60.91	107.20	107.91		107.94	0.003554	1.25	48.57	95.48	0.31
Federico Trib	1.24*	Max WS	exist 100	117.08	107.20	108.17		108.21	0.003881	1.55	75.45	115.14	0.34
Federico Trib	1.22*	Max WS	exist 10	60.89	106.60	107.68		107.70	0.001192	0.91	67.03	100.27	0.19
Federico Trib	1.22*	Max WS	exist 100	116.84	106.60	107.88		107.91	0.001910	1.36	88.71	118.94	0.25
Federico Trib	1.20	Max WS	exist 10	60.81	106.00	107.61		107.61	0.000372	0.71	96.73	120.69	0.12
Federico Trib	1.20	Max WS	exist 100	115.68	106.00	107.74		107.76	0.000889	1.18	114.03	132.33	0.18
Federico Trib	1.10	Max WS	exist 10	75.98	104.00	106.22		106.28	0.004585	2.36	58.14	195.91	0.40
Federico Trib	1.10	Max WS	exist 100	144.16	104.00	106.49		106.54	0.003480	2.39	118.05	236.06	0.36
Federico Trib	1.0	Max WS	exist 10	95.22	102.98	105.05		105.06	0.000543	1.04	135.53	189.64	0.15
Federico Trib	1.0	Max WS	exist 100	184.69	102.98	105.56		105.57	0.000562	1.28	240.75	228.64	0.16
Federico Trib	0.99	Max WS	exist 10	95.18	102.09	103.82		104.00	0.008055	3.48	32.34	37.99	0.54
Federico Trib	0.99	Max WS	exist 100	184.41	102.09	104.26		104.49	0.008409	4.33	67.22	107.83	0.58
Federico Trib	0.98	Max WS	exist 10	100.49	101.00	102.37		102.44	0.003821	2.19	56.00	85.90	0.37
Federico Trib	0.98	Max WS	exist 100	193.63	101.00	102.81		102.89	0.003572	2.63	97.75	106.77	0.37
Federico Trib	0.97	Max WS	exist 10	106.31	100.00	101.75		101.76	0.000521	0.90	135.54	152.04	0.14
Federico Trib	0.97	Max WS	exist 100	203.26	100.00	102.22		102.24	0.000592	1.17	217.76	208.60	0.16
Federico Trib	0.96	Max WS	exist 10	110.19	98.75	101.21		101.34	0.004479	3.14	53.66	86.28	0.42
Federico Trib	0.96	Max WS	exist 100	210.05	98.75	101.66		101.80	0.004532	3.67	101.43	128.15	0.44
Federico Trib	0.95	Max WS	exist 10	115.84	97.82	100.22		100.27	0.002455	2.15	82.52	115.61	0.30
Federico Trib	0.95	Max WS	exist 100	219.76	97.82	100.61		100.68	0.002737	2.63	132.91	144.13	0.33
Federico Trib	0.94	Max WS	exist 10	121.43	97.03	99.65		99.67	0.001078	1.41	161.50	226.22	0.20
Federico Trib	0.94	Max WS	exist 100	229.06	97.03	99.96		99.99	0.001293	1.75	233.25	234.54	0.23
Federico Trib	0.937	Max WS	exist 10	120.86	96.60	99.23		99.25	0.001338	1.19	160.44	356.61	0.21
Federico Trib	0.937	Max WS	exist 100	227.53	96.60	99.46		99.48	0.001602	1.50	246.25	396.70	0.24
Federico Trib	0.93	Max WS	exist 10	139.62	95.99	98.34		98.41	0.004461	2.69	106.46	260.15	0.40
Federico Trib	0.93	Max WS	exist 100	255.01	95.99	98.61		98.67	0.003878	2.83	180.26	278.34	0.39
Federico Trib	0.925	Max WS	exist 10	149.34	95.00	97.74		97.76	0.001544	0.95	156.86	246.56	0.21
Federico Trib	0.925	Max WS	exist 100	271.92	95.00	98.08		98.10	0.001441	1.07	255.07	332.08	0.21
Federico Trib	0.92	Max WS	exist 10	149.22	94.89	97.21		97.29	0.003411	2.97	101.39	176.59	0.37
Federico Trib	0.92	Max WS	exist 100	271.53	94.89	97.60		97.68	0.002999	3.14	173.35	189.40	0.36
Federico Trib	0.91	Max WS	exist 10	153.58	94.51	96.63		96.64	0.000844	0.93	199.36	190.39	0.17
Federico Trib	0.91	Max WS	exist 100	278.77	94.51	97.07		97.09	0.000899	1.10	285.88	201.88	0.18
Federico Trib	0.90	Max WS	exist 10	158.67	93.00	96.09		96.15	0.002148	2.41	123.08	167.61	0.29
Federico Trib	0.90	Max WS	exist 100	287.44	93.00	96.56		96.62	0.001916	2.61	209.53	197.63	0.29
Federico Trib	0.89	Max WS	exist 10	162.28	93.00	95.23		95.35	0.004541	3.36	76.57	100.79	0.43
Federico Trib	0.89	Max WS	exist 100	293.65	93.00	95.65		95.81	0.005036	4.05	123.80	123.71	0.47
Federico Trib	0.88	Max WS	exist 10	168.07	91.00	93.61		93.68	0.003806	2.51	105.33	142.52	0.37
Federico Trib	0.88	Max WS	exist 100	303.66	91.00	94.03		94.10	0.003412	2.83	166.67	151.88	0.37
Federico Trib	0.875	Max WS	exist 10	171.14	90.70	93.07		93.10	0.001164	1.24	142.27	181.90	0.20
Federico Trib	0.875	Max WS	exist 100	309.36	90.70	93.54		93.57	0.001122	1.50	232.52	203.85	0.21
Federico Trib	0.87	Max WS	exist 10	176.48	90.00	92.59		92.63	0.001200	1.65	134.74	179.50	0.22
Federico Trib	0.87	Max WS	exist 100	319.16	90.00	93.01		93.06	0.001457	2.10	227.36	264.76	0.25
Federico Trib	0.86	Max WS	exist 10	179.77	88.30	91.90		92.01	0.003985	2.71	66.37	43.12	0.38
Federico Trib	0.86	Max WS	exist 100	325.09	88.30	92.37		92.48	0.003498	2.99	164.66	225.07	0.38
Federico Trib	0.85	Max WS	exist 10	183.07	87.91	91.16		91.25	0.002117	2.65	113.45	160.00	0.30

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	0.85	Max WS	exist 100	331.17	87.91	91.72		91.80	0.001874	2.87	213.60	199.78	0.29
Federico Trib	0.84	Max WS	exist 10	187.58	87.92	90.68		90.71	0.000863	1.76	183.41	144.02	0.20
Federico Trib	0.84	Max WS	exist 100	339.64	87.92	91.25		91.28	0.000977	2.14	268.11	157.21	0.22
Federico Trib	0.83	Max WS	exist 10	190.74	86.77	90.11		90.25	0.003100	3.15	82.74	125.82	0.37
Federico Trib	0.83	Max WS	exist 100	345.64	86.77	90.69		90.83	0.002896	3.53	158.23	134.39	0.37
Federico Trib	0.82	Max WS	exist 10	190.72	86.01	89.42		89.52	0.002621	2.81	98.01	93.33	0.33
Federico Trib	0.82	Max WS	exist 100	345.55	86.01	89.88		90.03	0.003452	3.65	143.41	104.21	0.39
Federico Trib	0.81	Max WS	exist 10	194.01	85.71	89.05		89.09	0.001564	1.93	184.66	265.14	0.25
Federico Trib	0.81	Max WS	exist 100	351.44	85.71	89.47		89.51	0.001419	2.10	300.28	280.98	0.24
Federico Trib	0.807	Max WS	exist 10	201.91	84.70	88.23		88.25	0.001007	1.33	199.06	201.28	0.19
Federico Trib	0.807	Max WS	exist 100	365.56	84.70	88.76		88.78	0.000911	1.56	319.00	251.84	0.20
Federico Trib	0.80	Max WS	exist 10	207.68	84.46	87.94		87.96	0.000553	1.31	234.78	152.67	0.15
Federico Trib	0.80	Max WS	exist 100	376.04	84.46	88.45		88.47	0.000754	1.75	312.86	157.52	0.19
Federico Trib	0.79	Max WS	exist 10	207.34	83.77	87.39		87.46	0.002449	2.29	124.26	176.72	0.31
Federico Trib	0.79	Max WS	exist 100	375.50	83.77	87.87		87.95	0.002373	2.58	217.42	207.11	0.32
Federico Trib	0.78	Max WS	exist 10	212.33	82.76	86.87		86.89	0.000598	1.41	271.79	267.46	0.16
Federico Trib	0.78	Max WS	exist 100	385.16	82.76	87.35		87.38	0.000669	1.65	414.04	319.49	0.17
Federico Trib	0.77	Max WS	exist 10	216.53	82.68	86.04		86.19	0.004488	3.53	99.18	128.12	0.43
Federico Trib	0.77	Max WS	exist 100	393.01	82.68	86.55		86.70	0.004201	3.95	186.12	209.22	0.43
Federico Trib	0.76	Max WS	exist 10	221.46	81.79	85.06		85.09	0.001162	2.02	195.82	174.80	0.23
Federico Trib	0.76	Max WS	exist 100	402.35	81.79	85.60		85.65	0.001238	2.38	302.18	214.18	0.24
Federico Trib	.75875*	Max WS	exist 10	221.42	81.64	84.96		85.00	0.001296	2.17	186.09	178.11	0.24
Federico Trib	.75875*	Max WS	exist 100	402.27	81.64	85.50		85.55	0.001343	2.51	292.18	214.68	0.25
Federico Trib	.7575*	Max WS	exist 10	221.38	81.49	84.84		84.90	0.001525	2.36	171.46	180.89	0.26
Federico Trib	.7575*	Max WS	exist 100	402.18	81.49	85.39		85.45	0.001511	2.68	277.74	214.17	0.27
Federico Trib	.75625*	Max WS	exist 10	221.32	81.35	84.70		84.78	0.001907	2.63	151.05	179.28	0.29
Federico Trib	.75625*	Max WS	exist 100	402.10	81.35	85.26		85.33	0.001774	2.90	258.47	211.30	0.29
Federico Trib	0.755	Max WS	exist 10	221.31	81.20	84.57		84.64	0.001735	2.50	143.01	129.43	0.28
Federico Trib	0.755	Max WS	exist 100	402.02	81.20	85.09		85.19	0.002258	3.23	230.90	205.64	0.33
Federico Trib	.754*	Max WS	exist 10	221.29	80.96	84.44		84.52	0.001968	2.57	143.40	150.27	0.29
Federico Trib	.754*	Max WS	exist 100	401.97	80.96	84.94		85.03	0.002218	3.10	234.40	198.96	0.32
Federico Trib	.753*	Max WS	exist 10	221.27	80.72	84.31		84.38	0.002131	2.56	146.12	165.53	0.30
Federico Trib	.753*	Max WS	exist 100	401.93	80.72	84.80		84.88	0.002214	2.98	237.04	196.40	0.32
Federico Trib	.752*	Max WS	exist 10	221.27	80.48	84.16		84.23	0.002274	2.52	147.65	169.11	0.31
Federico Trib	.752*	Max WS	exist 100	401.89	80.48	84.65		84.73	0.002295	2.91	236.70	195.22	0.32
Federico Trib	.751*	Max WS	exist 10	221.26	80.24	84.00		84.07	0.002594	2.65	136.53	150.66	0.33
Federico Trib	.751*	Max WS	exist 100	401.87	80.24	84.46		84.56	0.003005	3.16	216.21	194.12	0.36
Federico Trib	0.75	Max WS	exist 10	221.25	80.00	83.78		83.88	0.003478	3.02	121.41	151.47	0.37
Federico Trib	0.75	Max WS	exist 100	401.85	80.00	84.22		84.33	0.003780	3.44	198.05	186.09	0.40
Federico Trib	.7494*	Max WS	exist 10	225.32	80.00	83.57		83.64	0.003078	2.75	147.40	181.95	0.35
Federico Trib	.7494*	Max WS	exist 100	409.57	80.00	84.01		84.09	0.003119	2.99	230.91	196.50	0.36
Federico Trib	.7488*	Max WS	exist 10	225.27	80.00	83.36		83.42	0.003321	2.75	151.25	189.76	0.36
Federico Trib	.7488*	Max WS	exist 100	409.46	80.00	83.80		83.87	0.003457	3.04	239.86	227.83	0.37
Federico Trib	.7482*	Max WS	exist 10	225.23	80.00	83.18		83.22	0.002537	2.32	182.35	222.53	0.31
Federico Trib	.7482*	Max WS	exist 100	409.30	80.00	83.63		83.67	0.002238	2.37	285.60	233.46	0.30
Federico Trib	.7476*	Max WS	exist 10	225.18	80.00	83.00		83.04	0.002679	2.30	180.50	216.18	0.31
Federico Trib	.7476*	Max WS	exist 100	409.17	80.00	83.48		83.52	0.002204	2.29	286.72	227.12	0.29
Federico Trib	0.747	Max WS	exist 10	225.03	80.00	82.81		82.85	0.002875	2.28	177.52	210.60	0.32
Federico Trib	0.747	Max WS	exist 100	409.03	80.00	83.34		83.37	0.002099	2.17	289.95	219.94	0.28
Federico Trib	0.744	Max WS	exist 10	224.89	79.00	82.56		82.63	0.001574	2.59	164.28	198.64	0.27
Federico Trib	0.744	Max WS	exist 100	408.86	79.00	83.13		83.20	0.001513	2.86	281.47	214.32	0.27
Federico Trib	0.74	Max WS	exist 10	224.79	77.38	81.88		81.93	0.001056	2.08	186.61	219.11	0.22
Federico Trib	0.74	Max WS	exist 100	408.62	77.38	82.48		82.53	0.000997	2.28	343.88	288.65	0.22
Federico Trib	0.73	Max WS	exist 10	260.83	76.00	81.37		81.47	0.001690	2.94	164.49	197.84	0.28
Federico Trib	0.73	Max WS	exist 100	476.30	76.00	82.00		82.09	0.001636	3.22	299.44	233.98	0.28
Federico Trib	0.72	Max WS	exist 10	264.40	76.00	80.85		80.92	0.001517	2.36	168.71	199.79	0.26
Federico Trib	0.72	Max WS	exist 100	483.14	76.00	81.52		81.59	0.001267	2.51	347.99	307.63	0.25

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	0.71	Max WS	exist 10	268.24	75.00	80.41		80.46	0.000963	2.08	214.09	223.46	0.21
Federico Trib	0.71	Max WS	exist 100	490.94	75.00	81.15		81.20	0.000829	2.23	403.72	279.37	0.20
Federico Trib	0.70	Max WS	exist 10	269.84	75.00	80.24		80.30	0.001073	2.24	182.67	155.48	0.22
Federico Trib	0.70	Max WS	exist 100	494.28	75.00	80.98		81.05	0.001113	2.63	307.51	181.73	0.24
Federico Trib	.65*	Max WS	exist 10	271.08	75.00	80.06		80.14	0.001415	2.51	164.21	133.38	0.25
Federico Trib	.65*	Max WS	exist 100	497.05	75.00	80.81		80.89	0.001394	2.88	311.34	265.65	0.26
Federico Trib	0.6	Max WS	exist 10	272.06	75.00	79.89		79.95	0.001362	2.40	209.23	220.49	0.25
Federico Trib	0.6	Max WS	exist 100	499.81	75.00	80.68		80.72	0.000879	2.26	434.11	299.02	0.21
Federico Trib	0.5	Max WS	exist 10	273.28	74.00	79.80		79.81	0.000105	0.82	451.19	229.01	0.07
Federico Trib	0.5	Max WS	exist 100	503.14	74.00	80.62		80.63	0.000148	1.10	666.23	280.28	0.09
Federico Trib	0.4	Max WS	exist 10	272.80	73.64	79.71		79.78	0.000968	2.32	164.81	110.36	0.21
Federico Trib	0.4	Max WS	exist 100	503.04	73.64	80.49		80.58	0.001267	2.89	251.72	112.35	0.25
Down	6.9	Max WS	exist 10	465.70	72.00	79.71		79.76	0.000737	1.92	303.09	205.08	0.15
Down	6.9	Max WS	exist 100	803.55	72.00	80.49		80.54	0.000801	2.15	681.58	654.44	0.16
Down	6.85*	Max WS	exist 10	465.69	71.71	79.63		79.69	0.000842	2.03	303.88	196.48	0.16
Down	6.85*	Max WS	exist 100	803.53	71.71	80.39		80.46	0.001084	2.48	511.35	353.69	0.18
Down	6.8	Max WS	exist 10	465.69	71.43	79.54	75.51	79.61	0.001061	2.23	278.10	155.52	0.17
Down	6.8	Max WS	exist 100	803.53	71.43	80.26	76.61	80.35	0.001460	2.82	419.79	256.40	0.21

**APPENDIX F
HEC-RAS POST-PROJECT
MITIGATED AND
FUTURE, FULLY DEVELOPED, UNMITIGATED
SUMMARY TABLE**

HEC-RAS Profile: Max WS

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	1.4	Max WS	10yr mit	48.20	109.90	110.82		110.82	0.003371	0.70	68.65	130.67	0.17
Federico Trib	1.4	Max WS	100yr mit	94.14	109.90	111.09		111.11	0.003325	0.87	108.91	152.20	0.18
Federico Trib	1.375*	Max WS	10yr mit	48.12	109.66	110.51		110.52	0.003277	0.69	70.12	135.17	0.17
Federico Trib	1.375*	Max WS	100yr mit	93.96	109.66	110.78		110.79	0.003446	0.85	111.04	165.71	0.18
Federico Trib	1.35*	Max WS	10yr mit	48.01	109.43	110.20		110.21	0.003435	0.69	69.96	143.52	0.17
Federico Trib	1.35*	Max WS	100yr mit	93.71	109.43	110.46		110.47	0.003614	0.84	113.07	186.12	0.18
Federico Trib	1.325*	Max WS	10yr mit	47.89	109.19	109.90		109.91	0.003049	0.64	76.43	166.85	0.16
Federico Trib	1.325*	Max WS	100yr mit	93.50	109.19	110.14		110.15	0.003376	0.80	120.42	204.41	0.18
Federico Trib	1.3	Max WS	10yr mit	47.84	108.95	109.54		109.55	0.004818	0.75	64.78	178.72	0.20
Federico Trib	1.3	Max WS	100yr mit	93.43	108.95	109.78		109.79	0.004420	0.88	111.75	216.03	0.20
Federico Trib	1.275*	Max WS	10yr mit	51.88	108.21	108.94		108.95	0.004717	0.80	65.27	143.43	0.20
Federico Trib	1.275*	Max WS	100yr mit	100.60	108.21	109.21		109.22	0.004358	0.93	112.72	201.57	0.20
Federico Trib	1.25*	Max WS	10yr mit	55.87	107.47	108.37		108.38	0.004496	0.85	66.09	118.14	0.20
Federico Trib	1.25*	Max WS	100yr mit	107.60	107.47	108.68		108.69	0.003865	1.01	109.59	180.90	0.20
Federico Trib	1.225*	Max WS	10yr mit	59.84	106.74	107.92		107.93	0.002504	0.78	77.73	115.32	0.16
Federico Trib	1.225*	Max WS	100yr mit	114.57	106.74	108.27		108.29	0.002431	0.99	124.85	156.25	0.16
Federico Trib	1.2	Max WS	10yr mit	63.63	106.00	107.71		107.71	0.000898	0.65	109.46	129.36	0.10
Federico Trib	1.2	Max WS	100yr mit	121.01	106.00	108.04		108.05	0.001340	0.93	157.61	183.15	0.13
Federico Trib	1.18*	Max WS	10yr mit	76.20	105.50	107.53		107.55	0.001873	0.95	94.68	134.95	0.15
Federico Trib	1.18*	Max WS	100yr mit	144.04	105.50	107.76		107.78	0.003227	1.39	126.83	146.67	0.20
Federico Trib	1.161	Max WS	10yr mit	78.41	105.00	107.44		107.44	0.000406	0.44	189.70	163.89	0.07
Federico Trib	1.161	Max WS	100yr mit	147.90	105.00	107.58		107.59	0.000988	0.74	213.54	165.85	0.11
Federico Trib	1.16	Max WS	10yr mit	78.73	105.00	107.43			0.000413	0.45	189.13	163.84	0.07
Federico Trib	1.16	Max WS	100yr mit	148.50	105.00	107.57			0.001020	0.75	211.95	165.71	0.11
Federico Trib	1.15			Mult Open									
Federico Trib	1.14	Max WS	10yr mit	78.72	105.00	106.92		106.92	0.000577	0.60	169.86	168.71	0.08
Federico Trib	1.14	Max WS	100yr mit	147.87	105.00	107.19		107.20	0.001177	0.95	243.19	310.18	0.12
Federico Trib	1.139	Max WS	10yr mit	79.05	104.80	106.91		106.92	0.000580	0.61	169.69	168.67	0.08
Federico Trib	1.139	Max WS	100yr mit	148.43	104.80	107.18		107.19	0.001218	0.97	240.28	309.62	0.13
Federico Trib	1.1	Max WS	10yr mit	83.96	104.00	106.38		106.40	0.005181	1.55	91.88	219.43	0.24
Federico Trib	1.1	Max WS	100yr mit	151.03	104.00	106.68		106.69	0.003733	1.51	163.31	262.35	0.21
Federico Trib	1.0	Max WS	10yr mit	76.54	102.98	105.02		105.03	0.000915	0.75	129.69	187.24	0.11
Federico Trib	1.0	Max WS	100yr mit	130.05	102.98	105.86		105.86	0.000313	0.59	312.05	240.91	0.07
Federico Trib	.999*	Max WS	10yr mit	76.49	102.79	104.99		105.00	0.000725	0.67	152.08	187.02	0.09
Federico Trib	.999*	Max WS	100yr mit	130.03	102.79	105.85		105.85	0.000251	0.53	342.41	259.39	0.06
Federico Trib	.998*	Max WS	10yr mit	76.45	102.59	104.97		104.97	0.000605	0.61	165.61	188.06	0.09
Federico Trib	.998*	Max WS	100yr mit	129.94	102.59	105.84		105.84	0.000207	0.48	361.72	253.57	0.05
Federico Trib	.9971	Max WS	10yr mit	76.44	102.40	104.95		104.95	0.000563	0.59	164.49	167.28	0.08
Federico Trib	.9971	Max WS	100yr mit	129.92	102.40	105.84		105.84	0.000218	0.49	323.38	190.08	0.06
Federico Trib	0.997	Max WS	10yr mit	76.44	102.40	104.94		104.95	0.000572	0.59	163.54	167.00	0.08
Federico Trib	0.997	Max WS	100yr mit	129.92	102.40	105.83		105.84	0.000219	0.49	322.96	190.02	0.06
Federico Trib	.9954			Culvert									
Federico Trib	0.993	Max WS	10yr mit	76.42	102.20	103.92		103.95	0.006443	1.27	60.26	76.74	0.25
Federico Trib	0.993	Max WS	100yr mit	129.70	102.20	104.25		104.28	0.005478	1.44	95.84	121.09	0.25
Federico Trib	.9929	Max WS	10yr mit	76.40	102.20	103.84		103.88	0.008439	1.41	54.36	72.64	0.29
Federico Trib	.9929	Max WS	100yr mit	129.61	102.20	104.19		104.22	0.006960	1.56	87.78	118.78	0.27
Federico Trib	0.98	Max WS	10yr mit	75.98	101.00	102.69		102.71	0.002100	1.08	86.19	101.42	0.16
Federico Trib	0.98	Max WS	100yr mit	128.56	101.00	103.35		103.36	0.001110	1.01	165.95	137.60	0.12
Federico Trib	.9799			Lat Struct									
Federico Trib	.979*	Max WS	10yr mit	58.17	100.80	102.64		102.65	0.001277	0.82	87.01	96.17	0.12
Federico Trib	.979*	Max WS	100yr mit	98.97	100.80	103.32		103.33	0.000693	0.77	163.11	126.40	0.09
Federico Trib	.978*	Max WS	10yr mit	58.16	100.60	102.59		102.60	0.001115	0.76	91.93	89.60	0.10
Federico Trib	.978*	Max WS	100yr mit	98.94	100.60	103.29		103.30	0.000714	0.77	168.23	137.00	0.09
Federico Trib	.9771	Max WS	10yr mit	58.15	100.40	102.57		102.57	0.000746	0.62	113.36	107.03	0.08
Federico Trib	.9771	Max WS	100yr mit	98.94	100.40	103.28		103.28	0.000485	0.61	209.73	180.73	0.07
Federico Trib	0.977	Max WS	10yr mit	58.15	100.40	102.56		102.57	0.000761	0.62	112.55	106.72	0.08
Federico Trib	0.977	Max WS	100yr mit	98.94	100.40	103.27		103.28	0.000490	0.62	208.85	180.21	0.07

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	.908*	Max WS	10yr mit	90.33	94.20	96.56		96.57	0.001844	1.08	130.16	170.77	0.15
Federico Trib	.908*	Max WS	100yr mit	160.70	94.20	96.97		96.98	0.001550	1.15	201.28	178.58	0.14
Federico Trib	.906*	Max WS	10yr mit	90.33	93.90	96.35		96.37	0.004014	1.62	80.66	91.84	0.22
Federico Trib	.906*	Max WS	100yr mit	160.67	93.90	96.74		96.77	0.004567	1.99	128.15	164.70	0.24
Federico Trib	.904*	Max WS	10yr mit	90.32	93.60	96.00		96.04	0.005773	1.88	72.57	99.32	0.26
Federico Trib	.904*	Max WS	100yr mit	160.65	93.60	96.35		96.39	0.006688	2.30	113.08	136.54	0.29
Federico Trib	.902*	Max WS	10yr mit	90.28	93.30	95.55		95.59	0.007601	1.98	75.63	128.29	0.29
Federico Trib	.902*	Max WS	100yr mit	160.60	93.30	95.89		95.93	0.006901	2.18	124.54	154.66	0.29
Federico Trib	0.90	Max WS	10yr mit	90.22	93.00	95.19		95.21	0.003348	1.26	116.58	156.74	0.19
Federico Trib	0.90	Max WS	100yr mit	160.55	93.00	95.56		95.58	0.003063	1.42	176.64	167.81	0.19
Federico Trib	0.89	Max WS	10yr mit	91.08	93.00	94.37		94.39	0.003411	0.87	104.39	110.84	0.18
Federico Trib	0.89	Max WS	100yr mit	162.11	93.00	94.77		94.79	0.003423	1.08	150.07	116.66	0.19
Federico Trib	0.88	Max WS	10yr mit	90.47	91.00	93.21		93.22	0.002447	1.18	101.49	134.42	0.17
Federico Trib	0.88	Max WS	100yr mit	141.02	91.00	93.85		93.86	0.001122	1.01	191.19	147.17	0.12
Federico Trib	.8799			Lat Struct									
Federico Trib	.875	Max WS	10yr mit	103.30	90.70	92.95		92.95	0.000588	0.64	275.08	313.73	0.09
Federico Trib	.875	Max WS	100yr mit	157.35	90.70	93.74		93.74	0.000186	0.46	595.39	464.84	0.05
Federico Trib	0.87	Max WS	10yr mit	71.24	90.00	92.89		92.89	0.000160	0.38	287.83	335.90	0.05
Federico Trib	0.87	Max WS	100yr mit	156.97	90.00	93.71		93.72	0.000111	0.39	591.05	383.39	0.04
Federico Trib	0.868	Max WS	10yr mit	105.38	89.01	92.44		92.46	0.002859	1.24	99.28	140.29	0.18
Federico Trib	0.868	Max WS	100yr mit	161.66	89.01	93.66		93.67	0.000280	0.59	381.11	277.98	0.06
Federico Trib	0.867	Max WS	10yr mit	105.38	89.00	92.36		92.39	0.003987	1.50	87.77	144.99	0.22
Federico Trib	0.867	Max WS	100yr mit	161.66	89.00	93.66		93.66	0.000263	0.59	395.04	283.14	0.06
Federico Trib	.865			Culvert									
Federico Trib	0.864	Max WS	10yr mit	105.38	88.45	91.62		91.68	0.004424	1.96	56.74	35.08	0.24
Federico Trib	0.864	Max WS	100yr mit	161.66	88.45	92.04		92.13	0.005638	2.50	78.23	197.41	0.28
Federico Trib	0.86	Max WS	10yr mit	105.37	88.30	91.35		91.45	0.008050	2.59	47.26	49.12	0.32
Federico Trib	0.86	Max WS	100yr mit	161.66	88.30	91.78		91.89	0.007836	2.89	70.74	60.44	0.32
Federico Trib	.856666*	Max WS	10yr mit	105.85	88.17	90.81		90.86	0.005816	1.95	56.95	45.36	0.27
Federico Trib	.856666*	Max WS	100yr mit	162.34	88.17	91.23		91.31	0.005822	2.27	78.24	52.53	0.28
Federico Trib	.853333*	Max WS	10yr mit	105.81	88.03	90.50		90.52	0.002329	1.28	86.52	60.17	0.17
Federico Trib	.853333*	Max WS	100yr mit	162.32	88.03	90.93		90.96	0.002486	1.54	114.10	70.96	0.18
Federico Trib	0.85	Max WS	10yr mit	105.79	87.90	90.39		90.40	0.000579	0.61	210.99	155.71	0.07
Federico Trib	0.85	Max WS	100yr mit	162.31	87.90	90.82		90.83	0.000600	0.71	278.35	159.50	0.08
Federico Trib	.845*	Max WS	10yr mit	107.08	87.55	89.76		89.82	0.006284	1.89	58.97	46.29	0.28
Federico Trib	.845*	Max WS	100yr mit	137.76	87.55	90.21		90.26	0.004096	1.83	82.75	70.12	0.23
Federico Trib	0.84	Max WS	10yr mit	108.35	87.20	89.17		89.18	0.000876	0.60	186.03	127.55	0.10
Federico Trib	0.84	Max WS	100yr mit	150.56	87.20	89.85		89.85	0.000505	0.61	276.04	136.45	0.08
Federico Trib	0.83	Max WS	10yr mit	82.07	86.80	89.04		89.04	0.000178	0.27	332.00	245.88	0.04
Federico Trib	0.83	Max WS	100yr mit	170.39	86.80	89.76		89.76	0.000193	0.38	525.23	290.45	0.05
Federico Trib	0.82	Max WS	10yr mit	110.59	86.00	88.96		88.97	0.000418	0.62	254.54	167.51	0.07
Federico Trib	0.82	Max WS	100yr mit	210.10	86.00	89.68		89.68	0.000451	0.78	377.02	175.60	0.08
Federico Trib	0.81	Max WS	10yr mit	127.60	85.70	88.58		88.61	0.003288	1.96	119.77	182.29	0.21
Federico Trib	0.81	Max WS	100yr mit	231.72	85.70	89.55		89.56	0.000813	1.20	356.45	282.94	0.11
Federico Trib	0.807	Max WS	10yr mit	132.13	84.70	88.11		88.12	0.000101	0.38	523.61	259.83	0.04
Federico Trib	0.807	Max WS	100yr mit	239.97	84.70	89.44		89.44	0.000064	0.39	867.29	259.83	0.03
Federico Trib	0.806	Max WS	10yr mit	155.78	84.72	88.08		88.09	0.000536	0.89	267.67	231.67	0.09
Federico Trib	0.806	Max WS	100yr mit	274.12	84.72	89.42		89.43	0.000175	0.65	592.67	246.47	0.05
Federico Trib	.805			Culvert									
Federico Trib	0.804	Max WS	10yr mit	152.93	84.90	87.60		87.60	0.001041	0.93	220.15	168.01	0.12
Federico Trib	0.804	Max WS	100yr mit	254.39	84.90	88.36		88.37	0.000726	0.97	360.47	197.24	0.10
Federico Trib	0.803	Max WS	10yr mit	152.45	84.46	87.58		87.59	0.000711	0.91	230.21	168.73	0.10
Federico Trib	0.803	Max WS	100yr mit	254.07	84.46	88.35		88.35	0.000561	0.96	365.48	182.27	0.09
Federico Trib	0.80	Max WS	10yr mit	150.35	84.50	87.42		87.43	0.001224	0.96	212.74	149.53	0.12
Federico Trib	0.80	Max WS	100yr mit	251.67	84.50	88.24		88.25	0.000781	0.98	336.78	155.40	0.11
Federico Trib	0.79	Max WS	10yr mit	149.64	83.80	87.05		87.06	0.000955	1.02	196.26	151.04	0.11
Federico Trib	0.79	Max WS	100yr mit	251.72	83.80	88.01		88.01	0.000572	0.98	373.06	215.77	0.09

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	0.777	Max WS	10yr mit	150.54	83.00	86.84		86.85	0.001122	1.28	218.08	233.17	0.13
Federico Trib	0.777	Max WS	100yr mit	253.17	83.00	87.92		87.93	0.000264	0.76	574.86	346.50	0.07
Federico Trib	0.776	Max WS	10yr mit	150.52	82.76	86.79		86.81	0.001020	1.31	204.62	231.03	0.12
Federico Trib	0.776	Max WS	100yr mit	253.16	82.76	87.91		87.92	0.000280	0.83	547.71	345.70	0.07
Federico Trib	.775			Culvert									
Federico Trib	0.774	Max WS	10yr mit	150.30	82.74	86.27		86.30	0.002691	1.76	137.56	168.96	0.19
Federico Trib	0.774	Max WS	100yr mit	250.33	82.74	86.66		86.69	0.002926	2.02	216.05	234.45	0.21
Federico Trib	.7739	Max WS	10yr mit	150.26	82.74	86.24		86.27	0.002887	1.81	132.72	164.06	0.20
Federico Trib	.7739	Max WS	100yr mit	248.32	82.74	86.63		86.67	0.003103	2.06	208.70	229.12	0.21
Federico Trib	0.773	Max WS	10yr mit	150.22	82.74	86.23		86.25	0.001185	1.21	157.92	195.28	0.13
Federico Trib	0.773	Max WS	100yr mit	247.45	82.74	86.61		86.64	0.001489	1.48	240.85	239.39	0.15
Federico Trib	.7715*	Max WS	10yr mit	150.16	82.72	86.12		86.14	0.001627	1.38	178.80	247.51	0.15
Federico Trib	.7715*	Max WS	100yr mit	218.75	82.72	86.50		86.52	0.001274	1.34	284.74	286.22	0.14
Federico Trib	0.77	Max WS	10yr mit	147.26	82.70	86.03		86.03	0.000926	1.00	246.06	233.61	0.11
Federico Trib	0.77	Max WS	100yr mit	205.50	82.70	86.43		86.44	0.000717	0.97	353.67	285.29	0.10
Federico Trib	.768*	Max WS	10yr mit	188.90	82.52	85.89		85.91	0.002343	1.62	215.47	252.83	0.18
Federico Trib	.768*	Max WS	100yr mit	332.17	82.52	86.29		86.32	0.002388	1.81	323.22	284.20	0.19
Federico Trib	.766*	Max WS	10yr mit	189.57	82.34	85.67		85.70	0.003493	1.97	172.65	234.33	0.22
Federico Trib	.766*	Max WS	100yr mit	333.07	82.34	86.08		86.11	0.003216	2.09	277.68	274.09	0.22
Federico Trib	.764*	Max WS	10yr mit	190.25	82.15	85.39		85.43	0.003952	2.05	151.14	186.42	0.23
Federico Trib	.764*	Max WS	100yr mit	334.17	82.15	85.81		85.85	0.003838	2.25	237.57	231.06	0.24
Federico Trib	.762*	Max WS	10yr mit	190.95	81.97	85.10		85.14	0.003880	1.99	150.83	169.18	0.23
Federico Trib	.762*	Max WS	100yr mit	335.33	81.97	85.52		85.56	0.003986	2.25	226.99	199.16	0.24
Federico Trib	.76	Max WS	10yr mit	191.66	81.79	84.82		84.85	0.003685	1.90	156.26	165.13	0.22
Federico Trib	.76	Max WS	100yr mit	336.57	81.79	85.21		85.26	0.004184	2.25	224.11	186.09	0.24
Federico Trib	.759375*	Max WS	10yr mit	192.01	81.72	84.68		84.71	0.004089	1.90	154.75	170.17	0.23
Federico Trib	.759375*	Max WS	100yr mit	337.11	81.72	85.05		85.09	0.004592	2.25	225.99	208.19	0.25
Federico Trib	.75875*	Max WS	10yr mit	192.38	81.64	84.51		84.55	0.004610	1.90	156.01	204.04	0.24
Federico Trib	.75875*	Max WS	100yr mit	337.71	81.64	84.87		84.91	0.004956	2.21	234.23	233.42	0.26
Federico Trib	.758125*	Max WS	10yr mit	192.74	81.57	84.33		84.37	0.005198	1.88	161.65	225.93	0.25
Federico Trib	.758125*	Max WS	100yr mit	338.26	81.57	84.69		84.72	0.005165	2.12	246.29	250.59	0.26
Federico Trib	.7575*	Max WS	10yr mit	193.10	81.49	84.12		84.16	0.006081	1.94	161.02	223.04	0.27
Federico Trib	.7575*	Max WS	100yr mit	338.61	81.49	84.47		84.51	0.006371	2.17	246.47	267.95	0.28
Federico Trib	.756875*	Max WS	10yr mit	193.47	81.42	83.88		83.91	0.007292	2.01	160.89	233.12	0.29
Federico Trib	.756875*	Max WS	100yr mit	337.73	81.42	84.21		84.25	0.007625	2.25	246.46	292.62	0.31
Federico Trib	.75625*	Max WS	10yr mit	192.40	81.35	83.56		83.59	0.009434	2.13	165.46	300.72	0.33
Federico Trib	.75625*	Max WS	100yr mit	330.47	81.35	83.97		84.00	0.005356	1.82	307.60	372.93	0.26
Federico Trib	.755625*	Max WS	10yr mit	189.98	81.27	83.34		83.35	0.003120	1.14	245.32	346.25	0.19
Federico Trib	.755625*	Max WS	100yr mit	325.34	81.27	83.85		83.86	0.001817	1.06	425.22	365.26	0.15
Federico Trib	.755	Max WS	10yr mit	188.94	81.20	83.26		83.27	0.001462	0.70	311.56	343.23	0.11
Federico Trib	.755	Max WS	100yr mit	324.31	81.20	83.80		83.81	0.001093	0.76	497.45	360.83	0.11
Federico Trib	.754*	Max WS	10yr mit	197.28	80.96	83.19		83.19	0.000712	0.53	403.84	342.13	0.09
Federico Trib	.754*	Max WS	100yr mit	340.64	80.96	83.74		83.75	0.000685	0.63	613.40	397.80	0.09
Federico Trib	.753*	Max WS	10yr mit	196.82	80.72	83.15		83.15	0.000636	0.54	389.99	316.12	0.08
Federico Trib	.753*	Max WS	100yr mit	340.28	80.72	83.70		83.70	0.000644	0.61	590.90	390.87	0.09
Federico Trib	.752*	Max WS	10yr mit	196.23	80.48	83.11		83.11	0.000414	0.46	442.57	287.09	0.07
Federico Trib	.752*	Max WS	100yr mit	340.08	80.48	83.66		83.66	0.000501	0.57	614.58	342.70	0.08
Federico Trib	.751*	Max WS	10yr mit	196.37	80.24	83.09		83.09	0.000263	0.39	521.38	287.58	0.05
Federico Trib	.751*	Max WS	100yr mit	340.47	80.24	83.63		83.64	0.000347	0.50	700.07	365.47	0.06
Federico Trib	0.75	Max WS	10yr mit	196.30	80.00	83.06		83.07	0.000509	0.56	456.80	341.34	0.08
Federico Trib	0.75	Max WS	100yr mit	340.73	80.00	83.60		83.61	0.000542	0.65	657.49	409.64	0.08
Federico Trib	.7494*	Max WS	10yr mit	198.21	79.90	83.04		83.04	0.000315	0.43	547.62	408.26	0.06
Federico Trib	.7494*	Max WS	100yr mit	345.04	79.90	83.57		83.58	0.000347	0.50	773.11	432.93	0.06
Federico Trib	.7488*	Max WS	10yr mit	198.64	79.80	83.02		83.02	0.000198	0.33	569.10	299.47	0.04
Federico Trib	.7488*	Max WS	100yr mit	345.64	79.80	83.55		83.55	0.000274	0.42	738.84	347.49	0.05
Federico Trib	.7482*	Max WS	10yr mit	198.99	79.70	83.01		83.01	0.000083	0.21	717.60	325.46	0.03
Federico Trib	.7482*	Max WS	100yr mit	346.54	79.70	83.54		83.54	0.000134	0.28	895.86	350.09	0.03

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	.7476*	Max WS	10yr mit	199.52	79.60	83.01		83.01	0.000033	0.13	908.69	331.45	0.02
Federico Trib	.7476*	Max WS	100yr mit	347.64	79.60	83.53		83.53	0.000061	0.18	1094.05	381.50	0.02
Federico Trib	.747	Max WS	10yr mit	199.92	79.50	82.97		82.98	0.000872	0.77	382.71	359.56	0.10
Federico Trib	.747	Max WS	100yr mit	348.36	79.50	83.50		83.51	0.000759	0.78	583.70	401.98	0.09
Federico Trib	.744	Max WS	10yr mit	200.19	79.00	82.84		82.86	0.001530	1.53	219.68	205.44	0.15
Federico Trib	.744	Max WS	100yr mit	349.23	79.00	83.37		83.39	0.001568	1.71	332.70	222.92	0.16
Federico Trib	0.74	Max WS	10yr mit	201.02	77.38	82.09		82.12	0.001338	1.43	238.21	257.05	0.14
Federico Trib	0.74	Max WS	100yr mit	352.89	77.38	82.64		82.66	0.001213	1.51	382.83	269.12	0.14
Federico Trib	0.73	Max WS	10yr mit	204.50	76.00	81.58		81.61	0.001684	1.71	207.02	209.90	0.16
Federico Trib	0.73	Max WS	100yr mit	359.59	76.00	82.14		82.17	0.001708	1.90	333.35	247.57	0.16
Federico Trib	0.72	Max WS	10yr mit	202.26	76.00	81.04		81.06	0.001598	1.48	210.79	255.65	0.15
Federico Trib	0.72	Max WS	100yr mit	356.91	76.00	81.64		81.66	0.001334	1.53	383.36	319.66	0.14
Federico Trib	0.71	Max WS	10yr mit	206.11	75.00	80.55		80.57	0.001079	1.32	248.17	236.52	0.13
Federico Trib	0.71	Max WS	100yr mit	365.13	75.00	81.22		81.24	0.000938	1.39	424.34	281.14	0.12
Federico Trib	0.70	Max WS	10yr mit	208.83	75.00	80.35		80.38	0.001428	1.49	200.91	159.58	0.15
Federico Trib	0.70	Max WS	100yr mit	369.14	75.00	81.03		81.06	0.001484	1.72	316.64	199.85	0.15
Federico Trib	.666666*	Max WS	10yr mit	209.94	75.00	80.20		80.24	0.001816	1.67	176.70	134.97	0.16
Federico Trib	.666666*	Max WS	100yr mit	368.81	75.00	80.87		80.91	0.001995	1.98	285.59	242.59	0.18
Federico Trib	.633333*	Max WS	10yr mit	209.42	75.00	80.02		80.06	0.002199	1.81	169.98	171.80	0.18
Federico Trib	.633333*	Max WS	100yr mit	363.98	75.00	80.71		80.74	0.001692	1.81	334.09	274.92	0.16
Federico Trib	0.6	Max WS	10yr mit	208.53	75.00	79.83		79.86	0.002268	1.79	195.52	214.75	0.18
Federico Trib	0.6	Max WS	100yr mit	358.22	75.00	80.59		80.61	0.001178	1.49	407.16	297.65	0.13
Federico Trib	.55*	Max WS	10yr mit	218.84	74.50	79.72		79.73	0.000759	1.14	246.73	171.60	0.11
Federico Trib	.55*	Max WS	100yr mit	368.73	74.50	80.52		80.53	0.000667	1.23	434.39	251.55	0.11
Federico Trib	0.5	Max WS	10yr mit	218.67	74.00	79.69		79.69	0.000212	0.64	425.95	213.23	0.06
Federico Trib	0.5	Max WS	100yr mit	366.59	74.00	80.49		80.50	0.000239	0.77	631.28	276.84	0.06
Federico Trib	.45*	Max WS	10yr mit	218.52	73.50	79.66		79.67	0.000543	0.98	282.90	184.95	0.09
Federico Trib	.45*	Max WS	100yr mit	363.29	73.50	80.46		80.48	0.000509	1.09	438.66	198.55	0.09
Federico Trib	0.4	Max WS	10yr mit	217.63	73.00	79.56		79.60	0.002352	1.75	150.38	109.98	0.18
Federico Trib	0.4	Max WS	100yr mit	346.92	73.00	80.37		80.41	0.001756	1.78	241.17	112.07	0.16
Main Reach	1.44	Max WS	10yr mit	145.43	112.07	114.17		114.18	0.002632	0.84	178.10	277.05	0.16
Main Reach	1.44	Max WS	100yr mit	284.71	112.07	114.55		114.57	0.002511	1.05	287.58	294.14	0.17
Main Reach	1.43666*	Max WS	10yr mit	145.37	111.85	113.92		113.93	0.002368	0.86	185.44	247.08	0.16
Main Reach	1.43666*	Max WS	100yr mit	284.25	111.85	114.30		114.31	0.002583	1.13	286.56	291.96	0.17
Main Reach	1.43333*	Max WS	10yr mit	145.24	111.62	113.72		113.73	0.001843	0.84	191.29	260.13	0.14
Main Reach	1.43333*	Max WS	100yr mit	283.25	111.62	114.04		114.06	0.002541	1.18	310.14	408.74	0.17
Main Reach	1.43	Max WS	10yr mit	145.18	111.40	113.24		113.27	0.007844	1.55	141.30	355.03	0.28
Main Reach	1.43	Max WS	100yr mit	282.66	111.40	113.46		113.49	0.009004	1.91	220.64	367.15	0.31
Main Reach	1.425*	Max WS	10yr mit	145.13	110.50	112.08		112.11	0.012264	1.90	107.16	203.51	0.35
Main Reach	1.425*	Max WS	100yr mit	272.04	110.50	112.50		112.52	0.006614	1.80	210.32	280.70	0.27
Main Reach	1.42	Max WS	10yr mit	142.92	109.60	111.30		111.31	0.001447	0.79	240.29	263.67	0.13
Main Reach	1.42	Max WS	100yr mit	245.69	109.60	112.11		112.12	0.000544	0.68	479.74	317.61	0.08
Main Reach	1.41	Max WS	10yr mit	154.41	108.40	111.13		111.13	0.000395	0.47	322.95	163.89	0.07
Main Reach	1.41	Max WS	100yr mit	254.10	108.40	112.03		112.04	0.000296	0.56	491.15	209.77	0.06
Main Reach	1.40	Max WS	10yr mit	154.02	108.40	110.99		110.99	0.000549	0.64	289.92	162.10	0.08
Main Reach	1.40	Max WS	100yr mit	253.64	108.40	111.94		111.94	0.000355	0.68	461.19	202.37	0.07
Main Reach	1.39	Max WS	10yr mit	153.37	107.50	110.87		110.88	0.000265	0.47	358.10	158.64	0.06
Main Reach	1.39	Max WS	100yr mit	255.41	107.50	111.86		111.86	0.000226	0.52	530.87	240.39	0.06
Main Reach	1.388	Max WS	10yr mit	154.27	107.00	110.67		110.68	0.001271	1.04	253.64	409.21	0.13
Main Reach	1.388	Max WS	100yr mit	259.00	107.00	111.81		111.81	0.000156	0.49	756.48	444.40	0.05
Main Reach	1.387	Max WS	10yr mit	154.24	107.00	110.65		110.65	0.000265	0.58	440.26	500.80	0.06
Main Reach	1.387	Max WS	100yr mit	258.98	107.00	111.81		111.81	0.000066	0.37	1019.28	500.80	0.03
Main Reach	1.385				Culvert								
Main Reach	1.384	Max WS	10yr mit	154.21	107.00	109.69		109.70	0.001314	0.95	285.09	435.26	0.13
Main Reach	1.384	Max WS	100yr mit	258.97	107.00	109.96		109.97	0.001310	1.04	407.52	474.32	0.13
Main Reach	1.38	Max WS	10yr mit	154.17	107.00	109.56		109.57	0.004345	1.56	212.54	384.71	0.22
Main Reach	1.38	Max WS	100yr mit	258.88	107.00	109.86		109.87	0.003202	1.51	329.44	409.29	0.20
Main Reach	1.37	Max WS	10yr mit	167.74	106.00	108.69		108.69	0.001264	0.90	245.35	188.96	0.12

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	1.37	Max WS	100yr mit	269.38	106.00	109.16		109.17	0.001210	1.04	338.32	204.17	0.13
Main Reach	1.36	Max WS	10yr mit	181.55	105.80	108.28		108.29	0.000678	0.78	336.79	220.70	0.10
Main Reach	1.36	Max WS	100yr mit	328.66	105.80	108.68		108.69	0.001092	1.11	427.92	233.94	0.12
Main Reach	1.35	Max WS	10yr mit	183.48	104.80	108.10		108.11	0.000757	0.82	291.40	271.43	0.10
Main Reach	1.35	Max WS	100yr mit	330.30	104.80	108.36		108.38	0.001420	1.22	366.63	304.30	0.14
Main Reach	1.34	Max WS	10yr mit	185.66	104.46	107.03		107.07	0.006148	2.16	150.48	272.46	0.28
Main Reach	1.34	Max WS	100yr mit	333.68	104.46	107.43		107.46	0.004442	2.09	266.02	302.94	0.25
Main Reach	1.335	Max WS	10yr mit	192.57	102.00	105.78		105.79	0.001422	1.25	208.49	145.72	0.13
Main Reach	1.335	Max WS	100yr mit	358.61	102.00	106.39		106.41	0.001659	1.55	347.29	281.33	0.15
Main Reach	1.333			Mult Open									
Main Reach	1.33	Max WS	10yr mit	192.57	102.00	105.49		105.51	0.003813	1.78	193.25	259.41	0.22
Main Reach	1.33	Max WS	100yr mit	358.51	102.00	105.97		106.00	0.003490	1.95	328.59	314.17	0.22
Main Reach	1.32	Max WS	10yr mit	152.17	101.80	104.98		104.98	0.000482	0.71	357.65	287.24	0.08
Main Reach	1.32	Max WS	100yr mit	357.89	101.80	105.30		105.31	0.001263	1.26	483.87	409.13	0.13
Main Reach	1.31	Max WS	10yr mit	204.99	100.90	104.89		104.89	0.000141	0.44	602.49	267.22	0.05
Main Reach	1.31	Max WS	100yr mit	415.09	100.90	105.06		105.06	0.000470	0.84	648.68	306.04	0.08
Main Reach	1.30	Max WS	10yr mit	208.92	100.63	103.80		103.82	0.004594	1.81	171.70	194.92	0.24
Main Reach	1.30	Max WS	100yr mit	331.32	100.63	104.75		104.76	0.001146	1.18	385.18	266.73	0.13
Main Reach	1.29	Max WS	10yr mit	214.88	100.40	103.26		103.27	0.000379	0.60	515.87	322.16	0.07
Main Reach	1.29	Max WS	100yr mit	341.19	100.40	104.62		104.62	0.000125	0.48	1011.21	402.80	0.04
Main Reach	1.28	Max WS	10yr mit	217.56	100.00	103.16		103.16	0.000351	0.63	429.74	263.83	0.07
Main Reach	1.28	Max WS	100yr mit	346.52	100.00	104.58		104.58	0.000134	0.53	887.26	379.50	0.05
Main Reach	1.27	Max WS	10yr mit	218.83	98.88	103.09		103.09	0.000165	0.48	576.11	293.72	0.05
Main Reach	1.27	Max WS	100yr mit	348.95	98.88	104.55		104.55	0.000072	0.41	1050.26	352.36	0.03
Main Reach	1.268	Max WS	10yr mit	219.55	98.63	103.06		103.06	0.000304	0.67	488.33	422.32	0.07
Main Reach	1.268	Max WS	100yr mit	350.11	98.63	104.54		104.54	0.000078	0.44	1144.87	452.33	0.04
Main Reach	1.267	Max WS	10yr mit	219.53	98.60	103.05		103.05	0.000305	0.61	494.14	393.00	0.07
Main Reach	1.267	Max WS	100yr mit	350.09	98.60	104.54		104.54	0.000086	0.44	1104.21	430.85	0.04
Main Reach	1.265			Culvert									
Main Reach	1.26	Max WS	10yr mit	219.04	98.56	102.20		102.24	0.003709	1.69	181.27	302.04	0.22
Main Reach	1.26	Max WS	100yr mit	347.63	98.56	103.11		103.12	0.000874	1.07	461.49	318.99	0.11
Main Reach	1.259	Max WS	10yr mit	218.68	98.50	102.11		102.13	0.001621	1.41	310.56	365.83	0.15
Main Reach	1.259	Max WS	100yr mit	347.65	98.50	103.09		103.09	0.000397	0.86	667.28	367.87	0.08
Main Reach	1.255	Max WS	10yr mit	229.72	97.97	101.78		101.78	0.000448	0.76	456.10	360.10	0.08
Main Reach	1.255	Max WS	100yr mit	366.05	97.97	103.00		103.00	0.000143	0.55	952.57	424.90	0.05
Main Reach	1.22	Max WS	10yr mit	229.67	97.50	101.71		101.71	0.000412	0.73	404.69	272.68	0.08
Main Reach	1.22	Max WS	100yr mit	366.04	97.50	102.97		102.98	0.000166	0.60	815.65	378.67	0.05
Main Reach	1.215			Culvert									
Main Reach	1.21	Max WS	10yr mit	229.66	96.51	99.86		100.05	0.006808	3.53	65.10	43.58	0.51
Main Reach	1.21	Max WS	100yr mit	366.00	96.51	100.18		100.50	0.010878	4.54	80.55	52.53	0.65
Main Reach	1.20	Max WS	10yr mit	229.38	96.05	99.48		99.62	0.006439	3.46	106.85	173.68	0.49
Main Reach	1.20	Max WS	100yr mit	364.57	96.05	99.81		99.94	0.005604	3.66	166.98	198.79	0.47
Main Reach	1.19	Max WS	10yr mit	252.75	95.07	98.45		98.49	0.001273	1.73	194.62	247.51	0.23
Main Reach	1.19	Max WS	100yr mit	406.58	95.07	98.94		98.98	0.001027	1.83	331.38	308.39	0.21
Main Reach	1.18	Max WS	10yr mit	251.53	94.10	97.80		97.90	0.002591	3.24	139.51	123.16	0.34
Main Reach	1.18	Max WS	100yr mit	404.69	94.10	98.35		98.45	0.002484	3.57	215.66	158.94	0.35
Main Reach	1.17	Max WS	10yr mit	283.27	93.80	97.15		97.20	0.000980	1.87	189.83	148.84	0.21
Main Reach	1.17	Max WS	100yr mit	487.11	93.80	97.59		97.68	0.001434	2.53	262.87	184.41	0.26
Main Reach	1.16	Max WS	10yr mit	282.57	93.25	96.59		96.66	0.002160	2.62	205.77	297.23	0.31
Main Reach	1.16	Max WS	100yr mit	482.62	93.25	97.00		97.06	0.002028	2.83	335.62	344.73	0.30
Main Reach	1.15	Max WS	10yr mit	282.12	92.65	96.15		96.17	0.000455	1.40	414.21	428.52	0.15
Main Reach	1.15	Max WS	100yr mit	481.95	92.65	96.57		96.59	0.000500	1.61	595.77	440.95	0.16
Main Reach	1.14	Max WS	10yr mit	282.07	92.20	94.85		94.93	0.003960	2.97	164.11	192.77	0.40
Main Reach	1.14	Max WS	100yr mit	481.77	92.20	95.25		95.34	0.003842	3.36	245.63	216.10	0.41
Main Reach	1.13	Max WS	10yr mit	281.33	91.95	94.06		94.07	0.000488	0.77	365.71	501.11	0.13
Main Reach	1.13	Max WS	100yr mit	475.65	91.95	94.50		94.52	0.000448	0.92	522.47	546.13	0.13
Main Reach	1.12	Max WS	10yr mit	280.70	90.70	93.48		93.57	0.003317	3.13	163.16	180.12	0.38

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	1.12	Max WS	100yr mit	470.52	90.70	94.00		94.08	0.002817	3.34	270.45	235.51	0.36
Main Reach	1.11	Max WS	10yr mit	280.04	90.02	92.99		93.00	0.000368	1.00	400.19	319.53	0.13
Main Reach	1.11	Max WS	100yr mit	468.64	90.02	93.59		93.60	0.000326	1.12	598.66	343.92	0.12
Main Reach	1.10	Max WS	10yr mit	279.37	88.90	92.37		92.50	0.003293	2.90	101.77	84.01	0.37
Main Reach	1.10	Max WS	100yr mit	466.87	88.90	92.92		93.10	0.003514	3.54	160.67	133.13	0.40
Main Reach	1.09	Max WS	10yr mit	278.59	87.51	91.81		91.82	0.000212	1.04	462.57	320.92	0.10
Main Reach	1.09	Max WS	100yr mit	465.62	87.51	92.33		92.34	0.000260	1.27	638.53	352.20	0.12
Main Reach	1.08	Max WS	10yr mit	281.23	87.08	91.55		91.61	0.001304	2.33	218.28	216.73	0.24
Main Reach	1.08	Max WS	100yr mit	470.69	87.08	92.04		92.10	0.001392	2.68	339.93	275.89	0.26
Main Reach	1.07	Max WS	10yr mit	280.61	87.00	90.53		90.71	0.004845	3.46	95.50	112.63	0.45
Main Reach	1.07	Max WS	100yr mit	418.63	87.00	91.22		91.34	0.002651	3.06	207.94	213.75	0.34
Main Reach	1.06	Max WS	10yr mit	271.62	86.38	89.68		89.73	0.000965	1.65	187.91	163.53	0.20
Main Reach	1.06	Max WS	100yr mit	409.88	86.38	90.80		90.81	0.000298	1.20	510.33	355.56	0.12
Main Reach	1.05	Max WS	10yr mit	270.26	85.43	89.50		89.53	0.000565	1.37	259.29	205.61	0.16
Main Reach	1.05	Max WS	100yr mit	410.68	85.43	90.74		90.75	0.000195	1.07	592.60	332.22	0.10
Main Reach	1.04	Max WS	10yr mit	271.84	84.89	89.44		89.45	0.000389	1.05	410.82	1271.70	0.13
Main Reach	1.04	Max WS	100yr mit	413.62	84.89	90.72		90.73	0.000042	0.46	2118.24	1987.18	0.05
Main Reach	1.03	Max WS	10yr mit	271.80	84.80	89.37		89.40	0.000971	1.49	221.38	1347.04	0.20
Main Reach	1.03	Max WS	100yr mit	413.61	84.80	90.72		90.72	0.000078	0.60	1537.96	1770.00	0.06
Main Reach	1.025			Culvert									
Main Reach	1.02	Max WS	10yr mit	270.28	84.82	88.39		88.41	0.001962	1.36	372.90	769.25	0.16
Main Reach	1.02	Max WS	100yr mit	412.42	84.82	88.96		88.97	0.000586	0.84	814.63	772.04	0.09
Main Reach	1.01	Max WS	10yr mit	269.29	84.30	87.96		87.97	0.000527	0.90	409.45	165.43	0.09
Main Reach	1.01	Max WS	100yr mit	412.37	84.30	88.77		88.78	0.000475	0.99	576.83	210.72	0.09
Main Reach	1.00	Max WS	10yr mit	281.20	83.80	87.89		87.90	0.000609	0.84	384.55	174.31	0.09
Main Reach	1.00	Max WS	100yr mit	432.97	83.80	88.70		88.71	0.000565	0.91	529.09	178.93	0.09
Main Reach	0.95	Max WS	10yr mit	281.69	83.20	87.77		87.78	0.000629	1.04	360.41	189.14	0.10
Main Reach	0.95	Max WS	100yr mit	433.97	83.20	88.59		88.60	0.000544	1.12	517.64	191.46	0.10
Main Reach	0.90	Max WS	10yr mit	279.86	82.09	87.08		87.16	0.004151	2.27	123.65	59.84	0.24
Main Reach	0.90	Max WS	100yr mit	433.93	82.09	88.20		88.26	0.002226	2.10	270.85	180.30	0.19
Main Reach	0.88	Max WS	10yr mit	279.06	82.00	86.77		86.89	0.006954	2.88	112.35	90.28	0.31
Main Reach	0.88	Max WS	100yr mit	433.93	82.00	88.12		88.16	0.001614	1.83	349.70	209.66	0.16
Main Reach	.87			Culvert									
Main Reach	0.86	Max WS	10yr mit	275.87	81.83	86.21		86.32	0.007816	2.77	114.86	74.86	0.33
Main Reach	0.86	Max WS	100yr mit	433.92	81.83	86.79		86.92	0.008197	3.14	165.37	98.61	0.34
Main Reach	0.85	Max WS	10yr mit	274.28	81.10	85.99		86.02	0.001155	1.50	263.60	126.95	0.14
Main Reach	0.85	Max WS	100yr mit	433.92	81.10	86.57		86.60	0.001446	1.84	338.97	134.59	0.15
Main Reach	0.80	Max WS	10yr mit	278.07	81.10	85.92		85.92	0.000239	0.65	548.65	234.13	0.06
Main Reach	0.80	Max WS	100yr mit	444.50	81.10	86.47		86.48	0.000341	0.85	679.47	236.20	0.07
Main Reach	0.75	Max WS	10yr mit	281.18	80.50	85.85		85.86	0.000401	0.76	426.77	183.75	0.08
Main Reach	0.75	Max WS	100yr mit	451.60	80.50	86.37		86.38	0.000600	1.03	531.45	212.90	0.10
Main Reach	0.70	Max WS	10yr mit	280.80	80.40	85.79		85.80	0.000191	0.58	581.55	182.02	0.05
Main Reach	0.70	Max WS	100yr mit	451.53	80.40	86.28		86.29	0.000314	0.81	672.67	189.64	0.07
Main Reach	0.65	Max WS	10yr mit	280.98	80.20	85.77		85.77	0.000080	0.40	869.85	285.60	0.04
Main Reach	0.65	Max WS	100yr mit	452.23	80.20	86.24		86.25	0.000134	0.56	1007.87	295.85	0.05
Main Reach	0.6	Max WS	10yr mit	281.58	80.20	85.75		85.75	0.000070	0.38	902.81	294.79	0.03
Main Reach	0.6	Max WS	100yr mit	453.55	80.20	86.21		86.22	0.000121	0.54	1043.50	309.99	0.05
Main Reach	0.555	Max WS	10yr mit	281.89	80.20	85.73			0.000121	0.52	685.05	263.99	0.04
Main Reach	0.555	Max WS	100yr mit	454.20	80.20	86.18			0.000206	0.73	808.51	283.48	0.06
Main Reach	0.552			Mult Open									
Main Reach	0.55	Max WS	10yr mit	278.13	79.60	83.80		83.81	0.000681	0.81	379.70	153.52	0.09
Main Reach	0.55	Max WS	100yr mit	452.97	79.60	84.75		84.76	0.000615	0.95	558.75	237.50	0.09
Main Reach	0.5	Max WS	10yr mit	276.45	79.60	83.60		83.61	0.000830	0.90	354.61	151.22	0.10
Main Reach	0.5	Max WS	100yr mit	452.60	79.60	84.58		84.59	0.000690	1.00	547.32	241.61	0.10
Main Reach	0.45	Max WS	10yr mit	276.01	79.60	83.50		83.50	0.000181	0.49	724.02	291.19	0.05
Main Reach	0.45	Max WS	100yr mit	453.06	79.60	84.49		84.50	0.000164	0.57	1020.16	308.42	0.05
Main Reach	0.40	Max WS	10yr mit	277.05	79.60	83.47		83.47	0.000091	0.33	904.28	318.48	0.04

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	0.40	Max WS	100yr mit	455.01	79.60	84.47		84.47	0.000091	0.41	1260.02	372.00	0.04
Main Reach	0.35	Max WS	10yr mit	277.04	79.23	83.34		83.37	0.001260	1.40	244.39	210.19	0.14
Main Reach	0.35	Max WS	100yr mit	454.99	79.23	84.39		84.41	0.000755	1.32	483.00	254.53	0.11
Main Reach	0.33			Mult Open									
Main Reach	0.30	Max WS	10yr mit	276.84	79.23	82.20		82.28	0.006982	2.28	121.23	67.44	0.30
Main Reach	0.30	Max WS	100yr mit	454.95	79.23	82.87		82.98	0.007822	2.64	172.10	83.72	0.32
Main Reach	0.25	Max WS	10yr mit	276.03	79.20	81.65		81.66	0.000809	0.78	430.29	238.93	0.10
Main Reach	0.25	Max WS	100yr mit	454.84	79.20	82.28		82.29	0.000854	0.94	583.80	253.00	0.11
Main Reach	0.20	Max WS	10yr mit	276.66	79.00	81.56		81.57	0.000768	0.68	470.38	284.42	0.10
Main Reach	0.20	Max WS	100yr mit	456.94	79.00	82.18		82.19	0.000747	0.82	652.05	299.34	0.10
Main Reach	0.15	Max WS	10yr mit	276.75	78.70	81.34		81.35	0.001409	1.01	357.38	234.64	0.13
Main Reach	0.15	Max WS	100yr mit	459.85	78.70	81.97		81.98	0.001300	1.14	512.36	252.92	0.13
Main Reach	0.10	Max WS	10yr mit	276.25	78.70	81.19		81.20	0.001306	0.95	380.31	254.10	0.13
Main Reach	0.10	Max WS	100yr mit	460.39	78.70	81.84		81.85	0.001140	1.10	551.03	270.19	0.13
Main Reach	0.05	Max WS	10yr mit	275.30	78.20	81.02		81.02	0.000250	0.49	677.10	303.96	0.06
Main Reach	0.05	Max WS	100yr mit	460.52	78.20	81.68		81.69	0.000307	0.61	884.53	321.23	0.07
Main Reach	0.04	Max WS	10yr mit	273.98	77.40	80.80		80.81	0.001550	1.10	362.25	261.96	0.14
Main Reach	0.04	Max WS	100yr mit	460.69	77.40	81.50		81.51	0.001173	1.18	553.01	279.55	0.13
Main Reach	0.03	Max WS	10yr mit	273.12	76.70	80.62		80.62	0.000203	0.55	691.47	284.64	0.06
Main Reach	0.03	Max WS	100yr mit	460.84	76.70	81.36		81.36	0.000245	0.70	903.10	289.44	0.06
Main Reach	0.02	Max WS	10yr mit	273.16	76.20	80.58		80.58	0.000171	0.48	688.99	327.85	0.05
Main Reach	0.02	Max WS	100yr mit	461.01	76.20	81.31		81.31	0.000207	0.61	964.17	415.90	0.06
Main Reach	0.01	Max WS	10yr mit	274.57	75.70	80.51		80.52	0.000439	0.84	440.03	292.29	0.08
Main Reach	0.01	Max WS	100yr mit	464.62	75.70	81.23		81.24	0.000454	0.97	668.12	324.20	0.09
Main Reach	0.009	Max WS	10yr mit	274.60	75.08	80.40		80.42	0.001309	1.49	281.15	223.00	0.14
Main Reach	0.009	Max WS	100yr mit	464.68	75.08	81.13		81.15	0.001061	1.53	445.39	223.00	0.13
Main Reach	0.008	Max WS	10yr mit	274.22	73.85	79.87		79.94	0.002628	2.18	130.54	48.35	0.20
Main Reach	0.008	Max WS	100yr mit	462.58	73.85	80.74		80.79	0.002010	2.12	379.69	402.67	0.18
Main Reach	0.007	Max WS	10yr mit	208.69	73.00	79.56		79.58	0.000781	1.27	196.50	87.02	0.11
Main Reach	0.007	Max WS	100yr mit	393.21	73.00	80.37		80.41	0.001132	1.68	320.67	240.74	0.14
Down	6.9	Max WS	10yr mit	426.32	72.00	79.56		79.60	0.000892	1.70	274.76	175.85	0.16
Down	6.9	Max WS	100yr mit	740.13	72.00	80.37		80.42	0.000904	1.94	609.07	646.42	0.16
Down	6.85*	Max WS	10yr mit	426.31	71.71	79.48		79.53	0.000802	1.80	286.23	183.52	0.15
Down	6.85*	Max WS	100yr mit	740.11	71.71	80.28		80.34	0.000988	2.20	489.65	344.82	0.17
Down	6.8	Max WS	10yr mit	426.31	71.43	79.38	75.36	79.45	0.001046	2.18	254.28	146.62	0.17
Down	6.8	Max WS	100yr mit	740.11	71.43	80.15	76.42	80.24	0.001412	2.73	392.96	251.35	0.20

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	1.4	Max WS	10yr FFDUNM	82.79	109.90	111.05		111.06	0.003258	0.82	101.36	150.87	0.17
Federico Trib	1.4	Max WS	Dev Unmit100	153.64	109.90	111.34		111.35	0.003399	1.06	146.34	158.65	0.19
Federico Trib	1.375*	Max WS	10yr FFDUNM	82.63	109.66	110.73		110.74	0.003356	0.80	103.07	160.26	0.18
Federico Trib	1.375*	Max WS	Dev Unmit100	152.85	109.66	111.01		111.03	0.003453	1.03	149.79	172.69	0.19
Federico Trib	1.35*	Max WS	10yr FFDUNM	82.37	109.43	110.41		110.42	0.003491	0.80	103.92	177.93	0.18
Federico Trib	1.35*	Max WS	Dev Unmit100	152.36	109.43	110.68		110.70	0.003555	1.00	156.42	200.89	0.19
Federico Trib	1.325*	Max WS	10yr FFDUNM	82.14	109.19	110.09		110.10	0.003262	0.77	110.81	196.82	0.17
Federico Trib	1.325*	Max WS	Dev Unmit100	151.87	109.19	110.35		110.37	0.003434	0.95	167.43	230.22	0.19
Federico Trib	1.3	Max WS	10yr FFDUNM	81.95	108.95	109.72		109.73	0.004556	0.86	100.19	207.47	0.20
Federico Trib	1.3	Max WS	Dev Unmit100	151.51	108.95	109.98		109.99	0.004526	1.01	158.46	247.62	0.21
Federico Trib	1.275*	Max WS	10yr FFDUNM	86.41	108.21	109.14		109.15	0.004458	0.90	99.17	187.76	0.20
Federico Trib	1.275*	Max WS	Dev Unmit100	159.35	108.21	109.42		109.44	0.004106	1.07	158.27	225.63	0.20
Federico Trib	1.25*	Max WS	10yr FFDUNM	90.69	107.47	108.58		108.60	0.004198	0.97	93.98	140.52	0.20
Federico Trib	1.25*	Max WS	Dev Unmit100	166.79	107.47	108.93		108.95	0.003488	1.14	156.95	198.15	0.20
Federico Trib	1.225*	Max WS	10yr FFDUNM	94.95	106.74	108.15		108.16	0.002602	0.94	105.85	139.00	0.17
Federico Trib	1.225*	Max WS	Dev Unmit100	174.38	106.74	108.54		108.56	0.002493	1.15	170.45	186.36	0.17
Federico Trib	1.2	Max WS	10yr FFDUNM	99.23	106.00	107.90		107.91	0.001254	0.85	135.67	145.60	0.12
Federico Trib	1.2	Max WS	Dev Unmit100	182.01	106.00	108.27		108.29	0.001627	1.13	203.77	207.06	0.15
Federico Trib	1.18*	Max WS	10yr FFDUNM	112.27	105.50	107.65		107.67	0.002792	1.23	110.54	142.14	0.18
Federico Trib	1.18*	Max WS	Dev Unmit100	206.60	105.50	107.93		107.97	0.004031	1.67	153.03	153.67	0.23
Federico Trib	1.161	Max WS	10yr FFDUNM	114.50	105.00	107.50		107.50	0.000737	0.62	199.43	164.69	0.09
Federico Trib	1.161	Max WS	Dev Unmit100	210.56	105.00	107.70		107.71	0.001508	0.97	233.32	167.45	0.14
Federico Trib	1.16	Max WS	10yr FFDUNM	114.83	105.00	107.49			0.000755	0.62	198.28	164.59	0.09
Federico Trib	1.16	Max WS	Dev Unmit100	211.21	105.00	107.68			0.001571	0.98	230.78	167.24	0.14
Federico Trib	1.15			Mult Open									
Federico Trib	1.14	Max WS	10yr FFDUNM	114.83	105.00	107.06		107.06	0.001225	0.92	200.46	303.69	0.12
Federico Trib	1.14	Max WS	Dev Unmit100	210.92	105.00	107.35		107.36	0.001405	1.10	292.08	317.44	0.14
Federico Trib	1.139	Max WS	10yr FFDUNM	115.14	104.80	107.05		107.05	0.001256	0.94	198.27	303.23	0.13
Federico Trib	1.139	Max WS	Dev Unmit100	211.40	104.80	107.34		107.35	0.001459	1.12	288.23	316.75	0.14
Federico Trib	1.1	Max WS	10yr FFDUNM	118.51	104.00	106.54		106.56	0.004277	1.52	129.40	242.93	0.22
Federico Trib	1.1	Max WS	Dev Unmit100	184.98	104.00	106.96		106.97	0.001912	1.21	242.65	302.98	0.16
Federico Trib	1.0	Max WS	10yr FFDUNM	105.43	102.98	105.48		105.49	0.000594	0.72	193.79	184.04	0.09
Federico Trib	1.0	Max WS	Dev Unmit100	168.18	102.98	106.52		106.52	0.000196	0.54	386.02	185.75	0.05
Federico Trib	.999*	Max WS	10yr FFDUNM	105.35	102.79	105.46		105.47	0.000390	0.59	229.00	185.63	0.07
Federico Trib	.999*	Max WS	Dev Unmit100	168.33	102.79	106.51		106.51	0.000149	0.48	453.84	239.91	0.05
Federico Trib	.998*	Max WS	10yr FFDUNM	105.33	102.59	105.45		105.45	0.000362	0.56	231.31	177.17	0.07
Federico Trib	.998*	Max WS	Dev Unmit100	168.25	102.59	106.50		106.51	0.000151	0.48	446.24	239.86	0.05
Federico Trib	.9971	Max WS	10yr FFDUNM	105.32	102.40	105.44		105.44	0.000368	0.57	209.89	127.05	0.07
Federico Trib	.9971	Max WS	Dev Unmit100	168.22	102.40	106.50		106.50	0.000187	0.54	344.70	127.05	0.05
Federico Trib	0.997	Max WS	10yr FFDUNM	105.32	102.40	105.43		105.44	0.000372	0.57	208.89	126.65	0.07
Federico Trib	0.997	Max WS	Dev Unmit100	168.21	102.40	106.50		106.50	0.000188	0.54	343.51	126.65	0.05
Federico Trib	.9954			Culvert									
Federico Trib	0.993	Max WS	10yr FFDUNM	104.71	102.20	104.18		104.21	0.004612	1.26	87.29	118.64	0.22
Federico Trib	0.993	Max WS	Dev Unmit100	165.40	102.20	104.74		104.76	0.002143	1.16	158.60	137.72	0.16
Federico Trib	.9929	Max WS	10yr FFDUNM	104.56	102.20	104.13		104.16	0.005647	1.35	80.87	116.77	0.24
Federico Trib	.9929	Max WS	Dev Unmit100	165.30	102.20	104.72		104.74	0.002265	1.18	155.49	136.95	0.17
Federico Trib	0.98	Max WS	10yr FFDUNM	104.70	101.00	103.45		103.46	0.000748	0.86	146.42	97.68	0.10
Federico Trib	0.98	Max WS	Dev Unmit100	164.75	101.00	104.44		104.45	0.000386	0.79	248.07	105.54	0.08
Federico Trib	.979*	Max WS	10yr FFDUNM	105.45	100.80	103.42		103.43	0.000860	0.88	131.68	68.83	0.10
Federico Trib	.979*	Max WS	Dev Unmit100	166.42	100.80	104.42		104.43	0.000553	0.90	200.75	68.83	0.09
Federico Trib	.978*	Max WS	10yr FFDUNM	105.03	100.60	103.39		103.40	0.000724	0.79	145.46	75.58	0.09
Federico Trib	.978*	Max WS	Dev Unmit100	166.41	100.60	104.40		104.41	0.000463	0.79	222.14	75.58	0.07
Federico Trib	.9771	Max WS	10yr FFDUNM	104.96	100.40	103.38		103.38	0.000451	0.61	178.69	87.05	0.07
Federico Trib	.9771	Max WS	Dev Unmit100	166.41	100.40	104.40		104.40	0.000303	0.62	267.51	87.05	0.06
Federico Trib	0.977	Max WS	10yr FFDUNM	104.93	100.40	103.37		103.38	0.000469	0.62	174.25	84.34	0.07
Federico Trib	0.977	Max WS	Dev Unmit100	166.41	100.40	104.39		104.40	0.000318	0.63	260.42	84.34	0.06
Federico Trib	0.975			Culvert									
Federico Trib	0.973	Max WS	10yr FFDUNM	103.28	100.00	101.94		101.95	0.001359	0.94	146.13	166.58	0.13
Federico Trib	0.973	Max WS	Dev Unmit100	166.12	100.00	102.31		102.32	0.001260	1.04	213.03	190.36	0.13

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	.9729	Max WS	10yr FFDUNM	103.19	100.00	101.92		101.93	0.001417	0.96	143.79	165.51	0.13
Federico Trib	.9729	Max WS	Dev Unmit100	166.12	100.00	102.30		102.31	0.001302	1.06	210.54	189.55	0.13
Federico Trib	0.97	Max WS	10yr FFDUNM	103.04	100.00	101.90		101.91	0.001014	0.76	147.49	128.94	0.11
Federico Trib	0.97	Max WS	Dev Unmit100	166.08	100.00	102.27		102.29	0.001124	0.93	198.84	148.12	0.12
Federico Trib	.9675*	Max WS	10yr FFDUNM	103.57	99.69	101.86		101.87	0.000825	0.77	159.69	154.42	0.10
Federico Trib	.9675*	Max WS	Dev Unmit100	167.15	99.69	102.23		102.24	0.000940	0.93	223.46	191.80	0.11
Federico Trib	.965*	Max WS	10yr FFDUNM	103.44	99.38	101.82		101.83	0.000840	0.85	152.34	148.86	0.11
Federico Trib	.965*	Max WS	Dev Unmit100	167.11	99.38	102.18		102.19	0.001004	1.04	212.31	183.12	0.12
Federico Trib	.9625*	Max WS	10yr FFDUNM	103.39	99.06	101.77		101.78	0.001100	1.06	135.42	141.33	0.12
Federico Trib	.9625*	Max WS	Dev Unmit100	167.09	99.06	102.12		102.14	0.001318	1.28	190.90	174.00	0.14
Federico Trib	0.96	Max WS	10yr FFDUNM	103.28	98.75	101.67		101.70	0.002429	1.65	101.27	115.96	0.18
Federico Trib	0.96	Max WS	Dev Unmit100	167.07	98.75	102.02		102.05	0.002559	1.84	143.08	124.45	0.19
Federico Trib	.956666*	Max WS	10yr FFDUNM	105.30	98.44	101.34		101.38	0.004092	2.04	84.59	140.05	0.24
Federico Trib	.956666*	Max WS	Dev Unmit100	169.99	98.44	101.71		101.75	0.003457	2.07	141.74	168.45	0.22
Federico Trib	.953333*	Max WS	10yr FFDUNM	105.20	98.13	100.98		101.02	0.003281	1.73	85.66	100.90	0.21
Federico Trib	.953333*	Max WS	Dev Unmit100	169.87	98.13	101.32		101.37	0.004171	2.15	130.13	167.79	0.24
Federico Trib	0.95	Max WS	10yr FFDUNM	104.88	97.82	100.63		100.66	0.003798	1.75	89.37	145.27	0.22
Federico Trib	0.95	Max WS	Dev Unmit100	169.67	97.82	100.94		100.98	0.003676	1.91	138.69	168.39	0.22
Federico Trib	0.94	Max WS	10yr FFDUNM	104.13	97.03	99.90		99.91	0.000652	0.68	217.82	224.77	0.09
Federico Trib	0.94	Max WS	Dev Unmit100	170.52	97.03	100.23		100.23	0.000722	0.80	291.24	235.47	0.10
Federico Trib	.937	Max WS	10yr FFDUNM	105.19	96.60	99.69		99.69	0.000594	0.83	299.70	424.50	0.09
Federico Trib	.937	Max WS	Dev Unmit100	172.28	96.60	100.01		100.01	0.000518	0.84	438.55	438.33	0.09
Federico Trib	.936*	Max WS	10yr FFDUNM	131.31	96.43	99.53		99.57	0.003069	1.80	155.57	339.70	0.21
Federico Trib	.936*	Max WS	Dev Unmit100	214.59	96.43	99.91		99.93	0.001894	1.56	293.16	388.04	0.17
Federico Trib	.935*	Max WS	10yr FFDUNM	131.08	96.27	99.35		99.37	0.002040	1.37	159.53	210.50	0.17
Federico Trib	.935*	Max WS	Dev Unmit100	214.46	96.27	99.78		99.79	0.001640	1.39	257.13	246.24	0.15
Federico Trib	.934	Max WS	10yr FFDUNM	130.97	96.10	99.19		99.21	0.002338	1.37	135.36	157.78	0.17
Federico Trib	.934	Max WS	Dev Unmit100	214.41	96.10	99.64		99.66	0.001956	1.45	214.26	189.37	0.17
Federico Trib	0.933	Max WS	10yr FFDUNM	130.97	96.10	99.16			0.002515	1.41	131.39	156.02	0.18
Federico Trib	0.933	Max WS	Dev Unmit100	214.39	96.10	99.62		99.64	0.002051	1.47	210.36	187.93	0.17
Federico Trib	.931			Mult Open									
Federico Trib	0.93	Max WS	10yr FFDUNM	129.95	96.00	98.90		98.90	0.000593	0.63	276.50	290.44	0.09
Federico Trib	0.93	Max WS	Dev Unmit100	213.71	96.00	99.30		99.30	0.000540	0.70	395.35	301.87	0.09
Federico Trib	.929	Max WS	10yr FFDUNM	129.95	96.00	98.89		98.90	0.000604	0.64	274.75	290.12	0.09
Federico Trib	.929	Max WS	Dev Unmit100	213.71	96.00	99.29		99.30	0.000547	0.70	393.70	301.75	0.09
Federico Trib	.927666*	Max WS	10yr FFDUNM	129.75	95.70	98.85		98.86	0.000560	0.67	272.15	304.71	0.09
Federico Trib	.927666*	Max WS	Dev Unmit100	213.55	95.70	99.26		99.26	0.000545	0.75	397.02	314.61	0.09
Federico Trib	.926333*	Max WS	10yr FFDUNM	129.60	95.40	98.78		98.79	0.001356	1.11	195.16	227.62	0.13
Federico Trib	.926333*	Max WS	Dev Unmit100	213.43	95.40	99.20		99.21	0.001075	1.11	333.35	357.12	0.12
Federico Trib	.925	Max WS	10yr FFDUNM	129.48	95.10	98.70		98.70	0.001146	1.06	221.33	341.28	0.12
Federico Trib	.925	Max WS	Dev Unmit100	213.36	95.10	99.13		99.14	0.000889	1.04	370.58	349.06	0.11
Federico Trib	.923333*	Max WS	10yr FFDUNM	129.45	95.03	98.52		98.56	0.003658	2.05	108.56	141.73	0.22
Federico Trib	.923333*	Max WS	Dev Unmit100	213.31	95.03	99.03		99.05	0.001949	1.69	267.30	323.61	0.17
Federico Trib	.921666*	Max WS	10yr FFDUNM	129.43	94.97	98.15		98.25	0.005981	2.66	57.31	28.18	0.29
Federico Trib	.921666*	Max WS	Dev Unmit100	213.30	94.97	98.67		98.80	0.007039	3.25	110.32	219.55	0.32
Federico Trib	0.92	Max WS	10yr FFDUNM	129.43	94.90	97.93		97.96	0.002331	1.70	102.51	64.88	0.18
Federico Trib	0.92	Max WS	Dev Unmit100	213.18	94.90	98.39		98.44	0.003014	2.14	132.81	67.55	0.21
Federico Trib	.9175*	Max WS	10yr FFDUNM	136.17	94.80	97.76		97.79	0.002269	1.62	115.12	80.60	0.18
Federico Trib	.9175*	Max WS	Dev Unmit100	225.45	94.80	98.17		98.21	0.003009	2.06	148.65	84.56	0.21
Federico Trib	.915*	Max WS	10yr FFDUNM	135.98	94.70	97.66		97.67	0.001091	1.10	202.29	196.09	0.12
Federico Trib	.915*	Max WS	Dev Unmit100	225.27	94.70	98.04		98.05	0.001311	1.32	284.21	234.92	0.14
Federico Trib	.9125*	Max WS	10yr FFDUNM	135.90	94.60	97.60		97.60	0.000608	0.81	241.17	190.60	0.09
Federico Trib	.9125*	Max WS	Dev Unmit100	225.21	94.60	97.96		97.97	0.000788	1.01	316.37	219.99	0.11
Federico Trib	0.91	Max WS	10yr FFDUNM	135.88	94.50	97.55		97.56	0.000745	0.88	236.06	206.75	0.10
Federico Trib	0.91	Max WS	Dev Unmit100	225.17	94.50	97.90		97.91	0.000870	1.05	310.17	210.41	0.11
Federico Trib	.908*	Max WS	10yr FFDUNM	135.79	94.20	97.36		97.41	0.004116	2.11	115.56	192.47	0.24
Federico Trib	.908*	Max WS	Dev Unmit100	225.01	94.20	97.73		97.77	0.003661	2.19	191.44	218.20	0.23
Federico Trib	.906*	Max WS	10yr FFDUNM	135.72	93.90	97.15		97.17	0.002378	1.63	157.05	205.29	0.18
Federico Trib	.906*	Max WS	Dev Unmit100	224.84	93.90	97.54		97.56	0.002130	1.70	243.03	233.21	0.18

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	.904*	Max WS	10yr FFDUNM	135.68	93.60	96.99		97.01	0.002356	1.66	140.10	137.18	0.18
Federico Trib	.904*	Max WS	Dev Unmit100	224.65	93.60	97.37		97.40	0.002789	1.98	200.27	225.24	0.20
Federico Trib	.902*	Max WS	10yr FFDUNM	135.63	93.30	96.72		96.78	0.004781	2.35	102.21	155.15	0.26
Federico Trib	.902*	Max WS	Dev Unmit100	223.87	93.30	97.10		97.16	0.004557	2.52	164.18	173.43	0.26
Federico Trib	0.90	Max WS	10yr FFDUNM	135.56	93.00	96.42		96.47	0.004104	2.16	118.91	188.54	0.24
Federico Trib	0.90	Max WS	Dev Unmit100	221.70	93.00	96.85		96.88	0.003192	2.11	204.39	212.83	0.22
Federico Trib	0.89	Max WS	10yr FFDUNM	136.66	93.00	95.47		95.49	0.003861	1.58	120.79	121.49	0.22
Federico Trib	0.89	Max WS	Dev Unmit100	214.29	93.00	96.29		96.30	0.001520	1.30	224.08	139.60	0.15
Federico Trib	0.88	Max WS	10yr FFDUNM	134.63	91.00	94.04		94.07	0.003179	1.80	118.68	104.23	0.21
Federico Trib	0.88	Max WS	Dev Unmit100	209.03	91.00	95.93		95.94	0.000333	0.87	341.08	145.45	0.07
Federico Trib	.875	Max WS	10yr FFDUNM	137.82	90.70	93.72		93.73	0.000650	0.86	230.43	153.68	0.10
Federico Trib	.875	Max WS	Dev Unmit100	216.41	90.70	95.90		95.90	0.000082	0.47	623.53	184.14	0.04
Federico Trib	0.87	Max WS	10yr FFDUNM	141.60	90.00	93.64		93.64	0.000266	0.60	284.77	147.43	0.06
Federico Trib	0.87	Max WS	Dev Unmit100	223.25	90.00	95.89		95.89	0.000052	0.39	675.07	178.52	0.03
Federico Trib	0.868	Max WS	10yr FFDUNM	144.65	89.01	93.55		93.56	0.000410	0.69	244.75	131.72	0.08
Federico Trib	0.868	Max WS	Dev Unmit100	228.97	89.01	95.87		95.87	0.000078	0.47	550.27	131.72	0.04
Federico Trib	0.867	Max WS	10yr FFDUNM	144.66	89.00	93.54		93.55	0.000608	0.87	193.88	97.17	0.09
Federico Trib	0.867	Max WS	Dev Unmit100	228.97	89.00	95.87		95.87	0.000130	0.61	420.30	97.17	0.05
Federico Trib	.865		Culvert										
Federico Trib	0.864	Max WS	10yr FFDUNM	144.57	88.45	92.26		92.30	0.002566	1.78	121.33	198.32	0.19
Federico Trib	0.864	Max WS	Dev Unmit100	228.90	88.45	92.69		92.72	0.002094	1.78	207.13	200.11	0.18
Federico Trib	0.86	Max WS	10yr FFDUNM	144.51	88.30	92.14		92.19	0.003588	2.14	112.99	202.31	0.22
Federico Trib	0.86	Max WS	Dev Unmit100	228.83	88.30	92.61		92.65	0.002298	1.89	209.26	203.84	0.18
Federico Trib	.856666*	Max WS	10yr FFDUNM	144.95	88.17	91.95		91.97	0.001453	1.38	122.55	74.72	0.15
Federico Trib	.856666*	Max WS	Dev Unmit100	229.66	88.17	92.43		92.47	0.001832	1.73	164.78	99.82	0.17
Federico Trib	.853333*	Max WS	10yr FFDUNM	144.88	88.03	91.88		91.89	0.000492	0.87	200.45	129.86	0.09
Federico Trib	.853333*	Max WS	Dev Unmit100	229.33	88.03	92.35		92.36	0.000660	1.11	269.17	161.56	0.10
Federico Trib	0.85	Max WS	10yr FFDUNM	144.82	87.90	91.81		91.83	0.001059	1.43	220.21	231.78	0.13
Federico Trib	0.85	Max WS	Dev Unmit100	228.87	87.90	92.28		92.30	0.000909	1.43	333.85	248.09	0.12
Federico Trib	.845*	Max WS	10yr FFDUNM	137.87	87.55	91.71		91.71	0.000275	0.69	296.23	203.21	0.07
Federico Trib	.845*	Max WS	Dev Unmit100	206.71	87.55	92.19		92.20	0.000291	0.78	397.88	220.75	0.07
Federico Trib	0.84	Max WS	10yr FFDUNM	147.26	87.20	91.31		91.39	0.003871	2.48	96.18	149.11	0.24
Federico Trib	0.84	Max WS	Dev Unmit100	217.90	87.20	91.96		92.00	0.002155	2.09	195.38	152.97	0.18
Federico Trib	0.83	Max WS	10yr FFDUNM	161.11	86.80	90.69		90.71	0.001284	1.30	188.68	132.11	0.14
Federico Trib	0.83	Max WS	Dev Unmit100	237.77	86.80	91.63		91.64	0.000629	1.10	342.80	181.89	0.10
Federico Trib	0.82	Max WS	10yr FFDUNM	187.17	86.00	90.17		90.22	0.002607	2.07	121.15	59.57	0.20
Federico Trib	0.82	Max WS	Dev Unmit100	278.76	86.00	91.33		91.37	0.001520	1.93	203.23	85.21	0.16
Federico Trib	0.81	Max WS	10yr FFDUNM	205.35	85.70	89.84		89.86	0.000942	1.36	243.78	146.98	0.12
Federico Trib	0.81	Max WS	Dev Unmit100	302.79	85.70	91.16		91.17	0.000358	1.02	447.72	168.42	0.08
Federico Trib	0.807	Max WS	10yr FFDUNM	212.91	84.70	89.64		89.64	0.000276	0.83	336.78	81.78	0.07
Federico Trib	0.807	Max WS	Dev Unmit100	311.80	84.70	91.07		91.08	0.000229	0.91	453.72	81.78	0.07
Federico Trib	0.806	Max WS	10yr FFDUNM	243.37	84.72	89.60		89.61	0.000395	1.00	305.73	100.40	0.08
Federico Trib	0.806	Max WS	Dev Unmit100	348.52	84.72	91.04		91.05	0.000246	0.95	450.52	100.40	0.07
Federico Trib	.805		Culvert										
Federico Trib	0.804	Max WS	10yr FFDUNM	242.01	84.90	88.81		88.85	0.001941	1.75	167.15	77.14	0.17
Federico Trib	0.804	Max WS	Dev Unmit100	318.07	84.90	89.63		89.65	0.000924	1.41	343.31	227.98	0.12
Federico Trib	0.803	Max WS	10yr FFDUNM	242.01	84.46	88.81		88.82	0.000691	1.17	317.09	157.13	0.11
Federico Trib	0.803	Max WS	Dev Unmit100	318.06	84.46	89.62		89.63	0.000417	1.03	445.29	157.13	0.08
Federico Trib	0.80	Max WS	10yr FFDUNM	235.31	84.50	88.61		88.68	0.001238	1.34	129.35	157.21	0.14
Federico Trib	0.80	Max WS	Dev Unmit100	317.27	84.50	89.57		89.60	0.000085	0.42	281.16	160.00	0.04
Federico Trib	0.79	Max WS	10yr FFDUNM	230.12	83.80	88.16		88.18	0.001566	1.67	224.04	148.58	0.16
Federico Trib	0.79	Max WS	Dev Unmit100	320.20	83.80	89.49		89.50	0.000474	1.14	421.61	148.58	0.09
Federico Trib	0.777	Max WS	10yr FFDUNM	230.49	83.00	87.98		87.98	0.000270	0.77	451.75	217.28	0.07
Federico Trib	0.777	Max WS	Dev Unmit100	321.89	83.00	89.43		89.44	0.000096	0.56	768.20	217.28	0.04
Federico Trib	0.776	Max WS	10yr FFDUNM	230.48	82.76	87.96		87.97	0.000290	0.85	436.92	227.66	0.07
Federico Trib	0.776	Max WS	Dev Unmit100	321.88	82.76	89.43		89.43	0.000097	0.59	770.32	227.66	0.04
Federico Trib	.775		Culvert										
Federico Trib	0.774	Max WS	10yr FFDUNM	228.58	82.74	86.94		86.97	0.001819	1.69	249.07	282.27	0.17

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	0.774	Max WS	Dev Unmit100	320.13	82.74	87.42		87.44	0.001115	1.45	402.94	334.46	0.13
Federico Trib	.7739	Max WS	10yr FFDUNM	228.26	82.74	86.92		86.95	0.001883	1.72	244.57	279.06	0.17
Federico Trib	.7739	Max WS	Dev Unmit100	320.04	82.74	87.41		87.43	0.001135	1.46	399.98	333.93	0.13
Federico Trib	0.773	Max WS	10yr FFDUNM	228.16	82.74	86.92		86.93	0.000739	1.11	310.90	274.90	0.11
Federico Trib	0.773	Max WS	Dev Unmit100	320.04	82.74	87.41		87.42	0.000584	1.08	465.00	330.69	0.10
Federico Trib	.77225*	Max WS	10yr FFDUNM	228.09	82.73	86.89		86.90	0.000787	1.14	316.88	322.26	0.11
Federico Trib	.77225*	Max WS	Dev Unmit100	320.36	82.73	87.38		87.39	0.000553	1.05	498.76	386.33	0.10
Federico Trib	.7715*	Max WS	10yr FFDUNM	228.23	82.72	86.86		86.87	0.000787	1.14	335.04	325.04	0.11
Federico Trib	.7715*	Max WS	Dev Unmit100	320.87	82.72	87.36		87.37	0.000542	1.04	518.37	386.29	0.09
Federico Trib	.77075*	Max WS	10yr FFDUNM	228.19	82.71	86.83		86.84	0.000702	1.07	362.83	322.61	0.10
Federico Trib	.77075*	Max WS	Dev Unmit100	321.25	82.71	87.34		87.35	0.000500	0.99	547.07	386.43	0.09
Federico Trib	0.77	Max WS	10yr FFDUNM	228.05	82.70	86.78		86.80	0.001683	1.61	241.96	250.68	0.16
Federico Trib	0.77	Max WS	Dev Unmit100	321.15	82.70	87.31		87.32	0.001030	1.39	393.15	314.30	0.13
Federico Trib	.769*	Max WS	10yr FFDUNM	253.92	82.61	86.71		86.73	0.001763	1.66	255.92	244.70	0.16
Federico Trib	.769*	Max WS	Dev Unmit100	375.31	82.61	87.26		87.28	0.001198	1.52	410.17	306.96	0.14
Federico Trib	.768*	Max WS	10yr FFDUNM	252.27	82.52	86.65		86.67	0.001433	1.51	273.86	243.80	0.15
Federico Trib	.768*	Max WS	Dev Unmit100	374.50	82.52	87.22		87.24	0.001003	1.41	430.71	298.59	0.13
Federico Trib	.767*	Max WS	10yr FFDUNM	251.01	82.43	86.60		86.62	0.001152	1.37	295.09	244.89	0.13
Federico Trib	.767*	Max WS	Dev Unmit100	374.51	82.43	87.19		87.20	0.000838	1.30	454.24	290.12	0.12
Federico Trib	.766*	Max WS	10yr FFDUNM	250.34	82.34	86.57		86.58	0.000920	1.24	319.38	248.82	0.12
Federico Trib	.766*	Max WS	Dev Unmit100	374.69	82.34	87.16		87.17	0.000698	1.20	480.74	285.01	0.11
Federico Trib	.765*	Max WS	10yr FFDUNM	249.88	82.24	86.54		86.55	0.000731	1.12	346.48	253.26	0.11
Federico Trib	.765*	Max WS	Dev Unmit100	375.01	82.24	87.14		87.15	0.000581	1.11	509.19	281.88	0.10
Federico Trib	.764*	Max WS	10yr FFDUNM	249.43	82.15	86.52		86.52	0.000585	1.02	374.50	257.99	0.10
Federico Trib	.764*	Max WS	Dev Unmit100	375.47	82.15	87.12		87.13	0.000487	1.03	537.84	279.94	0.09
Federico Trib	.763*	Max WS	10yr FFDUNM	249.37	82.06	86.50		86.50	0.000466	0.93	404.82	259.22	0.09
Federico Trib	.763*	Max WS	Dev Unmit100	375.68	82.06	87.10		87.11	0.000407	0.96	568.69	283.15	0.08
Federico Trib	.762*	Max WS	10yr FFDUNM	249.15	81.97	86.48		86.49	0.000373	0.84	435.73	259.18	0.08
Federico Trib	.762*	Max WS	Dev Unmit100	376.26	81.97	87.09		87.10	0.000343	0.89	601.18	287.63	0.08
Federico Trib	.761*	Max WS	10yr FFDUNM	249.28	81.88	86.47		86.48	0.000302	0.77	467.45	260.30	0.07
Federico Trib	.761*	Max WS	Dev Unmit100	376.62	81.88	87.08		87.09	0.000290	0.83	635.25	290.96	0.07
Federico Trib	.76	Max WS	10yr FFDUNM	249.42	81.79	86.46		86.46	0.000264	0.73	500.21	264.51	0.07
Federico Trib	.76	Max WS	Dev Unmit100	377.20	81.79	87.07		87.08	0.000270	0.81	670.08	292.45	0.07
Federico Trib	.759375*	Max WS	10yr FFDUNM	249.57	81.72	86.45		86.45	0.000293	0.77	467.00	256.04	0.07
Federico Trib	.759375*	Max WS	Dev Unmit100	377.68	81.72	87.06		87.06	0.000294	0.85	632.31	286.88	0.07
Federico Trib	.75875*	Max WS	10yr FFDUNM	249.58	81.64	86.44		86.44	0.000332	0.81	441.44	235.79	0.07
Federico Trib	.75875*	Max WS	Dev Unmit100	378.17	81.64	87.05		87.05	0.000338	0.91	599.40	278.46	0.08
Federico Trib	.758125*	Max WS	10yr FFDUNM	249.75	81.57	86.42		86.43	0.000354	0.84	429.19	207.39	0.08
Federico Trib	.758125*	Max WS	Dev Unmit100	378.67	81.57	87.03		87.04	0.000371	0.95	571.96	263.16	0.08
Federico Trib	.7575*	Max WS	10yr FFDUNM	249.91	81.49	86.41		86.42	0.000377	0.86	427.14	203.49	0.08
Federico Trib	.7575*	Max WS	Dev Unmit100	379.25	81.49	87.02		87.02	0.000399	0.98	555.43	225.06	0.08
Federico Trib	.756875*	Max WS	10yr FFDUNM	249.95	81.42	86.39		86.40	0.000401	0.88	426.35	206.55	0.08
Federico Trib	.756875*	Max WS	Dev Unmit100	379.75	81.42	87.00		87.01	0.000421	1.00	551.90	207.82	0.08
Federico Trib	.75625*	Max WS	10yr FFDUNM	250.01	81.35	86.38		86.39	0.000419	0.89	424.75	205.44	0.08
Federico Trib	.75625*	Max WS	Dev Unmit100	380.26	81.35	86.98		86.99	0.000440	1.01	549.39	206.48	0.09
Federico Trib	.755625*	Max WS	10yr FFDUNM	250.19	81.27	86.36		86.37	0.000448	0.91	420.53	204.55	0.08
Federico Trib	.755625*	Max WS	Dev Unmit100	380.84	81.27	86.97		86.98	0.000471	1.04	544.39	205.42	0.09
Federico Trib	.755	Max WS	10yr FFDUNM	250.27	81.20	86.34		86.35	0.000509	0.90	413.05	203.84	0.08
Federico Trib	.755	Max WS	Dev Unmit100	381.35	81.20	86.95		86.96	0.000528	1.02	536.28	204.58	0.09
Federico Trib	.754*	Max WS	10yr FFDUNM	258.93	80.96	86.31		86.32	0.000490	0.93	424.49	214.66	0.09
Federico Trib	.754*	Max WS	Dev Unmit100	397.79	80.96	86.91		86.92	0.000514	1.06	553.95	215.75	0.09
Federico Trib	.753*	Max WS	10yr FFDUNM	259.20	80.72	86.28		86.29	0.000471	0.90	436.06	225.94	0.08
Federico Trib	.753*	Max WS	Dev Unmit100	398.49	80.72	86.88		86.89	0.000487	1.02	572.21	227.62	0.09
Federico Trib	.752*	Max WS	10yr FFDUNM	259.38	80.48	86.25		86.26	0.000423	0.84	459.95	238.30	0.08
Federico Trib	.752*	Max WS	Dev Unmit100	399.41	80.48	86.85		86.86	0.000437	0.95	607.36	270.68	0.08
Federico Trib	.751*	Max WS	10yr FFDUNM	259.69	80.24	86.22		86.23	0.000361	0.75	507.18	291.48	0.07
Federico Trib	.751*	Max WS	Dev Unmit100	400.32	80.24	86.82		86.83	0.000359	0.84	698.39	330.88	0.07
Federico Trib	0.75	Max WS	10yr FFDUNM	260.11	80.00	86.20		86.21	0.000281	0.69	610.68	350.36	0.07
Federico Trib	0.75	Max WS	Dev Unmit100	401.19	80.00	86.80		86.81	0.000272	0.76	821.63	352.33	0.07

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Federico Trib	.7494*	Max WS	10yr FFDUNM	262.39	80.00	86.18		86.18	0.000451	0.90	506.81	349.86	0.08
Federico Trib	.7494*	Max WS	Dev Unmit100	405.65	80.00	86.78		86.79	0.000404	0.94	722.86	360.48	0.08
Federico Trib	.7488*	Max WS	10yr FFDUNM	262.80	80.00	86.13		86.14	0.000775	1.20	389.43	285.26	0.11
Federico Trib	.7488*	Max WS	Dev Unmit100	406.61	80.00	86.74		86.75	0.000636	1.20	591.94	351.43	0.10
Federico Trib	.7482*	Max WS	10yr FFDUNM	263.25	80.00	86.04		86.07	0.001529	1.70	283.33	259.58	0.15
Federico Trib	.7482*	Max WS	Dev Unmit100	407.58	80.00	86.67		86.70	0.001115	1.61	454.66	331.77	0.13
Federico Trib	.7476*	Max WS	10yr FFDUNM	263.72	80.00	85.76		85.89	0.005072	3.03	134.22	237.54	0.27
Federico Trib	.7476*	Max WS	Dev Unmit100	408.56	80.00	86.54		86.59	0.002311	2.31	344.55	328.57	0.19
Federico Trib	.747	Max WS	10yr FFDUNM	264.23	80.00	84.64		85.05	0.021715	5.17	52.22	21.51	0.53
Federico Trib	.747	Max WS	Dev Unmit100	409.56	80.00	85.39		85.98	0.023396	6.24	71.00	30.17	0.57
Federico Trib	.746*	Max WS	10yr FFDUNM	264.48	79.67	83.89		84.30	0.021376	5.18	53.59	25.12	0.54
Federico Trib	.746*	Max WS	Dev Unmit100	410.05	79.67	84.25		84.99	0.033623	7.03	63.60	31.44	0.68
Federico Trib	.745*	Max WS	10yr FFDUNM	264.28	79.33	83.37		83.67	0.014864	4.57	75.80	123.88	0.46
Federico Trib	.745*	Max WS	Dev Unmit100	409.96	79.33	83.81		84.05	0.012776	4.63	157.09	206.98	0.44
Federico Trib	.744	Max WS	10yr FFDUNM	264.19	79.00	83.23		83.25	0.001149	1.43	303.50	218.06	0.13
Federico Trib	.744	Max WS	Dev Unmit100	410.26	79.00	83.68		83.70	0.001266	1.63	404.41	234.43	0.14
Federico Trib	0.74	Max WS	10yr FFDUNM	265.26	77.38	82.43		82.46	0.001866	1.80	244.47	265.94	0.17
Federico Trib	0.74	Max WS	Dev Unmit100	414.56	77.38	82.90		82.93	0.001679	1.86	370.75	272.98	0.16
Federico Trib	0.73	Max WS	10yr FFDUNM	267.57	76.00	81.83		81.85	0.001691	1.79	260.58	224.17	0.16
Federico Trib	0.73	Max WS	Dev Unmit100	420.82	76.00	82.32		82.35	0.001734	1.96	379.06	264.98	0.16
Federico Trib	0.72	Max WS	10yr FFDUNM	264.77	76.00	81.29		81.32	0.001508	1.52	280.37	283.20	0.15
Federico Trib	0.72	Max WS	Dev Unmit100	418.58	76.00	81.81		81.84	0.001301	1.56	442.13	338.71	0.14
Federico Trib	0.71	Max WS	10yr FFDUNM	267.88	75.00	80.83		80.85	0.001062	1.38	315.91	260.53	0.13
Federico Trib	0.71	Max WS	Dev Unmit100	426.47	75.00	81.41		81.43	0.000928	1.43	476.80	285.59	0.12
Federico Trib	0.70	Max WS	10yr FFDUNM	270.23	75.00	80.62		80.65	0.001483	1.60	244.81	169.06	0.15
Federico Trib	0.70	Max WS	Dev Unmit100	430.63	75.00	81.20		81.23	0.001569	1.82	352.97	216.11	0.16
Federico Trib	.666666*	Max WS	10yr FFDUNM	271.25	75.00	80.46		80.50	0.001963	1.83	213.19	149.52	0.17
Federico Trib	.666666*	Max WS	Dev Unmit100	431.41	75.00	81.03		81.08	0.002073	2.08	327.72	262.22	0.18
Federico Trib	.633333*	Max WS	10yr FFDUNM	270.52	75.00	80.27		80.31	0.002258	1.93	218.32	217.39	0.18
Federico Trib	.633333*	Max WS	Dev Unmit100	429.52	75.00	80.87		80.90	0.001726	1.88	378.49	277.48	0.17
Federico Trib	0.6	Max WS	10yr FFDUNM	269.70	75.00	80.08		80.11	0.002192	1.85	257.32	289.97	0.18
Federico Trib	0.6	Max WS	Dev Unmit100	427.59	75.00	80.75		80.77	0.001235	1.57	453.85	300.01	0.14
Federico Trib	.55*	Max WS	10yr FFDUNM	279.34	74.50	79.96		79.99	0.000916	1.32	297.12	240.38	0.12
Federico Trib	.55*	Max WS	Dev Unmit100	443.51	74.50	80.67		80.69	0.000778	1.36	471.84	252.92	0.11
Federico Trib	0.5	Max WS	10yr FFDUNM	278.99	74.00	79.93		79.94	0.000267	0.75	481.28	246.52	0.07
Federico Trib	0.5	Max WS	Dev Unmit100	442.73	74.00	80.63		80.64	0.000297	0.88	671.15	280.76	0.07
Federico Trib	.45*	Max WS	10yr FFDUNM	278.74	73.50	79.90		79.91	0.000633	1.10	327.33	192.07	0.10
Federico Trib	.45*	Max WS	Dev Unmit100	441.67	73.50	80.60		80.62	0.000639	1.24	465.68	199.12	0.10
Federico Trib	0.4	Max WS	10yr FFDUNM	277.23	73.00	79.78		79.83	0.002663	1.95	174.59	110.55	0.20
Federico Trib	0.4	Max WS	Dev Unmit100	437.37	73.00	80.48		80.54	0.002436	2.14	253.00	112.33	0.19
Main Reach	1.44	Max WS	10yr FFDUNM	146.00	112.07	114.33		114.34	0.001391	0.69	223.62	285.17	0.12
Main Reach	1.44	Max WS	Dev Unmit100	285.12	112.07	114.75		114.76	0.001434	0.88	346.46	302.17	0.13
Main Reach	1.43666*	Max WS	10yr FFDUNM	145.98	111.85	114.18		114.19	0.001618	0.84	183.23	193.47	0.14
Main Reach	1.43666*	Max WS	Dev Unmit100	283.82	111.85	114.56		114.58	0.002252	1.19	268.74	280.79	0.17
Main Reach	1.43333*	Max WS	10yr FFDUNM	145.91	111.62	113.94		113.96	0.003056	1.23	159.78	320.82	0.19
Main Reach	1.43333*	Max WS	Dev Unmit100	280.21	111.62	114.29		114.31	0.003101	1.45	274.28	336.99	0.20
Main Reach	1.43	Max WS	10yr FFDUNM	145.75	111.40	113.49		113.51	0.006282	1.62	151.08	299.95	0.26
Main Reach	1.43	Max WS	Dev Unmit100	278.31	111.40	114.04		114.05	0.002098	1.20	320.36	315.61	0.16
Main Reach	1.425*	Max WS	10yr FFDUNM	143.87	110.50	113.00		113.01	0.002342	1.32	137.76	112.61	0.17
Main Reach	1.425*	Max WS	Dev Unmit100	277.80	110.50	113.67		113.71	0.004596	2.28	223.79	259.84	0.25
Main Reach	1.42	Max WS	10yr FFDUNM	141.55	109.60	112.84		112.85	0.000506	0.81	221.80	175.81	0.09
Main Reach	1.42	Max WS	Dev Unmit100	274.60	109.60	113.41		113.42	0.000762	1.13	322.43	177.72	0.11
Main Reach	1.41	Max WS	10yr FFDUNM	152.21	108.40	112.69		112.71	0.000984	1.20	244.61	288.41	0.12
Main Reach	1.41	Max WS	Dev Unmit100	282.71	108.40	113.25		113.26	0.000903	1.29	411.96	315.76	0.12
Main Reach	1.40	Max WS	10yr FFDUNM	153.32	108.40	112.46		112.47	0.000646	1.03	294.43	267.99	0.10
Main Reach	1.40	Max WS	Dev Unmit100	273.56	108.40	113.03		113.04	0.000620	1.12	468.24	339.15	0.10
Main Reach	1.39	Max WS	10yr FFDUNM	152.82	107.50	111.76		111.82	0.004091	2.16	91.73	213.22	0.24
Main Reach	1.39	Max WS	Dev Unmit100	272.16	107.50	112.83		112.84	0.000797	1.19	414.99	329.42	0.11
Main Reach	1.388	Max WS	10yr FFDUNM	153.67	107.00	111.21		111.21	0.000378	0.67	323.36	246.15	0.07
Main Reach	1.388	Max WS	Dev Unmit100	275.46	107.00	112.71		112.72	0.000112	0.50	694.04	246.15	0.04

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	1.387	Max WS	10yr FFDUNM	153.65	107.00	111.20		111.20	0.000123	0.45	478.61	283.35	0.04
Main Reach	1.387	Max WS	Dev Unmit100	275.41	107.00	112.71		112.71	0.000055	0.39	906.52	283.35	0.03
Main Reach	1.385			Culvert									
Main Reach	1.384	Max WS	10yr FFDUNM	153.65	107.00	110.70		110.71	0.000227	0.54	386.41	235.14	0.06
Main Reach	1.384	Max WS	Dev Unmit100	275.40	107.00	111.33		111.33	0.000257	0.66	533.37	235.14	0.06
Main Reach	1.38	Max WS	10yr FFDUNM	153.64	107.00	110.63		110.65	0.002438	1.66	144.83	99.50	0.18
Main Reach	1.38	Max WS	Dev Unmit100	275.39	107.00	111.26		111.28	0.002403	1.90	349.12	420.80	0.19
Main Reach	1.37	Max WS	10yr FFDUNM	166.40	106.00	109.95		109.98	0.001596	1.48	147.37	90.58	0.15
Main Reach	1.37	Max WS	Dev Unmit100	296.66	106.00	110.52		110.56	0.002075	1.91	234.86	212.83	0.18
Main Reach	1.36	Max WS	10yr FFDUNM	180.45	105.80	108.96		108.99	0.003145	2.02	174.32	212.10	0.21
Main Reach	1.36	Max WS	Dev Unmit100	318.69	105.80	109.37		109.40	0.003460	2.32	261.47	240.00	0.23
Main Reach	1.35	Max WS	10yr FFDUNM	182.22	104.80	108.15		108.19	0.003244	1.74	166.55	278.30	0.21
Main Reach	1.35	Max WS	Dev Unmit100	321.92	104.80	108.51		108.54	0.003325	1.95	271.85	312.57	0.22
Main Reach	1.34	Max WS	10yr FFDUNM	184.47	104.46	107.20		107.22	0.003056	1.62	197.21	285.18	0.20
Main Reach	1.34	Max WS	Dev Unmit100	325.88	104.46	107.66		107.68	0.002198	1.57	336.33	320.08	0.18
Main Reach	1.335	Max WS	10yr FFDUNM	192.01	102.00	106.39		106.40	0.001654	1.55	213.62	184.34	0.15
Main Reach	1.335	Max WS	Dev Unmit100	330.27	102.00	107.04		107.06	0.001382	1.60	345.67	242.80	0.14
Main Reach	1.333			Mult Open									
Main Reach	1.33	Max WS	10yr FFDUNM	191.89	102.00	106.32		106.37	0.003843	2.23	127.12	80.57	0.23
Main Reach	1.33	Max WS	Dev Unmit100	330.29	102.00	107.00		107.07	0.003933	2.60	181.86	80.57	0.24
Main Reach	1.32	Max WS	10yr FFDUNM	189.52	101.80	105.67		105.68	0.000851	1.13	337.27	311.55	0.11
Main Reach	1.32	Max WS	Dev Unmit100	330.03	101.80	106.39		106.39	0.000546	1.04	566.16	330.38	0.09
Main Reach	1.31	Max WS	10yr FFDUNM	211.67	100.90	105.06		105.11	0.003297	2.22	139.28	103.80	0.22
Main Reach	1.31	Max WS	Dev Unmit100	389.97	100.90	105.71		105.79	0.003856	2.73	213.70	123.75	0.25
Main Reach	1.30	Max WS	10yr FFDUNM	214.46	100.63	104.44		104.45	0.000877	0.96	308.63	238.56	0.11
Main Reach	1.30	Max WS	Dev Unmit100	352.92	100.63	105.13		105.14	0.000633	0.96	492.07	285.91	0.10
Main Reach	1.29	Max WS	10yr FFDUNM	228.55	100.40	103.99		104.04	0.002853	2.00	202.37	238.61	0.21
Main Reach	1.29	Max WS	Dev Unmit100	360.85	100.40	104.94		104.96	0.000992	1.43	443.19	264.64	0.13
Main Reach	1.28	Max WS	10yr FFDUNM	225.42	100.00	103.40		103.42	0.001484	1.41	287.57	284.31	0.15
Main Reach	1.28	Max WS	Dev Unmit100	362.24	100.00	104.77		104.78	0.000283	0.80	753.65	392.50	0.07
Main Reach	1.27	Max WS	10yr FFDUNM	226.31	98.88	103.18		103.18	0.000154	0.47	602.89	297.63	0.05
Main Reach	1.27	Max WS	Dev Unmit100	364.31	98.88	104.73		104.73	0.000066	0.41	1112.93	358.57	0.03
Main Reach	1.268	Max WS	10yr FFDUNM	227.12	98.63	103.14		103.14	0.000479	0.86	396.06	424.81	0.09
Main Reach	1.268	Max WS	Dev Unmit100	365.32	98.63	104.72		104.72	0.000092	0.50	1098.80	452.33	0.04
Main Reach	1.267	Max WS	10yr FFDUNM	227.11	98.60	103.11		103.12	0.000564	0.84	306.00	230.51	0.09
Main Reach	1.267	Max WS	Dev Unmit100	365.34	98.60	104.71		104.71	0.000128	0.55	962.10	437.18	0.05
Main Reach	1.265			Culvert									
Main Reach	1.26	Max WS	10yr FFDUNM	225.88	98.56	102.24		102.27	0.003480	1.66	193.07	302.79	0.21
Main Reach	1.26	Max WS	Dev Unmit100	362.02	98.56	103.20		103.21	0.000793	1.04	490.04	320.42	0.11
Main Reach	1.259	Max WS	10yr FFDUNM	225.47	98.50	102.16		102.17	0.001481	1.36	327.46	365.92	0.15
Main Reach	1.259	Max WS	Dev Unmit100	361.87	98.50	103.18		103.18	0.000367	0.84	700.88	368.07	0.08
Main Reach	1.255	Max WS	10yr FFDUNM	237.35	97.97	101.86		101.86	0.000405	0.74	484.48	366.68	0.08
Main Reach	1.255	Max WS	Dev Unmit100	379.70	97.97	103.10		103.10	0.000135	0.54	994.13	425.83	0.05
Main Reach	1.22	Max WS	10yr FFDUNM	237.31	97.50	101.79		101.80	0.000381	0.71	428.03	279.14	0.08
Main Reach	1.22	Max WS	Dev Unmit100	379.69	97.50	103.07		103.07	0.000158	0.59	853.50	384.21	0.05
Main Reach	1.215			Culvert									
Main Reach	1.21	Max WS	10yr FFDUNM	237.30	96.51	99.88		100.08	0.007058	3.60	65.90	43.95	0.52
Main Reach	1.21	Max WS	Dev Unmit100	379.65	96.51	100.21		100.54	0.011229	4.62	82.18	53.54	0.66
Main Reach	1.20	Max WS	10yr FFDUNM	237.08	96.05	99.50		99.64	0.006380	3.48	110.45	175.28	0.49
Main Reach	1.20	Max WS	Dev Unmit100	378.94	96.05	99.83		99.97	0.005595	3.69	172.36	200.88	0.47
Main Reach	1.19	Max WS	10yr FFDUNM	260.45	95.07	98.48		98.52	0.001258	1.74	201.52	250.94	0.23
Main Reach	1.19	Max WS	Dev Unmit100	410.47	95.07	98.95		98.99	0.001029	1.84	333.93	309.41	0.21
Main Reach	1.18	Max WS	10yr FFDUNM	257.85	94.10	97.82		97.92	0.002620	3.27	141.90	124.31	0.35
Main Reach	1.18	Max WS	Dev Unmit100	406.72	94.10	98.35		98.45	0.002504	3.59	215.84	159.02	0.35
Main Reach	1.17	Max WS	10yr FFDUNM	289.16	93.80	97.17		97.22	0.000995	1.89	192.17	150.11	0.21
Main Reach	1.17	Max WS	Dev Unmit100	486.59	93.80	97.59		97.68	0.001432	2.52	262.81	184.38	0.26
Main Reach	1.16	Max WS	10yr FFDUNM	288.66	93.25	96.61		96.68	0.002148	2.62	210.38	299.05	0.31
Main Reach	1.16	Max WS	Dev Unmit100	482.72	93.25	97.00		97.06	0.002027	2.83	335.72	344.77	0.30

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	1.15	Max WS	10yr FFDUNM	288.53	92.65	96.17		96.18	0.000457	1.41	420.97	428.98	0.15
Main Reach	1.15	Max WS	Dev Unmit100	482.38	92.65	96.57		96.59	0.000501	1.61	595.72	440.95	0.16
Main Reach	1.14	Max WS	10yr FFDUNM	288.86	92.20	94.87		94.94	0.003952	2.98	167.15	193.69	0.40
Main Reach	1.14	Max WS	Dev Unmit100	482.43	92.20	95.25		95.34	0.003837	3.36	246.00	216.20	0.41
Main Reach	1.13	Max WS	10yr FFDUNM	288.06	91.95	94.08		94.09	0.000483	0.77	372.33	503.10	0.13
Main Reach	1.13	Max WS	Dev Unmit100	476.73	91.95	94.50		94.52	0.000447	0.92	523.46	546.40	0.13
Main Reach	1.12	Max WS	10yr FFDUNM	287.44	90.70	93.50		93.59	0.003281	3.14	167.31	182.57	0.38
Main Reach	1.12	Max WS	Dev Unmit100	471.96	90.70	94.00		94.09	0.002806	3.33	271.57	431.06	0.36
Main Reach	1.11	Max WS	10yr FFDUNM	286.85	90.02	93.02		93.03	0.000364	1.00	408.67	320.62	0.12
Main Reach	1.11	Max WS	Dev Unmit100	470.59	90.02	93.60		93.61	0.000326	1.12	600.68	344.16	0.12
Main Reach	1.10	Max WS	10yr FFDUNM	286.44	88.90	92.39		92.53	0.003337	2.94	103.42	85.77	0.37
Main Reach	1.10	Max WS	Dev Unmit100	469.09	88.90	92.91		93.10	0.003585	3.57	159.85	132.57	0.40
Main Reach	1.09	Max WS	10yr FFDUNM	286.17	87.51	91.82		91.83	0.000220	1.06	466.07	322.10	0.10
Main Reach	1.09	Max WS	Dev Unmit100	468.12	87.51	92.31		92.32	0.000270	1.29	631.62	351.55	0.12
Main Reach	1.08	Max WS	10yr FFDUNM	287.95	87.08	91.54		91.60	0.001384	2.39	216.95	215.95	0.25
Main Reach	1.08	Max WS	Dev Unmit100	406.69	87.08	92.05		92.09	0.001030	2.30	341.18	276.13	0.22
Main Reach	1.07	Max WS	10yr FFDUNM	275.99	87.00	90.72		90.84	0.003209	2.96	119.26	140.22	0.37
Main Reach	1.07	Max WS	Dev Unmit100	396.49	87.00	91.78		91.82	0.000819	1.95	349.15	295.13	0.20
Main Reach	1.06	Max WS	10yr FFDUNM	262.86	86.38	90.22		90.23	0.000352	1.13	313.73	319.94	0.13
Main Reach	1.06	Max WS	Dev Unmit100	393.85	86.38	91.65		91.65	0.000081	0.74	833.97	405.85	0.07
Main Reach	1.05	Max WS	10yr FFDUNM	261.98	85.43	90.15		90.16	0.000181	0.91	415.53	273.72	0.09
Main Reach	1.05	Max WS	Dev Unmit100	395.04	85.43	91.63		91.63	0.000062	0.70	927.06	415.53	0.06
Main Reach	1.04	Max WS	10yr FFDUNM	263.38	84.89	90.13		90.14	0.000118	0.68	951.24	1965.47	0.07
Main Reach	1.04	Max WS	Dev Unmit100	397.54	84.89	91.63		91.63	0.000006	0.21	3925.28	2020.32	0.02
Main Reach	1.03	Max WS	10yr FFDUNM	263.28	84.80	90.12		90.13	0.000198	0.82	495.94	1708.85	0.09
Main Reach	1.03	Max WS	Dev Unmit100	397.53	84.80	91.63		91.63	0.000010	0.25	3175.84	1835.40	0.02
Main Reach	1.025			Culvert									
Main Reach	1.02	Max WS	10yr FFDUNM	262.51	84.82	89.45		89.45	0.000073	0.33	1196.48	835.15	0.03
Main Reach	1.02	Max WS	Dev Unmit100	396.29	84.82	90.15		90.15	0.000049	0.31	1762.74	907.02	0.03
Main Reach	1.01	Max WS	10yr FFDUNM	262.46	84.30	89.18		89.22	0.001374	1.80	227.50	129.04	0.15
Main Reach	1.01	Max WS	Dev Unmit100	396.21	84.30	89.91		89.95	0.001223	1.89	321.74	129.04	0.15
Main Reach	1.00	Max WS	10yr FFDUNM	274.07	83.80	88.86		88.91	0.003276	2.25	193.13	179.34	0.22
Main Reach	1.00	Max WS	Dev Unmit100	412.88	83.80	89.73		89.76	0.001561	1.80	365.96	215.58	0.16
Main Reach	0.95	Max WS	10yr FFDUNM	274.61	83.20	88.25		88.33	0.002786	2.39	124.42	41.34	0.21
Main Reach	0.95	Max WS	Dev Unmit100	413.63	83.20	89.33		89.41	0.002199	2.50	222.91	105.36	0.20
Main Reach	0.90	Max WS	10yr FFDUNM	274.48	82.09	87.67		87.72	0.001894	1.75	173.11	90.69	0.17
Main Reach	0.90	Max WS	Dev Unmit100	413.50	82.09	88.93		88.97	0.001100	1.66	287.58	90.69	0.14
Main Reach	0.88	Max WS	10yr FFDUNM	274.45	82.00	87.60		87.63	0.001541	1.63	244.52	193.31	0.15
Main Reach	0.88	Max WS	Dev Unmit100	413.51	82.00	88.91		88.93	0.000502	1.15	525.93	235.32	0.09
Main Reach	.87			Culvert									
Main Reach	0.86	Max WS	10yr FFDUNM	274.20	81.83	87.14		87.17	0.002006	1.66	207.28	152.28	0.17
Main Reach	0.86	Max WS	Dev Unmit100	413.08	81.83	87.78		87.82	0.001723	1.76	315.57	183.34	0.16
Main Reach	0.85	Max WS	10yr FFDUNM	274.09	81.10	87.06		87.08	0.000769	1.44	243.61	89.32	0.11
Main Reach	0.85	Max WS	Dev Unmit100	412.97	81.10	87.70		87.74	0.000949	1.74	322.67	193.53	0.13
Main Reach	0.80	Max WS	10yr FFDUNM	278.77	81.10	86.94		86.96	0.001105	1.65	286.50	242.60	0.14
Main Reach	0.80	Max WS	Dev Unmit100	420.27	81.10	87.59		87.61	0.000909	1.63	445.13	242.60	0.13
Main Reach	0.75	Max WS	10yr FFDUNM	281.59	80.50	86.66		86.70	0.001480	1.71	248.98	222.45	0.15
Main Reach	0.75	Max WS	Dev Unmit100	424.87	80.50	87.38		87.41	0.001067	1.63	410.42	224.10	0.13
Main Reach	0.70	Max WS	10yr FFDUNM	280.03	80.40	86.43		86.45	0.000914	1.42	327.60	192.56	0.12
Main Reach	0.70	Max WS	Dev Unmit100	423.75	80.40	87.20		87.22	0.000740	1.43	482.62	206.97	0.11
Main Reach	0.65	Max WS	10yr FFDUNM	279.50	80.20	86.31		86.32	0.000401	0.97	475.71	253.75	0.08
Main Reach	0.65	Max WS	Dev Unmit100	423.51	80.20	87.11		87.11	0.000345	1.01	690.15	302.97	0.08
Main Reach	0.6	Max WS	10yr FFDUNM	279.32	80.20	86.20		86.21	0.000509	1.10	443.77	309.49	0.09
Main Reach	0.6	Max WS	Dev Unmit100	424.03	80.20	87.02		87.03	0.000368	1.04	709.72	366.89	0.08
Main Reach	0.555	Max WS	10yr FFDUNM	279.22	80.20	86.09		86.10	0.000619	1.25	367.63	210.70	0.10
Main Reach	0.555	Max WS	Dev Unmit100	424.29	80.20	86.94		86.95	0.000496	1.25	559.81	242.10	0.09
Main Reach	0.552			Mult Open									
Main Reach	0.55	Max WS	10yr FFDUNM	278.68	79.60	85.89		85.91	0.000922	1.43	348.63	306.46	0.12

HEC-RAS Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Reach	0.55	Max WS	Dev Unmit100	423.76	79.60	86.69		86.70	0.000557	1.25	603.02	323.16	0.10
Main Reach	0.5	Max WS	10yr FFDUNM	276.65	79.60	85.61		85.63	0.001187	1.58	298.06	294.00	0.14
Main Reach	0.5	Max WS	Dev Unmit100	423.29	79.60	86.55		86.56	0.000556	1.24	588.46	314.70	0.10
Main Reach	0.45	Max WS	10yr FFDUNM	276.09	79.60	85.43		85.44	0.000521	1.17	456.95	333.80	0.09
Main Reach	0.45	Max WS	Dev Unmit100	423.53	79.60	86.46		86.47	0.000277	0.97	801.49	333.80	0.07
Main Reach	0.40	Max WS	10yr FFDUNM	277.10	79.60	85.37		85.37	0.000131	0.58	780.93	372.00	0.05
Main Reach	0.40	Max WS	Dev Unmit100	425.05	79.60	86.43		86.43	0.000093	0.56	1173.72	372.00	0.04
Main Reach	0.35	Max WS	10yr FFDUNM	277.08	79.23	85.35		85.35	0.000091	0.53	770.23	362.30	0.04
Main Reach	0.35	Max WS	Dev Unmit100	425.03	79.23	86.41		86.42	0.000070	0.52	1154.61	362.30	0.04
Main Reach	0.33			Multi Open									
Main Reach	0.30	Max WS	10yr FFDUNM	276.95	79.23	84.19		84.20	0.000469	0.89	348.12	157.57	0.09
Main Reach	0.30	Max WS	Dev Unmit100	424.89	79.23	84.68		84.70	0.000629	1.13	426.03	161.16	0.10
Main Reach	0.25	Max WS	10yr FFDUNM	276.89	79.20	84.05		84.08	0.001247	1.62	331.54	315.70	0.14
Main Reach	0.25	Max WS	Dev Unmit100	424.83	79.20	84.54		84.56	0.001161	1.68	485.82	315.70	0.14
Main Reach	0.20	Max WS	10yr FFDUNM	277.64	79.00	83.81		83.87	0.002524	2.21	210.38	327.12	0.20
Main Reach	0.20	Max WS	Dev Unmit100	426.38	79.00	84.34		84.39	0.002012	2.16	388.47	345.64	0.18
Main Reach	0.15	Max WS	10yr FFDUNM	278.33	78.70	83.41		83.44	0.001600	1.75	269.53	237.35	0.16
Main Reach	0.15	Max WS	Dev Unmit100	428.25	78.70	84.01		84.04	0.001376	1.79	411.84	237.35	0.15
Main Reach	0.10	Max WS	10yr FFDUNM	278.51	78.70	83.22		83.26	0.001723	1.84	283.40	250.08	0.17
Main Reach	0.10	Max WS	Dev Unmit100	428.59	78.70	83.86		83.89	0.001293	1.77	444.05	254.29	0.15
Main Reach	0.05	Max WS	10yr FFDUNM	278.44	78.20	82.95		82.96	0.000852	1.29	440.24	336.94	0.12
Main Reach	0.05	Max WS	Dev Unmit100	428.65	78.20	83.66		83.67	0.000564	1.17	681.78	338.96	0.10
Main Reach	0.04	Max WS	10yr FFDUNM	278.51	77.40	82.32		82.41	0.004115	2.65	136.84	55.12	0.25
Main Reach	0.04	Max WS	Dev Unmit100	428.77	77.40	83.06		83.16	0.004175	3.03	211.76	141.97	0.26
Main Reach	0.03	Max WS	10yr FFDUNM	278.57	76.70	81.56		81.65	0.003087	2.56	134.65	89.61	0.22
Main Reach	0.03	Max WS	Dev Unmit100	428.89	76.70	82.09		82.25	0.004628	3.41	183.32	294.37	0.28
Main Reach	0.02	Max WS	10yr FFDUNM	278.64	76.20	81.05		81.08	0.001835	1.74	330.20	390.91	0.17
Main Reach	0.02	Max WS	Dev Unmit100	428.94	76.20	81.50		81.52	0.001391	1.65	512.70	415.90	0.15
Main Reach	0.01	Max WS	10yr FFDUNM	279.97	75.70	80.65		80.68	0.001740	1.72	282.83	304.17	0.16
Main Reach	0.01	Max WS	Dev Unmit100	431.11	75.70	81.20		81.22	0.001272	1.62	457.53	324.20	0.14
Main Reach	0.009	Max WS	10yr FFDUNM	279.71	75.08	80.49		80.51	0.001125	1.41	302.78	223.00	0.13
Main Reach	0.009	Max WS	Dev Unmit100	430.65	75.08	81.07		81.09	0.001004	1.47	430.90	223.00	0.13
Main Reach	0.008	Max WS	10yr FFDUNM	276.51	73.85	80.03		80.09	0.002320	2.09	144.27	261.96	0.19
Main Reach	0.008	Max WS	Dev Unmit100	367.98	73.85	80.71		80.74	0.001333	1.72	369.56	397.63	0.15
Main Reach	0.007	Max WS	10yr FFDUNM	210.69	73.00	79.78		79.79	0.000631	1.17	215.66	87.46	0.10
Main Reach	0.007	Max WS	Dev Unmit100	361.37	73.00	80.48		80.51	0.000838	1.46	346.42	247.92	0.12
Down	6.9	Max WS	10yr FFDUNM	487.92	72.00	79.78		79.82	0.000918	1.79	318.10	218.99	0.16
Down	6.9	Max WS	Dev Unmit100	798.73	72.00	80.48		80.52	0.000871	1.93	677.61	654.01	0.16
Down	6.85*	Max WS	10yr FFDUNM	487.90	71.71	79.70		79.75	0.000833	1.89	328.61	207.44	0.16
Down	6.85*	Max WS	Dev Unmit100	798.71	71.71	80.38		80.44	0.001013	2.26	525.98	359.07	0.18
Down	6.8	Max WS	10yr FFDUNM	487.89	71.43	79.59	75.59	79.66	0.001100	2.28	286.82	158.65	0.18
Down	6.8	Max WS	Dev Unmit100	798.70	71.43	80.25	76.59	80.34	0.001471	2.81	417.73	256.01	0.21

**APPENDIX G
PROPOSED ONSITE PROJECT
STORM DRAIN ANALYSIS
10-YEAR and 100-YEAR
CSDS REPORT**

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
AA14	AA13	24	0.1	0.1	13.5	0.16	0.16	0.015	308.00	5.13	117.00	111.60	111.60	5.40	0.0000	111.60	111.60
		N/A				0.00		0.015	308.00	0.00	117.00	111.60	111.60	N/A	0.0000	111.60	111.60
AA13	AA12	36	8.04	8.14	14.5	11.31	11.31	0.015	364.00	6.07	116.50	111.56	111.42	4.94	0.0004	111.60	111.46
		N/A				0.00		0.015	364.00	0.00	116.50	111.56	111.42	N/A	0.0004	111.60	111.46
AA12	AA11	42	16.74	24.88	15.5	33.50	33.5	0.015	282.00	4.70	116.20	111.25	110.84	4.95	0.0015	111.46	111.05
		N/A				0.00		0.015	282.00	0.00	116.20	111.25	110.84	N/A	0.0015	111.46	111.05
AA11	AA10	42	3.49	28.37	16.5	36.40	36.4	0.015	307.00	5.12	115.00	110.80	110.26	4.20	0.0017	111.04	110.51
		N/A				0.00		0.015	307.00	0.00	115.00	110.80	110.26	N/A	0.0017	111.04	110.51
AA10	AA9	42	5.	33.37	17.5	40.88	40.88	0.015	297.00	4.95	113.50	110.20	109.54	3.30	0.0022	110.51	109.86
		N/A				0.00		0.015	297.00	0.00	113.50	110.20	109.54	N/A	0.0022	110.51	109.86
AA9	AA2	42	4.35	37.72	18.5	44.37	44.37	0.015	228.00	4.61	111.50	109.49	108.90	2.01	0.0026	109.85	109.26
		N/A				0.00		0.015	228.00	0.00	111.50	109.49	108.90	N/A	0.0026	109.85	109.26
AA8	AA7	36	13.67	13.67	13.5	19.89	19.89	0.015	268.00	4.47	118.10	111.69	111.37	6.41	0.0012	111.83	111.51
		N/A				0.00		0.015	268.00	0.00	118.10	111.69	111.37	N/A	0.0012	111.83	111.51
AA7	AA6	36	0.1	13.77	14.5	19.29	19.29	0.015	282.00	4.70	117.00	111.38	111.06	5.62	0.0011	111.51	111.19
		N/A				0.00		0.015	282.00	0.00	117.00	111.38	111.06	N/A	0.0011	111.51	111.19
AA6	AA5	36	4.03	17.8	15.5	23.43	23.43	0.015	301.00	5.02	116.00	111.00	110.51	5.00	0.0016	111.19	110.70
		N/A				0.00		0.015	301.00	0.00	116.00	111.00	110.51	N/A	0.0016	111.19	110.70
AA5	AA4	36	4.87	22.67	16.5	28.34	28.34	0.015	308.00	5.13	114.00	110.42	109.68	3.58	0.0024	110.69	109.95
		N/A				0.00		0.015	308.00	0.00	114.00	110.42	109.68	N/A	0.0024	110.69	109.95
AA4	AA3	42	5.28	27.95	17.5	33.38	33.38	0.015	218.00	3.63	111.50	109.75	109.43	1.75	0.0015	109.95	109.63
		N/A				0.00		0.015	218.00	0.00	111.50	109.75	109.43	N/A	0.0015	109.95	109.63
AA3	AA2	48	4.76	32.71	18.5	37.51	37.51	0.015	405.00	6.75	110.00	109.48	109.11	0.52	0.0009	109.63	109.26
		N/A				0.00		0.015	405.00	0.00	110.00	109.48	109.11	N/A	0.0009	109.63	109.26
AA2	AA1	48	3.52	73.95	19.32	83.36	83.36	0.015	112.00	6.63	109.50	108.50	108.00	1.00	0.0045	109.26	108.76
		N/A				0.00		0.015	112.00	0.00	109.50	108.50	108.00	N/A	0.0045	109.26	108.76
B4	B3	24	0.01	0.01	13.5	0.01	0.01	0.015	316.00	5.27	102.00	89.77	89.76	12.23	0.0000	89.77	89.76
		N/A				0.00		0.015	316.00	0.00	102.00	89.77	89.76	N/A	0.0000	89.77	89.76
B3	B2	30	9.89	9.9	14.5	13.87	13.87	0.015	317.00	5.28	102.00	89.62	89.14	12.38	0.0015	89.76	89.27
		N/A				0.00		0.015	317.00	0.00	102.00	89.62	89.14	N/A	0.0015	89.76	89.27
B2	B1	30	9.65	19.55	15.5	26.36	26.36	0.015	104.00	6.31	101.00	88.59	87.75	12.41	0.0061	89.27	88.64
		N/A				0.00		0.015	104.00	0.00	101.00	88.59	87.75	N/A	0.0061	89.27	88.64
BB2	BB1	48	17.81	17.81	13.5	25.42	27.6	0.015	87.00	4.36	105.00	105.02	104.49	-0.02	0.0025	105.32	105.10
		N/A				2.19		0.015	87.00	1.45	105.00	105.27	103.50	-0.27	0.0209	105.32	103.50
CC11	CC12	24	3.33	3.33	15.	4.88	4.88	0.015	310.00	5.17	110.50	103.19	103.01	7.31	0.0006	103.25	103.06
		N/A				0.00		0.015	310.00	0.00	110.50	103.19	103.01	N/A	0.0006	103.25	103.06
CC12	CC13	24	0.01	3.34	16.	4.74	4.74	0.015	260.00	4.33	107.60	103.01	102.89	4.59	0.0005	103.06	102.93
		N/A				0.00		0.015	260.00	0.00	107.60	103.01	102.89	N/A	0.0005	103.06	102.93
CC13	CC14	24	2.91	6.25	17.	8.60	8.6	0.015	260.00	4.33	106.50	102.78	102.31	3.72	0.0019	102.93	102.44
		N/A				0.00		0.015	260.00	0.00	106.50	102.78	102.31	N/A	0.0019	102.93	102.44
CC14	CC1-1	30	0.01	6.26	18.	8.37	8.37	0.015	63.00	1.70	106.00	102.39	102.35	3.61	0.0006	102.44	102.40
		N/A				0.00		0.015	918.00	0.00	106.00	102.39	102.35	N/A	0.0000	102.44	102.40
CC4	CC3	54	10.6	65.03	19.8	78.10	78.1	0.015	206.00	4.91	107.80	107.23	106.80	0.57	0.0021	107.65	107.21
		N/A				0.00		0.015	206.00	0.00	107.80	107.23	106.80	N/A	0.0021	107.65	107.21
CC3	CC2	60	4.7	69.73	20.5	78.91	81.8	0.015	109.00	4.02	106.50	106.57	106.43	-0.07	0.0012	106.84	106.71
		N/A				2.89		0.015	109.00	1.82	106.50	106.80	106.71	-0.30	0.0013	106.85	106.71
CC5	CC4	54	3.58	54.43	18.87	68.72	68.72	0.015	242.00	4.32	110.30	107.72	107.33	2.58	0.0016	108.04	107.65
		N/A				0.00		0.015	242.00	0.00	110.30	107.72	107.33	N/A	0.0016	108.04	107.65
CC6	CC5	48	11.71	50.85	18.	65.77	65.77	0.015	272.00	5.23	112.40	108.33	107.57	4.07	0.0028	108.80	108.04
		N/A				0.00		0.015	272.00	0.00	112.40	108.33	107.57	N/A	0.0028	108.80	108.04
CC7	CC6	48	16.14	39.14	17.	52.03	52.03	0.015	307.00	5.12	115.20	109.05	108.51	6.15	0.0017	109.34	108.80
		N/A				0.00		0.015	307.00	0.00	115.20	109.05	108.51	N/A	0.0017	109.34	108.80
CC8	CC7	48	0.1	23.	16.	32.37	32.37	0.015	400.00	6.67	117.10	109.50	109.23	7.60	0.0007	109.61	109.34
		N/A				0.00		0.015	400.00	0.00	117.10	109.50	109.23	N/A	0.0007	109.61	109.34
CC10	CC8	48	9.17	9.17	15.	13.17	13.17	0.015	354.00	5.90	118.10	109.64	109.60	8.47	0.0001	109.65	109.61
		N/A				0.00		0.015	354.00	0.00	118.10	109.64	109.60	N/A	0.0001	109.65	109.61
CC9	CC8	48	13.73	13.73	15.	20.14	20.14	0.015	374.00	6.23	116.30	109.67	109.57	6.63	0.0003	109.71	109.61
		N/A				0.00		0.015	374.00	0.00	116.30	109.67	109.57	N/A	0.0003	109.71	109.61
CC2	CC1	66	0.01	69.74	20.93	54.47	80.19	0.015	337.00	5.62	106.00	106.62	106.50	-0.62	0.0003	106.71	106.59
		N/A				25.72		0.015	337.00	5.62	106.00	106.64	106.50	-0.64	0.0030	106.71	106.59
DD3	DD2	18	0.1	0.1	13.5	0.16	0.16	0.015	170.00	2.83	107.00	102.12	102.12	4.88	0.0000	102.12	102.12

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	170.00	0.00	107.00	102.12	102.12	N/A	0.0000	102.12	102.12
DD2	DD1	18	3.77	3.87	14.5	4.93	4.93	0.015	40.00	2.79	106.00	101.99	101.87	4.01	0.0029	102.12	102.00
		N/A				0.00		0.015	40.00	0.00	106.00	101.99	101.87	N/A	0.0029	102.12	102.00
GG4	GG3	24	0.27	0.27	0.01	15.65	15.65	0.015	380.00	6.33	106.00	105.25	102.83	0.75	0.0064	105.68	103.26
		N/A				0.00		0.015	380.00	0.00	106.00	105.25	102.83	N/A	0.0064	105.68	103.26
GG3	GG2	24	1.22	1.49	13.5	1.98	1.98	0.015	310.00	5.17	105.00	102.13	102.11	2.87	0.0001	102.14	102.11
		N/A				0.00		0.015	310.00	0.00	105.00	102.13	102.11	N/A	0.0001	102.14	102.11
GG2	GG1	24	0.99	2.48	14.5	3.05	3.05	0.015	45.00	0.97	104.00	102.10	102.09	1.90	0.0002	102.11	102.10
		N/A				0.00		0.015	45.00	0.00	104.00	102.10	102.09	N/A	0.0002	102.11	102.10
H5A	H4A	24	9.77	9.77	13.5	12.94	12.94	0.015	432.00	7.20	98.00	91.88	90.00	6.12	0.0044	92.17	90.29
		N/A				0.00		0.015	432.00	0.00	98.00	91.88	90.00	N/A	0.0044	92.17	90.29
H4A	H3A	24	9.11	18.88	14.5	23.81	23.81	0.015	416.00	7.58	94.00	89.30	83.16	4.70	0.0147	90.29	84.15
		N/A				0.00		0.015	416.00	0.00	94.00	89.30	83.16	N/A	0.0147	90.29	84.15
H3A	H2A	36	0.01	18.89	15.41	22.96	22.96	0.015	286.00	4.77	89.00	83.97	83.52	5.03	0.0016	84.15	83.70
		N/A				0.00		0.015	286.00	0.00	89.00	83.97	83.52	N/A	0.0016	84.15	83.70
H2A	H1A	36	11.07	29.96	16.41	35.12	35.12	0.015	73.00	4.97	87.00	83.27	83.00	3.73	0.0037	83.70	83.43
		N/A				0.00		0.015	73.00	0.00	87.00	83.27	83.00	N/A	0.0037	83.70	83.43
H16B	H15B	30	4.42	4.42	15.	5.68	5.68	0.015	340.00	5.67	101.00	93.74	93.52	7.26	0.0007	93.82	93.58
		N/A				0.00		0.015	340.00	0.00	101.00	93.74	93.52	N/A	0.0007	93.82	93.58
H15B	H14B	30	0.3	4.72	16.	5.90	5.9	0.015	352.00	5.87	101.00	93.51	93.35	7.49	0.0005	93.58	93.40
		N/A				0.00		0.015	352.00	0.00	101.00	93.51	93.35	N/A	0.0005	93.58	93.40
H14B	H13B	36	2.44	7.16	20.	7.95	7.95	0.015	340.00	5.67	101.00	93.36	93.30	7.64	0.0002	93.40	93.33
		N/A				0.00		0.015	340.00	0.00	101.00	93.36	93.30	N/A	0.0002	93.40	93.33
H13B	H12B	36	0.3	7.46	21.	7.93	7.93	0.015	398.00	6.63	101.00	93.30	93.23	7.70	0.0002	93.33	93.25
		N/A				0.00		0.015	398.00	0.00	101.00	93.30	93.23	N/A	0.0002	93.33	93.25
H12B	H11B	36	0.3	7.76	22.	7.92	7.92	0.015	467.00	7.78	100.70	93.23	93.14	7.47	0.0002	93.25	93.16
		N/A				0.00		0.015	467.00	0.00	100.70	93.23	93.14	N/A	0.0002	93.25	93.16
H11B	H10B	42	11.4	19.16	23.	19.29	19.29	0.015	402.00	6.70	102.00	93.09	92.89	8.91	0.0005	93.16	92.96
		N/A				0.00		0.015	402.00	0.00	102.00	93.09	92.89	N/A	0.0005	93.16	92.96
H10B	H6B	60	5.09	24.25	24.	23.48	23.48	0.015	493.00	8.22	100.20	92.94	92.88	7.26	0.0001	92.96	92.91
		N/A				0.00		0.015	493.00	0.00	100.20	92.94	92.88	N/A	0.0001	92.96	92.91
H9B	H8B	60	20.09	20.09	15.	24.79	24.79	0.015	387.00	6.45	99.30	93.41	93.36	5.89	0.0001	93.44	93.39
		N/A				0.00		0.015	387.00	0.00	99.30	93.41	93.36	N/A	0.0001	93.44	93.39
H8B	H7B	60	16.88	36.97	16.	43.97	43.97	0.015	326.00	5.43	98.50	93.30	93.18	5.20	0.0004	93.39	93.26
		N/A				0.00		0.015	326.00	0.00	98.50	93.30	93.18	N/A	0.0004	93.39	93.26
H7B	H6B	60	24.21	61.18	17.	70.74	70.74	0.015	359.00	5.98	98.00	93.04	92.69	4.96	0.0010	93.26	92.91
		N/A				0.00		0.015	359.00	0.00	98.00	93.04	92.69	N/A	0.0010	93.26	92.91
H6B	H5B	66	5.9	91.33	18.	104.99	104.99	0.015	428.00	7.13	97.30	92.57	92.01	4.73	0.0013	92.91	92.35
		N/A				0.00		0.015	359.00	0.00	97.30	92.57	92.01	N/A	0.0016	92.91	92.35
H5B	H4B	66	0.01	91.34	19.	101.67	101.67	0.015	368.00	6.13	95.00	92.03	91.58	2.97	0.0012	92.35	91.90
		N/A				0.00		0.015	368.00	0.00	95.00	92.03	91.58	N/A	0.0012	92.35	91.90
H4B	H3B	66	5.94	97.28	20.	107.28	107.28	0.015	414.00	6.90	93.00	91.55	90.98	1.45	0.0014	91.90	91.34
		N/A				0.00		0.015	414.00	0.00	93.00	91.55	90.98	N/A	0.0014	91.90	91.34
H3B	H2B	66	8.18	105.46	21.	111.30	112.04	0.015	169.00	4.68	91.00	90.96	90.71	0.04	0.0015	91.33	91.09
		N/A				0.74		0.015	169.00	2.82	91.00	91.33	91.19	-0.33	0.0006	91.33	91.23
H2B	H1B	72	3.95	109.41	21.6	55.05	113.06	0.015	80.00	1.95	89.90	91.02	91.00	-1.12	0.0002	91.08	91.07
		N/A				58.01		0.015	80.00	3.52	89.90	90.63	89.73	-0.73	0.0113	90.82	89.92
HH4	HH3	24	5.3	5.3	10.	9.56	9.56	0.015	352.00	5.87	102.50	97.23	96.39	5.27	0.0024	97.39	96.55
		N/A				0.00		0.015	352.00	0.00	102.50	97.23	96.39	N/A	0.0024	97.39	96.55
HH3	HH2	24	3.7	9.	11.	15.46	15.46	0.015	417.00	6.95	101.00	96.13	93.54	4.87	0.0062	96.55	93.96
		N/A				0.00		0.015	417.00	0.00	101.00	96.13	93.54	N/A	0.0062	96.55	93.96
HH2	HH1	30	9.23	18.23	15.	26.74	26.74	0.015	186.00	5.45	99.50	93.44	92.39	6.06	0.0057	93.95	92.90
		N/A				0.00		0.015	186.00	0.00	99.50	93.44	92.39	N/A	0.0057	93.95	92.90
JJ17	JJ9	18	0.3	0.3	15.	0.45	0.45	0.015	470.00	7.83	109.00	95.34	95.33	13.66	0.0000	95.34	95.33
		N/A				0.00		0.015	470.00	0.00	109.00	95.34	95.33	N/A	0.0000	95.34	95.33
JJ16	JJ15	36	11.33	11.33	15.	15.13	15.13	0.015	550.00	9.17	110.10	98.93	98.56	11.17	0.0007	99.01	98.63
		N/A				0.00		0.015	550.00	0.00	110.10	98.93	98.56	N/A	0.0007	99.01	98.63
JJ15	JJ14	42	12.6	23.93	16.	30.18	30.18	0.015	350.00	5.83	106.50	98.46	98.04	8.04	0.0012	98.63	98.21
		N/A				0.00		0.015	350.00	0.00	106.50	98.46	98.04	N/A	0.0012	98.63	98.21
JJ14	JJ12	42	10.98	34.91	17.	40.16	40.16	0.015	110.00	4.17	105.50	97.91	97.68	7.59	0.0021	98.21	97.98
		N/A				0.00		0.015	110.00	0.00	105.50	97.91	97.68	N/A	0.0021	98.21	97.98
JJ13	JJ12	48	9.5	9.5	15.	11.80	11.8	0.015	550.00	9.17	109.20	98.01	97.96	11.19	0.0001	98.03	97.98

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		15				0.00		0.015	550.00	0.00	109.20	98.01	97.96	N/A	0.0001	98.03	97.98
JJ12	JJ11	48	9.16	53.57	17.44	60.70	60.7	0.015	335.00	5.58	105.00	97.57	96.78	7.43	0.0024	97.98	97.18
		N/A				0.00		0.015	335.00	0.00	105.00	97.57	96.78	N/A	0.0024	97.98	97.18
JJ11	JJ10	48	10.67	64.24	18.44	70.34	70.34	0.015	330.00	5.60	104.00	96.64	95.59	7.36	0.0032	97.18	96.12
		N/A				0.00		0.015	330.00	0.00	104.00	96.64	95.59	N/A	0.0032	97.18	96.12
JJ10	JJ9	60	10.17	74.41	19.42	78.91	78.91	0.015	650.00	10.83	103.20	95.84	95.05	7.36	0.0012	96.12	95.33
		N/A				0.00		0.015	650.00	0.00	103.20	95.84	95.05	N/A	0.0012	96.12	95.33
JJ9	JJ8	60	11.73	86.44	20.42	89.51	89.51	0.015	350.00	5.83	101.50	94.97	94.42	6.53	0.0016	95.33	94.78
		N/A				0.00		0.015	350.00	0.00	101.50	94.97	94.42	N/A	0.0016	95.33	94.78
JJ8	JJ7	66	9.6	96.04	21.42	95.38	95.38	0.015	335.00	5.58	100.50	94.50	94.14	6.00	0.0011	94.77	94.41
		N/A				0.00		0.015	335.00	0.00	100.50	94.50	94.14	N/A	0.0011	94.77	94.41
JJ7	JJ6	66	4.2	100.24	22.42	95.62	95.62	0.015	370.00	6.17	99.50	94.13	93.73	5.37	0.0011	94.41	94.01
		N/A				0.00		0.015	370.00	0.00	99.50	94.13	93.73	N/A	0.0011	94.41	94.01
JJ6	JJ5	66	0.	100.24	23.42	91.18	91.18	0.015	425.00	7.08	98.50	93.76	93.34	4.74	0.0010	94.01	93.59
		N/A				0.00		0.015	425.00	0.00	98.50	93.76	93.34	N/A	0.0010	94.01	93.59
JJ5	JJ4	66	4.9	105.14	24.42	92.34	92.34	0.015	510.00	8.50	97.50	93.33	92.82	4.17	0.0010	93.59	93.08
		N/A				0.00		0.015	510.00	0.00	97.50	93.33	92.82	N/A	0.0010	93.59	93.08
JJ4	JJ3	72	11.04	116.18	25.42	99.79	99.79	0.015	410.00	6.83	96.00	92.86	92.56	3.14	0.0007	93.08	92.77
		N/A				0.00		0.015	410.00	0.00	96.00	92.86	92.56	N/A	0.0007	93.08	92.77
JJ3	JJ2	72	10.43	126.61	26.42	106.13	106.13	0.015	480.00	8.00	94.40	92.53	92.13	1.87	0.0008	92.77	92.37
		N/A				0.00		0.015	480.00	0.00	94.40	92.53	92.13	N/A	0.0008	92.77	92.37
JJ2	JJ1	72	14.81	141.42	27.42	112.73	116.31	0.015	100.00	3.99	92.00	92.10	92.00	-0.10	0.0009	92.37	92.28
		N/A				3.58		0.015	100.00	3.19	92.00	92.31	91.50	-0.31	0.0086	92.37	91.51
L2	L1	60	6.38	88.65	27.67	66.38	66.38	0.015	250.00	4.17	98.00	96.22	96.00	1.78	0.0009	96.41	96.20
		N/A				0.00		0.015	250.00	0.00	98.00	96.22	96.00	N/A	0.0009	96.41	96.20
L3	L2	54	7.86	82.27	26.67	64.04	64.04	0.015	350.00	5.83	100.00	96.63	96.14	3.37	0.0014	96.91	96.41
		N/A				0.00		0.015	350.00	0.00	100.00	96.63	96.14	N/A	0.0014	96.91	96.41
L4	L3	54	7.26	7.26	20.	7.74	7.74	0.015	330.00	5.50	101.00	96.91	96.91	4.09	0.0000	96.91	96.91
		N/A				0.00		0.015	330.00	0.00	101.00	96.91	96.91	N/A	0.0000	96.91	96.91
L5	L3	48	15.41	67.15	25.67	54.48	54.48	0.015	480.00	8.00	103.00	97.51	96.59	5.49	0.0019	97.83	96.91
		N/A				0.00		0.015	480.00	0.00	103.00	97.51	96.59	N/A	0.0019	97.83	96.91
L6	L5	48	10.14	51.74	24.67	44.04	44.04	0.015	340.00	5.67	106.00	97.77	97.61	8.23	0.0017	98.39	97.83
		N/A				0.00		0.015	340.00	0.00	106.00	97.77	97.61	N/A	0.0017	98.39	97.83
L7	L6	42	4.88	41.6	23.95	36.71	36.71	0.015	310.00	7.20	108.00	99.44	98.08	8.56	0.0050	100.28	98.74
		N/A				0.00		0.015	310.00	0.00	108.00	99.44	98.08	N/A	0.0050	100.28	98.74
L8	L7	24	5.6	5.6	20.	5.93	5.93	0.015	415.00	6.92	110.00	102.50	100.42	7.50	0.0050	102.86	100.78
		N/A				0.00		0.015	415.00	0.00	110.00	102.50	100.42	N/A	0.0050	102.86	100.78
L9	L7	36	9.8	31.12	22.95	28.94	28.94	0.015	525.00	8.75	110.00	102.43	99.70	7.57	0.0050	103.23	100.62
		N/A				0.00		0.015	525.00	0.00	110.00	102.43	99.70	N/A	0.0050	103.23	100.62
L10	L9	36	7.11	21.32	21.95	21.00	21.	0.015	490.00	8.17	112.00	105.11	102.96	6.89	0.0049	105.75	103.36
		N/A				0.00		0.015	490.00	0.00	112.00	105.11	102.96	N/A	0.0049	105.75	103.36
L11	L10	30	0.3	14.21	21.	14.87	14.87	0.015	305.00	5.34	113.10	106.96	105.44	6.14	0.0050	107.53	106.01
		N/A				0.00		0.015	305.00	0.00	113.10	106.96	105.44	N/A	0.0050	107.53	106.01
L12	L11	30	7.5	7.5	20.	8.26	8.26	0.015	650.00	10.83	113.90	110.37	107.27	3.53	0.0050	110.76	107.53
		N/A				0.00		0.015	650.00	0.00	113.90	110.37	107.27	N/A	0.0050	110.76	107.53
L13	L11	24	6.41	6.41	20.	7.06	7.06	0.015	550.00	9.17	115.00	109.85	107.26	5.15	0.0050	110.26	107.53
		N/A				0.00		0.015	550.00	0.00	115.00	109.85	107.26	N/A	0.0050	110.26	107.53
LL12	LL5	24	7.52	7.52	15.	10.36	10.36	0.015	410.00	6.83	100.50	98.68	97.54	1.82	0.0028	98.87	97.73
		N/A				0.00		0.015	410.00	0.00	100.50	98.68	97.54	N/A	0.0028	98.87	97.73
LL11	LL10	24	7.22	7.22	15.	0.49	9.79	0.015	470.00	7.83	107.00	107.41	106.49	-0.41	0.0021	107.50	106.49
		N/A				9.29		0.015	470.00	7.83	107.00	107.46	106.41	-0.46	0.0022	107.51	106.49
LL10	LL9	24	7.46	14.68	15.99	10.22	19.4	0.015	430.00	7.17	106.00	106.22	104.54	-0.22	0.0033	106.49	105.06
		N/A				9.18		0.015	430.00	7.17	106.00	106.41	104.12	-0.41	0.0055	106.49	104.13
LL9	LL8	36	6.56	21.23	16.99	7.83	27.3	0.015	400.00	6.67	103.50	103.85	102.37	-0.35	0.0037	104.13	102.66
		N/A				19.47		0.015	400.00	6.67	103.50	104.02	102.21	-0.52	0.0047	104.13	102.23
LL8	LL7	42	5.64	26.87	17.7	3.82	33.82	0.015	450.00	7.50	101.50	102.01	100.23	-0.51	0.0044	102.23	100.27
		N/A				29.99		0.015	450.00	7.50	101.50	102.14	100.21	-0.64	0.0044	102.23	100.27
LL7	LL6	42	8.62	35.49	18.59	7.75	43.51	0.015	350.00	5.83	99.50	99.94	98.21	-0.44	0.0057	100.27	98.28
		N/A				35.76		0.015	350.00	5.83	99.50	100.15	98.19	-0.65	0.0057	100.27	98.28
LL6	LL5	42	6.1	41.59	19.41	13.55	49.61	0.015	130.00	4.35	97.50	97.83	97.63	-0.33	0.0043	98.28	97.73
		N/A				36.05		0.015	130.00	2.34	97.50	98.19	97.63	-0.69	0.0039	98.28	97.77
LL5	LL4	42	6.01	55.12	20.08	35.46	65.44	0.015	340.00	8.59	97.00	96.91	94.61	0.09	0.0081	97.72	94.96

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				29.98		0.015	340.00	5.67	97.00	97.59	94.80	-0.59	0.0085	97.72	94.83
LL4	LL3	48	10.87	65.98	20.9	43.03	76.29	0.015	295.00	9.79	94.00	93.90	90.19	0.10	0.0103	94.75	91.71
		N/A				33.26		0.015	295.00	4.92	94.00	94.61	91.85	-0.61	0.0097	94.75	91.88
LL3	LL2	54	8.8	74.78	21.62	55.56	84.32	0.015	205.00	10.00	91.00	90.78	89.38	0.22	0.0099	91.71	89.68
		N/A				28.76		0.015	205.00	3.42	91.00	91.59	89.53	-0.59	0.0096	91.71	89.74
LL2	LL1	54	11.85	86.62	21.84	88.53	96.16	0.015	100.00	7.89	89.00	88.51	87.75	0.49	0.0041	89.47	89.05
		N/A				7.63		0.015	100.00	3.77	89.00	89.38	88.60	-0.38	0.0086	89.46	88.61
M4A	M3A	30	7.16	7.16	15.	8.52	8.52	0.015	305.00	5.08	103.00	95.46	95.01	7.54	0.0014	95.60	95.17
		N/A				0.00		0.015	305.00	0.00	103.00	95.46	95.01	N/A	0.0014	95.60	95.17
M3A	M2A	42	0.3	16.86	16.	19.85	19.85	0.015	358.00	5.97	102.40	95.03	94.76	7.37	0.0007	95.16	94.90
		N/A				0.00		0.015	358.00	0.00	102.40	95.03	94.76	N/A	0.0007	95.16	94.90
M5A	M3A	30	9.4	9.4	15.	11.69	11.69	0.015	388.00	6.47	106.00	95.85	95.03	10.15	0.0018	96.02	95.32
		N/A				0.00		0.015	388.00	0.00	106.00	95.85	95.03	N/A	0.0018	96.02	95.32
M2A	M1A	42	4.38	21.24	17.	23.93	23.93	0.015	445.00	7.42	102.00	94.68	93.50	7.32	0.0017	94.89	94.14
		N/A				0.00		0.015	445.00	0.00	102.00	94.68	93.50	N/A	0.0017	94.89	94.14
M10B	M3B	24	0.3	0.3	15.	0.45	0.45	0.015	305.00	5.08	110.00	105.03	105.04	4.97	0.0000	105.03	105.04
		N/A				0.00		0.015	305.00	0.00	110.00	105.03	105.04	N/A	0.0000	105.03	105.04
M9B	M8B	36	10.6	10.6	20.	12.55	12.55	0.015	358.00	5.97	115.00	106.53	106.36	8.47	0.0005	106.58	106.42
		N/A				0.00		0.015	358.00	0.00	115.00	106.53	106.36	N/A	0.0005	106.58	106.42
M8B	M7B	36	7.16	17.76	21.	19.80	19.8	0.015	388.00	6.47	112.50	106.28	105.82	6.22	0.0012	106.41	105.96
		N/A				0.00		0.015	388.00	0.00	112.50	106.28	105.82	N/A	0.0012	106.41	105.96
M7B	M3B	36	6.45	24.21	22.	26.22	26.22	0.015	445.00	7.42	110.00	105.72	104.80	4.28	0.0021	105.96	105.04
		N/A				0.00		0.015	445.00	0.00	110.00	105.72	104.80	N/A	0.0021	105.96	105.04
M6B	M5B	36	10.29	10.29	20.	11.41	11.41	0.015	417.00	6.95	109.40	105.58	105.42	3.82	0.0004	105.63	105.47
		N/A				0.00		0.015	417.00	0.00	109.40	105.58	105.42	N/A	0.0004	105.63	105.47
M5B	M4B	36	7.41	17.7	21.	13.75	17.72	0.015	450.00	7.50	104.00	105.40	105.14	-1.40	0.0006	105.46	105.21
		N/A				3.97		0.015	450.00	7.50	104.00	105.46	105.32	-1.46	0.0002	105.46	105.38
M4B	M3B	48	9.37	27.07	21.78	25.07	25.07	0.015	420.00	7.00	105.00	105.13	104.97	-0.13	0.0004	105.21	105.04
		N/A				0.00		0.015	420.00	0.00	105.00	105.13	104.97	N/A	0.0004	105.21	105.04
M3B	M2B	54	2.29	53.87	22.78	51.83	51.83	0.015	330.00	5.50	108.30	104.84	104.58	3.46	0.0008	105.04	104.77
		N/A				0.00		0.015	330.00	0.00	108.30	104.84	104.58	N/A	0.0008	105.04	104.77
M2B	M1B	54	0.01	53.88	23.78	49.52	49.52	0.015	114.00	3.11	107.00	104.60	104.50	2.40	0.0008	104.77	104.67
		N/A				0.00		0.015	114.00	0.00	107.00	104.60	104.50	N/A	0.0008	104.77	104.67
M2C	M1C	18	3.67	3.67	15.	4.52	4.52	0.015	384.00	6.40	108.50	106.13	103.82	2.37	0.0060	106.50	104.18
		N/A				0.00		0.015	384.00	0.00	108.50	106.13	103.82	N/A	0.0060	106.50	104.18
M4D	M2D	24	7.25	7.25	15.	9.04	9.04	0.015	384.00	6.40	110.00	108.00	107.18	2.00	0.0021	108.14	107.32
		N/A				0.00		0.015	384.00	0.00	110.00	108.00	107.18	N/A	0.0021	108.14	107.32
M3D	M2D	30	4.52	4.52	15.	5.65	5.65	0.015	222.00	3.70	111.00	106.32	106.19	4.68	0.0006	106.39	106.25
		N/A				0.00		0.015	222.00	0.00	111.00	106.32	106.19	N/A	0.0006	106.39	106.25
M2D	M1D	36	0.3	12.07	16.	14.49	14.49	0.015	192.00	3.20	108.00	106.08	105.82	1.92	0.0013	106.24	106.00
		N/A				0.00		0.015	192.00	0.00	108.00	106.08	105.82	N/A	0.0013	106.24	106.00
M6E	M2E	36	49.9	49.9	15.	62.00	62.	0.015	384.00	8.77	118.00	111.63	107.22	6.37	0.0115	112.96	108.54
		N/A				0.00		0.015	384.00	0.00	118.00	111.63	107.22	N/A	0.0115	112.96	108.54
M4E	M3E	24	3.05	3.05	15.	3.81	3.81	0.015	222.00	3.70	114.00	108.63	108.54	5.37	0.0004	108.65	108.57
		N/A				0.00		0.015	222.00	0.00	114.00	108.63	108.54	N/A	0.0004	108.65	108.57
M3E	M2E	36	2.55	5.6	16.	6.74	6.74	0.015	192.00	3.20	111.00	108.55	108.53	2.45	0.0001	108.57	108.54
		N/A				0.00		0.015	192.00	0.00	111.00	108.55	108.53	N/A	0.0001	108.57	108.54
M5E	M2E	24	2.68	2.68	15.	3.43	3.43	0.015	280.00	4.67	111.90	108.61	108.52	3.29	0.0003	108.63	108.54
		N/A				0.00		0.015	280.00	0.00	111.90	108.61	108.52	N/A	0.0003	108.63	108.54
M2E	M1E	42	0.3	58.48	15.73	70.76	70.76	0.015	112.00	7.35	108.00	107.61	106.87	0.39	0.0066	108.54	107.80
		N/A				0.00		0.015	112.00	0.00	108.00	107.61	106.87	N/A	0.0066	108.54	107.80
M7F	M6F	36	2.87	2.87	20.	3.17	3.17	0.015	625.00	10.42	121.00	116.32	116.28	4.68	0.0001	116.32	116.28
		N/A				0.00		0.015	625.00	0.00	121.00	116.32	116.28	N/A	0.0001	116.32	116.28
M6F	M4F	36	6.48	9.35	21.	10.65	10.65	0.015	436.00	7.27	120.20	116.24	116.09	3.96	0.0003	116.28	116.13
		N/A				0.00		0.015	436.00	0.00	120.20	116.24	116.09	N/A	0.0003	116.28	116.13
M5F	M4F	42	10.79	10.79	20.	11.09	11.09	0.015	297.00	4.95	119.50	116.15	116.11	3.35	0.0002	116.18	116.13
		N/A				0.00		0.015	297.00	0.00	119.50	116.15	116.11	N/A	0.0002	116.18	116.13
M4F	M3F	42	6.98	27.12	21.	27.75	27.75	0.015	500.00	8.33	119.00	115.99	115.48	3.01	0.0010	116.13	115.62
		N/A				0.00		0.015	500.00	0.00	119.00	115.99	115.48	N/A	0.0010	116.13	115.62
M3F	M2F	48	11.48	38.6	22.	37.08	37.08	0.015	495.00	8.25	117.00	115.47	115.03	1.53	0.0009	115.62	115.18
		N/A				0.00		0.015	495.00	0.00	117.00	115.47	115.03	N/A	0.0009	115.62	115.18
M8F	M2F	36	2.56	2.56	20.	2.73	2.73	0.015	242.00	4.03	115.50	114.58	114.57	0.92	0.0001	114.59	114.57

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	242.00	0.00	115.50	114.58	114.57	N/A	0.0001	114.59	114.57
M2F	M1F	48	10.69	51.85	23.	47.41	47.41	0.015	82.00	10.41	115.00	113.66	111.51	1.34	0.0121	114.57	113.57
		N/A				0.00		0.015	82.00	0.00	115.00	113.66	111.51	N/A	0.0121	114.57	113.57
M2G	M1G	36	8.02	8.02	15.	0.33	8.78	0.015	92.00	1.53	114.00	114.45	114.18	-0.45	0.0025	114.48	114.25
		N/A				8.45		0.015	92.00	3.66	114.00	114.40	113.63	-0.40	0.0091	114.48	113.64
M2H	M1H	24	4.99	4.99	15.	4.15	6.84	0.015	80.00	3.61	112.00	112.05	111.72	-0.05	0.0041	112.34	112.01
		N/A				2.69		0.015	80.00	2.62	112.00	112.28	111.51	-0.28	0.0103	112.34	111.51
MM2	MM1	18	4.22	4.22	13.5	4.97	4.97	0.015	120.00	2.81	90.00	86.02	85.67	3.98	0.0030	86.16	85.80
		N/A				0.00		0.015	120.00	0.00	90.00	86.02	85.67	N/A	0.0030	86.16	85.80
N6A	N5A	18	3.64	3.64	15.	4.77	4.77	0.015	433.00	7.22	104.00	97.98	96.79	6.02	0.0027	98.10	96.91
		N/A				0.00		0.015	433.00	0.00	104.00	97.98	96.79	N/A	0.0027	98.10	96.91
N5A	N4A	36	13.64	17.28	16.	22.24	22.24	0.015	471.00	7.85	102.00	96.74	96.04	5.26	0.0015	96.91	96.21
		N/A				0.00		0.015	471.00	0.00	102.00	96.74	96.04	N/A	0.0015	96.91	96.21
N4A	N3A	42	6.84	29.32	17.	37.62	37.62	0.015	600.00	10.00	100.20	95.95	94.83	4.25	0.0019	96.21	95.10
		N/A				0.00		0.015	600.00	0.00	100.20	95.95	94.83	N/A	0.0019	96.21	95.10
N3A	N2A	48	5.98	35.3	18.	43.36	43.36	0.015	269.00	4.48	98.00	94.89	94.56	3.11	0.0012	95.09	94.77
		N/A				0.00		0.015	269.00	0.00	98.00	94.89	94.56	N/A	0.0012	95.09	94.77
N7A	N4A	24	5.2	5.2	15.	7.53	7.53	0.015	638.00	10.63	106.00	97.06	96.11	8.94	0.0015	97.15	96.21
		N/A				0.00		0.015	638.00	0.00	106.00	97.06	96.11	N/A	0.0015	97.15	96.21
N2A	N1A	48	1.42	36.72	19.	43.74	43.74	0.015	111.00	3.48	98.00	94.14	94.00	3.86	0.0012	94.35	94.21
		N/A				0.00		0.015	111.00	0.00	98.00	94.14	94.00	N/A	0.0012	94.35	94.21
N3B	N2B	18	2.94	2.94	15.	4.08	4.08	0.015	266.00	4.43	108.00	101.37	100.41	6.63	0.0029	101.52	100.74
		N/A				0.00		0.015	266.00	0.00	108.00	101.37	100.41	N/A	0.0029	101.52	100.74
N2B	N1B	24	1.9	4.84	16.	6.23	6.23	0.015	82.00	3.25	106.70	100.34	99.88	6.36	0.0031	100.51	100.26
		N/A				0.00		0.015	82.00	0.00	106.70	100.34	99.88	N/A	0.0031	100.51	100.26
N8C	N2C	24	5.76	5.76	13.5	7.63	7.63	0.015	266.00	4.43	108.00	101.94	101.11	6.06	0.0024	102.10	101.46
		N/A				0.00		0.015	266.00	0.00	108.00	101.94	101.11	N/A	0.0024	102.10	101.46
N7C	N6C	18	0.1	0.1	13.5	0.16	0.16	0.015	196.00	3.27	102.00	101.61	101.34	0.39	0.0014	101.62	101.35
		N/A				0.00		0.015	196.00	0.00	102.00	101.61	101.34	N/A	0.0014	101.62	101.35
N5C	N6C	24	2.61	2.61	13.5	3.38	3.38	0.015	105.00	2.52	112.00	101.63	101.32	10.37	0.0021	101.73	101.50
		N/A				0.00		0.015	105.00	0.00	112.00	101.63	101.32	N/A	0.0021	101.73	101.50
N6C	N4C	30	0.1	2.81	14.2	3.58	3.58	0.015	192.00	3.20	109.00	101.31	101.24	7.69	0.0004	101.35	101.27
		N/A				0.00		0.015	192.00	0.00	109.00	101.31	101.24	N/A	0.0004	101.35	101.27
N4C	N3C	30	1.84	4.65	15.2	5.67	5.67	0.015	197.00	3.28	110.00	101.17	100.88	8.83	0.0013	101.27	101.01
		N/A				0.00		0.015	197.00	0.00	110.00	101.17	100.88	N/A	0.0013	101.27	101.01
N3C	N3C1	30	0.21	4.86	16.2	5.76	5.76	0.015	150.00	2.97	109.00	100.87	100.34	8.13	0.0023	101.01	100.66
		N/A				0.00		0.015	150.00	0.00	109.00	100.87	100.34	N/A	0.0023	101.01	100.66
N2C	N1C	36	1.96	7.72	14.5	9.78	9.78	0.015	79.00	3.30	108.00	100.41	99.99	7.59	0.0024	100.58	100.39
		N/A				0.00		0.015	79.00	0.00	108.00	100.41	99.99	N/A	0.0024	100.58	100.39
N2D	N1D	24	1.89	1.89	15.	2.34	2.34	0.015	109.00	2.28	105.00	101.91	101.53	3.09	0.0023	101.99	101.74
		N/A				0.00		0.015	109.00	0.00	105.00	101.91	101.53	N/A	0.0023	101.99	101.74
N5E	N4E	30	10.46	10.46	15.	13.00	13.	0.015	461.00	7.68	107.00	101.85	101.24	5.15	0.0013	101.97	101.36
		N/A				0.00		0.015	461.00	0.00	107.00	101.85	101.24	N/A	0.0013	101.97	101.36
N4E	N3E	30	7.06	17.52	16.	20.94	20.94	0.015	285.00	4.75	109.00	101.04	100.05	7.96	0.0035	101.36	100.37
		N/A				0.00		0.015	285.00	0.00	109.00	101.04	100.05	N/A	0.0035	101.36	100.37
N3E	N2E	30	0.3	17.82	17.	20.60	20.6	0.015	154.00	4.20	110.00	100.06	99.55	9.94	0.0034	100.37	99.85
		N/A				0.00		0.015	154.00	0.00	110.00	100.06	99.55	N/A	0.0034	100.37	99.85
N2E	N1E	36	0.01	17.83	17.61	20.17	20.17	0.015	119.00	2.85	109.00	99.71	99.56	9.29	0.0012	99.85	99.70
		N/A				0.00		0.015	119.00	0.00	109.00	99.71	99.56	N/A	0.0012	99.85	99.70
N2F	N1F	30	5.5	5.5	15.	6.83	6.83	0.015	105.00	3.09	109.00	95.33	94.87	13.67	0.0024	95.48	95.22
		N/A				0.00		0.015	105.00	0.00	109.00	95.33	94.87	N/A	0.0024	95.48	95.22
N2G	N1G	30	4.5	4.5	15.	5.59	5.59	0.015	83.00	2.90	101.00	97.17	96.79	3.83	0.0025	97.30	97.10
		N/A				0.00		0.015	83.00	0.00	101.00	97.17	96.79	N/A	0.0025	97.30	97.10
N2H	N1H	36	7.3	7.3	15.	9.07	9.07	0.015	108.00	3.26	105.00	101.41	101.02	3.59	0.0021	101.57	101.34
		N/A				0.00		0.015	108.00	0.00	105.00	101.41	101.02	N/A	0.0021	101.57	101.34
N2I	N1I	18	2.1	2.1	15.	2.61	2.61	0.015	90.00	2.68	110.00	103.03	102.61	6.97	0.0031	103.15	102.87
		N/A				0.00		0.015	90.00	0.00	110.00	103.03	102.61	N/A	0.0031	103.15	102.87
N2J	N1J	18	2.8	2.8	15.	3.48	3.48	0.015	77.00	1.97	104.00	99.75	99.64	4.25	0.0015	99.81	99.70
		N/A				0.00		0.015	77.00	0.00	104.00	99.75	99.64	N/A	0.0015	99.81	99.70
N2k	N1k	60	62.79	62.79	15.	78.02	78.02	0.015	120.00	3.97	105.00	99.57	99.43	5.43	0.0012	99.85	99.70
		N/A				0.00		0.015	120.00	0.00	105.00	99.57	99.43	N/A	0.0012	99.85	99.70
NN7	NN6	18	0.1	0.1	13.5	0.15	0.15	0.015	518.00	8.63	97.00	88.28	87.24	8.72	0.0021	88.34	87.24

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	518.00	0.00	97.00	88.28	87.24	N/A	0.0021	88.34	87.24
NN6	NN5	18	0.1	0.2	14.5	0.30	0.3	0.015	462.00	7.70	96.00	87.22	86.91	8.78	0.0007	87.24	86.91
		N/A				0.00		0.015	462.00	0.00	96.00	87.22	86.91	N/A	0.0007	87.24	86.91
NN5	NN4	18	0.1	0.3	15.5	0.43	0.43	0.015	402.00	6.70	94.00	86.91	86.90	7.09	0.0000	86.91	86.90
		N/A				0.00		0.015	402.00	0.00	94.00	86.91	86.90	N/A	0.0000	86.91	86.90
NN4	NN3	18	0.01	0.31	16.5	0.43	0.43	0.015	438.00	7.30	93.00	86.90	86.89	6.10	0.0000	86.90	86.89
		N/A				0.00		0.015	438.00	0.00	93.00	86.90	86.89	N/A	0.0000	86.90	86.89
NN3	NN2	24	5.71	6.02	17.5	8.17	8.17	0.015	400.00	6.67	91.50	86.78	86.08	4.72	0.0017	86.89	86.20
		N/A				0.00		0.015	400.00	0.00	91.50	86.78	86.08	N/A	0.0017	86.89	86.20
NN2	NN1	24	0.1	6.12	18.5	8.08	8.08	0.015	47.00	2.57	90.00	86.08	86.00	3.92	0.0017	86.20	86.12
		N/A				0.00		0.015	47.00	0.00	90.00	86.08	86.00	N/A	0.0017	86.20	86.12
OO2	OO1	30	6.91	6.91	13.5	9.14	9.14	0.015	105.00	1.86	90.00	85.81	85.74	4.19	0.0007	85.87	85.80
		N/A				0.00		0.015	105.00	0.00	90.00	85.81	85.74	N/A	0.0007	85.87	85.80
P5	P4	36	10.66	10.66	13.5	14.61	14.61	0.015	270.00	4.50	100.50	94.60	94.43	5.90	0.0006	94.68	94.51
		N/A				0.00		0.015	270.00	0.00	100.50	94.60	94.43	N/A	0.0006	94.68	94.51
P4	P3	36	8.47	19.13	14.5	25.70	25.7	0.015	298.00	4.97	99.00	94.28	93.69	4.72	0.0020	94.50	93.91
		N/A				0.00		0.015	298.00	0.00	99.00	94.28	93.69	N/A	0.0020	94.50	93.91
P3	P2	42	8.29	27.42	15.5	35.91	35.91	0.015	346.00	5.77	97.60	93.67	93.09	3.93	0.0017	93.91	93.33
		N/A				0.00		0.015	346.00	0.00	97.60	93.67	93.09	N/A	0.0017	93.91	93.33
P2	P1	48	9.54	36.96	16.5	47.02	47.02	0.015	57.00	3.74	95.00	93.08	93.00	1.92	0.0014	93.32	93.24
		N/A				0.00		0.015	57.00	0.00	95.00	93.08	93.00	N/A	0.0014	93.32	93.24
PP3	PP2	48	19.55	19.55	15.	28.64	28.64	0.015	259.00	4.32	89.00	86.18	86.04	2.82	0.0005	86.27	86.13
		N/A				0.00		0.015	259.00	0.00	89.00	86.18	86.04	N/A	0.0005	86.27	86.13
PP2	PP1	48	0.3	19.85	16.	28.14	28.14	0.015	85.00	2.24	88.00	86.05	86.00	1.95	0.0005	86.13	86.09
		N/A				0.00		0.015	85.00	0.00	88.00	86.05	86.00	N/A	0.0005	86.13	86.09
Q7	Q6	36	8.95	8.95	13.5	11.93	11.93	0.015	418.00	6.97	106.00	94.81	94.63	11.19	0.0004	94.86	94.68
		N/A				0.00		0.015	418.00	0.00	106.00	94.81	94.63	N/A	0.0004	94.86	94.68
Q6	Q4	36	9.69	18.64	14.5	23.87	23.87	0.015	612.00	10.20	103.50	94.48	93.44	9.02	0.0017	94.68	93.63
		N/A				0.00		0.015	612.00	0.00	103.50	94.48	93.44	N/A	0.0017	94.68	93.63
Q5	Q4	36	8.34	8.34	13.5	10.50	10.5	0.015	604.00	10.07	105.00	93.80	93.60	11.20	0.0003	93.83	93.63
		N/A				0.00		0.015	604.00	0.00	105.00	93.80	93.60	N/A	0.0003	93.83	93.63
Q4	Q3	42	7.04	34.02	15.5	41.41	41.41	0.015	425.00	7.08	100.30	93.31	92.36	6.99	0.0023	93.63	92.67
		N/A				0.00		0.015	425.00	0.00	100.30	93.31	92.36	N/A	0.0023	93.63	92.67
Q3	Q2	48	14.93	48.95	16.5	57.27	57.27	0.015	380.00	6.33	96.20	92.32	91.51	3.88	0.0021	92.67	91.87
		N/A				0.00		0.015	380.00	0.00	96.20	92.32	91.51	N/A	0.0021	92.67	91.87
Q2	Q1	54	3.1	52.05	17.5	58.88	58.88	0.015	108.00	3.70	92.00	91.63	91.50	0.37	0.0012	91.87	91.74
		N/A				0.00		0.015	108.00	0.00	92.00	91.63	91.50	N/A	0.0012	91.87	91.74
QQ3	QQ2	42	0.3	0.3	15.	0.35	0.35	0.015	450.00	7.50	88.00	86.02	86.02	1.98	0.0000	86.02	86.02
		N/A				0.00		0.015	450.00	0.00	88.00	86.02	86.02	N/A	0.0000	86.02	86.02
QQ2	QQ1	42	33.85	34.15	16.	36.37	36.37	0.015	156.00	3.78	88.00	85.77	85.50	2.23	0.0017	86.02	85.75
		N/A				0.00		0.015	156.00	0.00	88.00	85.77	85.50	N/A	0.0017	86.02	85.75
R2	R1	42	14.36	14.36	13.5	18.66	18.66	0.015	129.00	3.93	94.00	89.92	89.32	4.08	0.0023	90.16	89.87
		N/A				0.00		0.015	129.00	0.00	94.00	89.92	89.32	N/A	0.0023	90.16	89.87
S5	S3	30	13.04	13.04	15.	17.73	17.73	0.015	708.00	11.80	103.00	93.30	91.54	9.70	0.0025	93.52	91.76
		N/A				0.00		0.015	708.00	0.00	103.00	93.30	91.54	N/A	0.0025	93.52	91.76
S4	S3	36	13.33	13.33	13.5	19.38	19.38	0.015	441.00	7.35	99.00	92.13	91.63	6.87	0.0011	92.26	91.76
		N/A				0.00		0.015	441.00	0.00	99.00	92.13	91.63	N/A	0.0011	92.26	91.76
S3	S2	42	8.55	34.92	14.5	48.75	48.75	0.015	724.00	12.07	96.00	91.32	89.05	4.68	0.0031	91.76	89.50
		N/A				0.00		0.015	724.00	0.00	96.00	91.32	89.05	N/A	0.0031	91.76	89.50
S2	S1	48	6.61	41.53	15.5	55.94	55.94	0.015	75.00	4.45	89.80	89.15	89.00	0.65	0.0020	89.49	89.34
		N/A				0.00		0.015	75.00	0.00	89.80	89.15	89.00	N/A	0.0020	89.49	89.34
SS2	SS1	30	7.63	7.63	13.5	8.96	8.96	0.015	160.00	2.67	85.00	81.75	81.64	3.25	0.0006	81.80	81.70
		N/A				0.00		0.015	160.00	0.00	85.00	81.75	81.64	N/A	0.0006	81.80	81.70
U8	U7	30	8.26	8.26	15.	12.12	12.12	0.015	350.00	7.04	131.00	123.94	120.28	7.06	0.0098	124.44	121.02
		N/A				0.00		0.015	350.00	0.00	131.00	123.94	120.28	N/A	0.0098	124.44	121.02
U7	U6	30	7.46	15.72	15.83	22.44	22.44	0.015	430.00	7.17	128.00	120.60	118.38	7.40	0.0050	121.18	119.02
		N/A				0.00		0.015	430.00	0.00	128.00	120.60	118.38	N/A	0.0050	121.18	119.02
U6	U5	36	6.88	22.6	16.83	29.27	29.27	0.015	450.00	7.50	124.00	118.37	116.83	5.63	0.0032	118.83	117.38
		N/A				0.00		0.015	450.00	0.00	124.00	118.37	116.83	N/A	0.0032	118.83	117.38
U5	U4	48	6.8	29.4	17.83	35.97	35.97	0.015	500.00	8.33	123.00	117.14	116.83	5.86	0.0008	117.38	116.97
		N/A				0.00		0.015	500.00	0.00	123.00	117.14	116.83	N/A	0.0008	117.38	116.97
U4	U3	48	7.1	36.5	22.	37.91	37.91	0.015	475.00	7.92	120.00	116.81	116.37	3.19	0.0009	116.97	116.53

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	475.00	0.00	120.00	116.81	116.37	N/A	0.0009	116.97	116.53
U3	U2	48	7.69	44.19	23.	43.22	43.22	0.015	160.00	3.44	118.00	116.32	116.13	1.68	0.0012	116.53	116.33
		N/A				0.00		0.015	160.00	0.00	118.00	116.32	116.13	N/A	0.0012	116.53	116.33
U2	U1	48	0.3	44.49	23.78	42.02	42.02	0.015	130.00	3.48	117.70	116.13	116.00	1.57	0.0010	116.32	116.19
		N/A				0.00		0.015	130.00	0.00	117.70	116.13	116.00	N/A	0.0010	116.32	116.19
T2B	T1B	18	3.29	3.29	13.5	4.35	4.35	0.015	91.00	3.19	88.00	84.33	83.80	3.67	0.0037	84.49	84.16
		N/A				0.00		0.015	91.00	0.00	88.00	84.33	83.80	N/A	0.0037	84.49	84.16
T2C	T1C	30	7.05	7.05	13.5	9.26	9.26	0.015	100.00	3.31	89.00	83.54	83.30	5.46	0.0020	83.72	83.52
		N/A				0.00		0.015	100.00	0.00	89.00	83.54	83.30	N/A	0.0020	83.72	83.52
T5A	T4A	36	15.39	15.39	16.5	19.73	19.73	0.015	401.00	6.68	93.00	85.36	84.89	7.64	0.0012	85.49	85.03
		N/A				0.00		0.015	401.00	0.00	93.00	85.36	84.89	N/A	0.0012	85.49	85.03
T4A	T3A	36	7.45	22.84	17.5	27.62	27.62	0.015	419.00	6.98	91.00	84.76	83.81	6.24	0.0023	85.03	84.07
		N/A				0.00		0.015	419.00	0.00	91.00	84.76	83.81	N/A	0.0023	85.03	84.07
T3A	T2A	42	10.02	32.86	18.5	37.72	37.72	0.015	354.00	5.90	88.50	83.80	83.14	4.70	0.0019	84.07	83.41
		N/A				0.00		0.015	354.00	0.00	88.50	83.80	83.14	N/A	0.0019	84.07	83.41
T2A	T1A	48	4.71	37.57	19.5	41.52	41.52	0.015	123.00	3.30	86.00	83.14	83.00	2.86	0.0011	83.33	83.19
		N/A				0.00		0.015	123.00	0.00	86.00	83.14	83.00	N/A	0.0011	83.33	83.19
TT5	TT4	30	0.7	0.7	15.	1.04	1.04	0.015	465.00	7.75	83.00	83.41	83.05	-0.41	0.0008	83.45	83.06
		N/A				0.00		0.015	465.00	0.00	83.00	83.41	83.05	N/A	0.0008	83.45	83.06
TT4	TT2	48	6.1	16.09	16.	19.81	19.81	0.015	506.00	8.43	84.00	82.90	81.84	1.10	0.0014	83.06	82.37
		N/A				0.00		0.015	506.00	0.00	84.00	82.90	81.84	N/A	0.0014	83.06	82.37
TT3	TT4	48	9.29	9.29	15.	11.66	11.66	0.015	243.00	4.05	85.00	83.07	83.04	1.93	0.0002	83.10	83.07
		N/A				0.00		0.015	243.00	0.00	85.00	83.07	83.04	N/A	0.0002	83.10	83.07
TT2	TT1	48	0.1	16.19	17.	19.25	19.25	0.015	63.00	3.38	85.00	81.90	81.80	3.10	0.0013	82.09	82.01
		N/A				0.00		0.015	63.00	0.00	85.00	81.90	81.80	N/A	0.0013	82.09	82.01
V3	V2	12	0.3	0.3	13.5	0.47	0.47	0.015	423.00	7.05	120.00	114.35	114.25	5.65	0.0003	114.37	114.25
		N/A				0.00		0.015	423.00	0.00	120.00	114.35	114.25	N/A	0.0003	114.37	114.25
V2	V1	12	0.3	0.6	14.5	0.91	0.91	0.015	55.00	1.16	119.00	114.23	114.18	4.77	0.0009	114.25	114.20
		N/A				0.00		0.015	55.00	0.00	119.00	114.23	114.18	N/A	0.0009	114.25	114.20
W2	W1	54	0.01	62.84	19.16	67.79	67.79	0.015	75.00	4.26	114.00	111.62	111.50	2.38	0.0016	111.93	111.81
		N/A				0.00		0.015	75.00	0.00	114.00	111.62	111.50	N/A	0.0016	111.93	111.81
W3	W2	54	7.5	62.83	18.51	69.23	69.23	0.015	170.00	4.35	114.50	111.89	111.61	2.61	0.0017	112.21	111.93
		N/A				0.00		0.015	170.00	0.00	114.50	111.89	111.61	N/A	0.0017	112.21	111.93
W4	W3	48	13.2	55.33	18.	62.07	62.07	0.015	150.00	4.94	114.60	112.17	111.80	2.43	0.0025	112.59	112.22
		N/A				0.00		0.015	150.00	0.00	114.60	112.17	111.80	N/A	0.0025	112.59	112.22
W5	W4	42	14.	42.13	17.	49.06	49.06	0.015	425.00	7.08	115.50	113.49	112.14	2.01	0.0032	113.94	112.59
		N/A				0.00		0.015	425.00	0.00	115.50	113.49	112.14	N/A	0.0032	113.94	112.59
W6	W5	36	11.12	28.13	16.	34.15	34.15	0.015	340.00	5.67	117.30	114.72	113.54	2.58	0.0035	115.12	113.94
		N/A				0.00		0.015	340.00	0.00	117.30	114.72	113.54	N/A	0.0035	115.12	113.94
W7	W6	30	12.69	12.69	15.	16.31	16.31	0.015	340.00	5.67	119.50	115.65	114.94	3.85	0.0021	115.84	115.13
		N/A				0.00		0.015	340.00	0.00	119.50	115.65	114.94	N/A	0.0021	115.84	115.13
W8	W6	24	4.32	4.32	15.	5.37	5.37	0.015	680.00	11.33	119.00	115.59	115.08	3.41	0.0007	115.64	115.13
		N/A				0.00		0.015	680.00	0.00	119.00	115.59	115.08	N/A	0.0007	115.64	115.13
X13	X2	48	21.23	21.23	15.	29.09	29.09	0.015	337.00	5.62	113.50	111.92	111.73	1.58	0.0005	112.01	111.83
		N/A				0.00		0.015	337.00	0.00	113.50	111.92	111.73	N/A	0.0005	112.01	111.83
X3	X2	42	15.5	30.82	19.	40.07	40.07	0.015	315.00	5.25	115.50	112.19	111.53	3.31	0.0021	112.49	111.83
		N/A				0.00		0.015	315.00	0.00	115.50	112.19	111.53	N/A	0.0021	112.49	111.83
X4	X3	36	5.1	15.32	18.	20.47	20.47	0.015	242.00	4.03	116.50	112.65	112.35	3.85	0.0013	112.80	112.49
		N/A				0.00		0.015	242.00	0.00	116.50	112.65	112.35	N/A	0.0013	112.80	112.49
X5	X4	36	0.01	10.22	17.	14.06	14.06	0.015	272.00	4.53	117.30	112.89	112.73	4.41	0.0006	112.96	112.80
		N/A				0.00		0.015	272.00	0.00	117.30	112.89	112.73	N/A	0.0006	112.96	112.80
X6	X5	36	5.4	10.21	16.	14.49	14.49	0.015	307.00	5.12	120.00	113.08	112.89	6.92	0.0006	113.15	112.96
		N/A				0.00		0.015	307.00	0.00	120.00	113.08	112.89	N/A	0.0006	113.15	112.96
X7	X6	30	4.81	4.81	15.	7.06	7.06	0.015	400.00	6.67	122.00	113.27	113.12	8.73	0.0004	113.31	113.16
		N/A				0.00		0.015	400.00	0.00	122.00	113.27	113.12	N/A	0.0004	113.31	113.16
X12	X11	24	0.3	0.3	15.	0.44	0.44	0.015	354.00	5.90	122.50	112.08	112.04	10.43	0.0001	112.09	112.05
		N/A				0.00		0.015	354.00	0.00	122.50	112.08	112.04	N/A	0.0001	112.09	112.05
X11	X10	30	0.3	0.6	16.	0.85	0.85	0.015	918.00	15.30	121.50	112.04	112.09	9.46	-0.0001	112.04	112.09
		N/A				0.00		0.015	918.00	0.00	121.50	112.04	112.09	N/A	-0.0001	112.04	112.09
X10	X9	30	0.3	0.9	17.	1.24	1.24	0.015	260.00	4.33	121.00	112.09	112.09	8.91	0.0000	112.09	112.09
		N/A				0.00		0.015	260.00	0.00	121.00	112.09	112.09	N/A	0.0000	112.09	112.09
X9	X8	36	7.5	8.4	18.	11.23	11.23	0.015	260.00	4.33	119.00	112.05	111.95	6.95	0.0004	112.09	111.99

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	260.00	0.00	119.00	112.05	111.95	N/A	0.0004	112.09	111.99
X8	X2	42	7.	15.4	19.	20.02	20.02	0.015	310.00	5.17	116.00	111.91	111.75	4.09	0.0005	111.99	111.83
		N/A				0.00		0.015	310.00	0.00	116.00	111.91	111.75	N/A	0.0005	111.99	111.83
X2	X1	60	0.3	67.75	20.	84.88	84.88	0.015	354.00	5.90	114.50	111.50	111.00	3.00	0.0014	111.82	111.32
		N/A				0.00		0.015	354.00	0.00	114.50	111.50	111.00	N/A	0.0014	111.82	111.32
Z2	Z1	48	20.19	20.19	13.5	31.15	31.15	0.015	73.00	3.04	111.00	107.09	107.04	3.91	0.0007	107.25	107.20
		N/A				0.00		0.015	73.00	0.00	111.00	107.09	107.04	N/A	0.0007	107.25	107.20

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Compute d Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
AA14	AA13	24	0.1	0.1	13.5	0.25	0.25	0.015	308.00	5.13	117.00	116.45	116.45	0.55	0.0000	116.45	116.45
		N/A				0.00		0.015	308.00	0.00	117.00	116.45	116.45	N/A	0.0000	116.45	116.45
AA13	AA12	36	8.04	8.14	14.5	18.75	18.75	0.015	364.00	6.07	116.50	116.33	115.95	0.17	0.0011	116.45	116.07
		N/A				0.00		0.015	364.00	0.00	116.50	116.33	115.95	N/A	0.0011	116.45	116.07
AA12	AA11	42	16.74	24.88	15.5	55.49	55.49	0.015	282.00	5.77	116.20	115.49	114.35	0.71	0.0041	116.07	114.92
		N/A				0.00		0.015	282.00	0.00	116.20	115.49	114.35	N/A	0.0041	116.07	114.92
AA11	AA10	42	3.49	28.37	16.31	61.07	61.07	0.015	307.00	6.35	115.00	114.23	112.72	0.77	0.0049	114.92	113.42
		N/A				0.00		0.015	307.00	0.00	115.00	114.23	112.72	N/A	0.0049	114.92	113.42
AA10	AA9	42	5.	33.37	17.12	69.45	69.45	0.015	297.00	7.22	113.50	112.52	110.63	0.98	0.0063	113.41	111.53
		N/A				0.00		0.015	297.00	0.00	113.50	112.52	110.63	N/A	0.0063	113.41	111.53
AA9	AA2	42	4.35	37.72	17.81	64.97	76.45	0.015	228.00	6.75	111.50	110.74	109.48	0.76	0.0056	111.53	110.26
		N/A				11.48		0.015	228.00	3.80	111.50	111.44	110.44	0.06	0.0044	111.53	110.53
AA8	AA7	36	13.67	13.67	13.5	32.84	32.84	0.015	268.00	4.65	118.10	116.54	115.68	1.56	0.0032	116.92	116.05
		N/A				0.00		0.015	268.00	0.00	118.10	116.54	115.68	N/A	0.0032	116.92	116.05
AA7	AA6	36	0.1	13.77	14.46	31.92	31.92	0.015	282.00	4.70	117.00	115.70	114.84	1.30	0.0030	116.05	115.19
		N/A				0.00		0.015	282.00	0.00	117.00	115.70	114.84	N/A	0.0030	116.05	115.19
AA6	AA5	36	4.03	17.8	15.46	39.22	39.22	0.015	301.00	5.55	116.00	114.66	113.27	1.34	0.0046	115.19	113.80
		N/A				0.00		0.015	301.00	0.00	116.00	114.66	113.27	N/A	0.0046	115.19	113.80
AA5	AA4	36	4.87	22.67	16.37	47.97	47.97	0.015	308.00	6.79	114.00	113.01	110.89	0.99	0.0069	113.80	111.68
		N/A				0.00		0.015	308.00	0.00	114.00	113.01	110.89	N/A	0.0069	113.80	111.68
AA4	AA3	42	5.28	27.95	17.12	56.93	57.31	0.015	218.00	5.92	111.50	111.07	110.14	0.43	0.0043	111.68	110.75
		N/A				0.37		0.015	218.00	3.63	111.50	111.64	110.75	-0.14	0.0043	111.67	110.75
AA3	AA2	48	4.76	32.71	17.73	42.99	65.49	0.015	405.00	6.75	110.00	110.54	110.06	-0.54	0.0012	110.75	110.26
		N/A				22.50		0.015	405.00	6.75	110.00	110.71	110.23	-0.71	0.0012	110.74	110.26
AA2	AA1	48	3.52	73.95	18.28	111.61	146.27	0.015	112.00	8.88	109.50	108.90	108.00	0.60	0.0080	110.26	109.36
		N/A				34.66		0.015	112.00	6.72	109.50	110.12	107.43	-0.62	0.0252	110.26	107.44
B4	B3	24	0.01	0.01	13.5	0.02	0.02	0.015	316.00	5.27	102.00	92.75	92.75	9.25	0.0000	92.75	92.75
		N/A				0.00		0.015	316.00	0.00	102.00	92.75	92.75	N/A	0.0000	92.75	92.75
B3	B2	30	9.89	9.9	14.5	22.92	22.92	0.015	317.00	5.28	102.00	92.38	91.06	9.62	0.0042	92.75	91.43
		N/A				0.00		0.015	317.00	0.00	102.00	92.38	91.06	N/A	0.0042	92.75	91.43
B2	B1	30	9.65	19.55	15.5	43.65	43.65	0.015	104.00	8.89	101.00	90.07	88.50	10.93	0.0151	91.43	89.86
		N/A				0.00		0.015	104.00	0.00	101.00	90.07	88.50	N/A	0.0151	91.43	89.86
BB2	BB1	48	17.81	17.81	13.5	30.90	44.47	0.015	87.00	4.73	105.00	105.21	104.65	-0.21	0.0025	105.56	105.34
		N/A				13.58		0.015	87.00	1.45	105.00	105.46	103.50	-0.46	0.0237	105.56	103.50
CC11	CC12	24	3.33	3.33	15.	7.88	7.88	0.015	310.00	5.17	110.50	104.59	104.09	5.91	0.0016	104.70	104.20
		N/A				0.00		0.015	310.00	0.00	110.50	104.59	104.09	N/A	0.0016	104.70	104.20
CC12	CC13	24	0.01	3.34	16.	7.65	7.65	0.015	260.00	4.33	107.60	104.09	103.70	3.51	0.0015	104.19	103.80
		N/A				0.00		0.015	260.00	0.00	107.60	104.09	103.70	N/A	0.0015	104.19	103.80
CC13	CC14	24	2.91	6.25	17.	13.88	13.88	0.015	260.00	4.42	106.50	103.46	102.16	3.04	0.0050	103.80	102.49
		N/A				0.00		0.015	260.00	0.00	106.50	103.46	102.16	N/A	0.0050	103.80	102.49
CC14	CC1-1	30	0.01	6.26	17.98	13.51	13.51	0.015	63.00	2.75	106.00	102.36	102.27	3.64	0.0014	102.49	102.40
		N/A				0.00		0.015	918.00	0.00	106.00	102.36	102.27	N/A	0.0001	102.49	102.40
CC4	CC3	54	10.6	65.03	18.84	118.49	132.35	0.015	206.00	7.45	107.80	108.12	107.13	-0.32	0.0048	109.08	108.08
		N/A				12.55		0.015	206.00	3.43	107.80	108.09	107.21	-0.29	0.0090	109.08	107.22
CC3	CC2	60	4.7	69.73	19.26	111.00	139.6	0.015	109.00	5.65	106.50	106.67	106.40	-0.17	0.0024	107.22	106.95
		N/A				28.60		0.015	109.00	1.82	106.50	107.13	106.93	-0.62	0.0024	107.22	106.95
CC5	CC4	54	3.58	54.43	18.28	114.17	114.17	0.015	242.00	7.18	110.30	109.28	108.19	1.02	0.0045	110.17	109.08
		N/A				0.00		0.015	242.00	0.00	110.30	109.28	108.19	N/A	0.0045	110.17	109.08
CC6	CC5	48	11.71	50.85	17.76	108.24	108.24	0.015	272.00	8.61	112.40	110.95	108.89	1.45	0.0076	112.22	110.17
		N/A				0.00		0.015	272.00	0.00	112.40	110.95	108.89	N/A	0.0076	112.22	110.17
CC7	CC6	48	16.14	39.14	17.	85.07	85.07	0.015	307.00	6.77	115.20	112.87	111.44	2.33	0.0047	113.66	112.23
		N/A				0.00		0.015	307.00	0.00	115.20	112.87	111.44	N/A	0.0047	113.66	112.23
CC8	CC7	48	0.1	23.	16.	52.38	52.38	0.015	400.00	6.67	117.10	114.07	113.36	3.03	0.0018	114.37	113.66
		N/A				0.00		0.015	400.00	0.00	117.10	114.07	113.36	N/A	0.0018	114.37	113.66
CC10	CC8	48	9.17	9.17	15.	21.41	21.41	0.015	354.00	5.90	118.10	114.43	114.32	3.67	0.0003	114.48	114.37
		N/A				0.00		0.015	354.00	0.00	118.10	114.43	114.32	N/A	0.0003	114.48	114.37
CC9	CC8	48	13.73	13.73	15.	32.48	32.48	0.015	374.00	6.23	116.30	114.51	114.26	1.79	0.0007	114.63	114.37
		N/A				0.00		0.015	374.00	0.00	116.30	114.51	114.26	N/A	0.0007	114.63	114.37
CC2	CC1	66	0.01	69.74	19.51	79.90	138.65	0.015	337.00	5.62	106.00	106.76	106.50	-0.76	0.0008	106.95	106.70
		N/A				58.75		0.015	337.00	5.62	106.00	106.84	105.73	-0.84	0.0031	106.95	105.92
DD3	DD2	18	0.1	0.1	13.5	0.25	0.25	0.015	170.00	2.83	107.00	102.35	102.35	4.65	0.0000	102.35	102.35

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	170.00	0.00	107.00	102.35	102.35	N/A	0.0000	102.35	102.35
DD2	DD1	18	3.77	3.87	14.5	8.47	8.47	0.015	40.00	4.79	106.00	101.95	101.61	4.05	0.0087	102.35	102.00
		N/A				0.00		0.015	40.00	0.00	106.00	101.95	101.61	N/A	0.0087	102.35	102.00
GG4	GG3	24	0.27	0.27	0.01	17.67	25.05	0.015	380.00	6.33	106.00	105.92	102.83	0.08	0.0081	106.47	103.38
		N/A				7.38		0.015	380.00	6.33	106.00	106.42	105.39	-0.42	0.0026	106.47	105.46
GG3	GG2	24	1.22	1.49	13.5	3.39	3.39	0.015	310.00	5.17	105.00	102.21	102.12	2.79	0.0003	102.23	102.14
		N/A				0.00		0.015	310.00	0.00	105.00	102.21	102.12	N/A	0.0003	102.23	102.14
GG2	GG1	24	0.99	2.48	14.5	5.31	5.31	0.015	45.00	1.69	104.00	102.09	102.05	1.91	0.0007	102.13	102.10
		N/A				0.00		0.015	45.00	0.00	104.00	102.09	102.05	N/A	0.0007	102.13	102.10
H5A	H4A	24	9.77	9.77	13.5	18.40	22.2	0.015	432.00	7.20	98.00	97.78	93.98	0.22	0.0088	98.37	94.57
		N/A				3.24		0.015	432.00	7.20	98.00	98.27	94.57	-0.27	0.0088	98.37	94.57
H4A	H3A	24	9.11	18.88	14.33	27.12	41.34	0.015	416.00	8.63	94.00	93.28	85.32	0.72	0.0191	94.57	86.61
		N/A				14.22		0.015	416.00	6.93	94.00	94.47	89.72	-0.47	0.0116	94.57	89.73
H3A	H2A	36	0.01	18.89	14.86	40.53	40.53	0.015	286.00	5.73	89.00	86.04	84.63	2.96	0.0049	86.60	85.20
		N/A				0.00		0.015	286.00	0.00	89.00	86.04	84.63	N/A	0.0049	86.60	85.20
H2A	H1A	36	11.07	29.96	15.69	62.42	62.42	0.015	73.00	8.83	87.00	83.85	83.00	3.15	0.0117	85.20	84.34
		N/A				0.00		0.015	73.00	0.00	87.00	83.85	83.00	N/A	0.0117	85.20	84.34
H16B	H15B	30	4.42	4.42	15.	9.65	9.65	0.015	340.00	5.67	101.00	98.36	98.11	2.64	0.0007	98.43	98.18
		N/A				0.00		0.015	340.00	0.00	101.00	98.36	98.11	N/A	0.0007	98.43	98.18
H15B	H14B	30	0.3	4.72	16.	10.01	10.01	0.015	352.00	5.87	101.00	98.10	97.82	2.90	0.0008	98.17	97.90
		N/A				0.00		0.015	352.00	0.00	101.00	98.10	97.82	N/A	0.0008	98.17	97.90
H14B	H13B	36	2.44	7.16	20.	13.59	13.59	0.015	340.00	5.67	101.00	97.83	97.64	3.17	0.0006	97.89	97.71
		N/A				0.00		0.015	340.00	0.00	101.00	97.83	97.64	N/A	0.0006	97.89	97.71
H13B	H12B	36	0.3	7.46	21.	13.57	13.57	0.015	398.00	6.63	101.00	97.64	97.42	3.36	0.0006	97.70	97.49
		N/A				0.00		0.015	398.00	0.00	101.00	97.64	97.42	N/A	0.0006	97.70	97.49
H12B	H11B	36	0.3	7.76	22.	13.55	13.55	0.015	467.00	7.78	100.70	97.42	97.16	3.28	0.0005	97.48	97.23
		N/A				0.00		0.015	467.00	0.00	100.70	97.42	97.16	N/A	0.0005	97.48	97.23
H11B	H10B	42	11.4	19.16	23.	32.66	32.66	0.015	402.00	6.70	102.00	97.03	96.46	4.97	0.0014	97.22	96.66
		N/A				0.00		0.015	402.00	0.00	102.00	97.03	96.46	N/A	0.0014	97.22	96.66
H10B	H6B	60	5.09	24.25	24.	39.79	39.79	0.015	493.00	8.22	100.20	96.59	96.44	3.61	0.0003	96.66	96.51
		N/A				0.00		0.015	493.00	0.00	100.20	96.59	96.44	N/A	0.0003	96.66	96.51
H9B	H8B	60	20.09	20.09	15.	42.85	42.85	0.015	387.00	6.45	99.30	97.99	97.85	1.31	0.0004	98.08	97.94
		N/A				0.00		0.015	387.00	0.00	99.30	97.99	97.85	N/A	0.0004	98.08	97.94
H8B	H7B	60	16.88	36.97	16.	76.13	76.13	0.015	326.00	5.43	98.50	97.67	97.30	0.83	0.0011	97.93	97.56
		N/A				0.00		0.015	326.00	0.00	98.50	97.67	97.30	N/A	0.0011	97.93	97.56
H7B	H6B	60	24.21	61.18	17.	122.38	122.38	0.015	359.00	6.23	98.00	96.89	95.84	1.11	0.0029	97.56	96.51
		N/A				0.00		0.015	359.00	0.00	98.00	96.89	95.84	N/A	0.0029	97.56	96.51
H6B	H5B	66	5.9	91.33	17.96	180.12	180.12	0.015	428.00	7.58	97.30	95.51	93.88	1.79	0.0038	96.50	94.86
		N/A				0.00		0.015	359.00	0.00	97.30	95.51	93.88	N/A	0.0046	96.50	94.86
H5B	H4B	66	0.01	91.34	18.9	175.10	175.1	0.015	368.00	7.37	95.00	93.93	92.60	1.07	0.0036	94.86	93.53
		N/A				0.00		0.015	368.00	0.00	95.00	93.93	92.60	N/A	0.0036	94.86	93.53
H4B	H3B	66	5.94	97.28	19.73	171.32	182.7	0.015	414.00	7.21	93.00	92.63	91.20	0.37	0.0035	93.53	92.09
		N/A				11.38		0.015	414.00	6.90	93.00	93.43	92.09	-0.43	0.0035	93.53	92.09
H3B	H2B	66	8.18	105.46	20.63	144.57	195.03	0.015	169.00	6.09	91.00	91.45	91.04	-0.45	0.0025	92.09	91.68
		N/A				50.45		0.015	169.00	2.82	91.00	92.06	91.73	-1.06	0.0013	92.09	91.87
H2B	H1B	72	3.95	109.41	20.97	75.06	199.29	0.015	80.00	2.65	89.90	91.04	91.00	-1.14	0.0004	91.16	91.12
		N/A				124.23		0.015	80.00	6.65	89.90	90.86	90.28	-0.96	0.0095	91.16	90.39
HH4	HH3	24	5.3	5.3	10.	12.84	15.39	0.015	352.00	5.87	102.50	102.55	101.04	-0.05	0.0043	102.83	101.32
		N/A				2.55		0.015	352.00	5.87	102.50	102.79	101.29	-0.29	0.0043	102.83	101.33
HH3	HH2	24	3.7	9.	10.83	22.88	25.1	0.015	417.00	7.28	101.00	100.41	94.73	0.59	0.0136	101.32	95.64
		N/A				2.22		0.015	417.00	6.95	101.00	101.29	99.78	-0.28	0.0036	101.32	99.82
HH2	HH1	30	9.23	18.23	15.	43.12	43.12	0.015	186.00	8.78	99.50	94.31	91.57	5.19	0.0147	95.64	92.90
		N/A				0.00		0.015	186.00	0.00	99.50	94.31	91.57	N/A	0.0147	95.64	92.90
JJ17	JJ9	18	0.3	0.3	15.	0.72	0.72	0.015	470.00	7.83	109.00	102.26	102.23	6.74	0.0001	102.26	102.23
		N/A				0.00		0.015	470.00	0.00	109.00	102.26	102.23	N/A	0.0001	102.26	102.23
JJ16	JJ15	36	11.33	11.33	15.	25.31	25.31	0.015	550.00	9.17	110.10	107.83	106.77	2.27	0.0019	108.05	106.99
		N/A				0.00		0.015	550.00	0.00	110.10	107.83	106.77	N/A	0.0019	108.05	106.99
JJ15	JJ14	42	12.6	23.93	16.	42.22	51.	0.015	350.00	5.83	106.50	106.66	105.84	-0.16	0.0023	106.99	106.17
		N/A				8.79		0.015	350.00	5.83	106.50	106.93	106.16	-0.43	0.0023	106.99	106.17
JJ14	JJ12	42	10.98	34.91	16.83	47.52	70.03	0.015	110.00	4.94	105.50	105.75	105.42	-0.25	0.0030	106.17	105.84
		N/A				22.51		0.015	110.00	2.03	105.50	106.01	105.82	-0.51	0.0030	106.17	105.84
JJ13	JJ12	48	9.5	9.5	15.	20.34	20.34	0.015	550.00	9.17	109.20	105.94	105.80	3.26	0.0003	105.99	105.84

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		15				0.00		0.015	550.00	0.00	109.20	105.94	105.80	N/A	0.0003	105.99	105.84
JJ12	JJ11	48	9.16	53.57	17.08	64.46	106.58	0.015	335.00	5.58	105.00	105.39	104.49	-0.39	0.0027	105.84	104.94
		N/A				42.12		0.015	335.00	5.58	105.00	105.75	104.90	-0.75	0.0027	105.84	104.94
JJ11	JJ10	48	10.67	64.24	18.08	69.59	123.78	0.015	330.00	5.54	104.00	104.41	103.38	-0.41	0.0031	104.94	103.91
		N/A				54.19		0.015	330.00	5.50	104.00	104.85	103.91	-0.85	0.0026	104.94	104.09
JJ10	JJ9	60	10.17	74.41	19.08	114.61	139.08	0.015	650.00	10.83	103.20	103.32	101.64	-0.12	0.0026	103.91	102.23
		N/A				24.47		0.015	650.00	10.83	103.20	103.85	102.19	-0.65	0.0026	103.91	102.23
JJ9	JJ8	60	11.73	86.44	19.9	129.92	157.81	0.015	350.00	6.62	101.50	101.47	100.31	0.03	0.0033	102.23	101.07
		N/A				27.88		0.015	350.00	5.83	101.50	102.16	101.09	-0.66	0.0029	102.23	101.21
JJ8	JJ7	66	9.6	96.04	20.63	159.07	173.06	0.015	335.00	6.70	100.50	100.30	99.29	0.20	0.0030	101.07	100.07
		N/A				13.99		0.015	335.00	5.58	100.50	100.99	99.97	-0.49	0.0030	101.07	100.06
JJ7	JJ6	66	4.2	100.24	21.39	163.51	175.12	0.015	370.00	6.88	99.50	99.25	98.08	0.25	0.0032	100.06	98.90
		N/A				11.61		0.015	370.00	6.17	99.50	99.89	98.94	-0.39	0.0028	100.06	99.03
JJ6	JJ5	66	0.	100.24	22.23	164.46	168.59	0.015	425.00	7.08	98.50	98.07	96.71	0.43	0.0032	98.89	97.54
		N/A				4.14		0.015	425.00	7.08	98.50	98.86	97.82	-0.36	0.0024	98.89	97.88
JJ5	JJ4	66	4.9	105.14	23.2	170.44	170.45	0.015	510.00	8.50	97.50	96.63	94.88	0.87	0.0034	97.51	95.76
		N/A				0.01		0.015	510.00	8.50	97.50	97.53	96.03	-0.03	0.0029	97.53	96.03
JJ4	JJ3	72	11.04	116.18	24.2	182.77	182.77	0.015	410.00	6.83	96.00	95.04	94.02	0.96	0.0025	95.76	94.74
		N/A				0.00		0.015	410.00	0.00	96.00	95.04	94.02	N/A	0.0025	95.76	94.74
JJ3	JJ2	72	10.43	126.61	25.2	190.87	193.22	0.015	480.00	8.00	94.40	93.96	92.66	0.44	0.0027	94.74	93.44
		N/A				2.34		0.015	480.00	8.00	94.40	94.68	92.82	-0.28	0.0040	94.72	92.82
JJ2	JJ1	72	14.81	141.42	26.19	167.72	210.22	0.015	100.00	5.93	92.00	92.21	92.00	-0.21	0.0021	92.61	92.61
		N/A				42.50		0.015	100.00	4.26	92.00	92.66	91.86	-0.66	0.0090	92.82	91.91
L2	L1	60	6.38	88.65	27.47	119.96	119.96	0.015	250.00	6.11	98.00	96.71	96.00	1.29	0.0028	97.35	96.64
		N/A				0.00		0.015	250.00	0.00	98.00	96.71	96.00	N/A	0.0028	97.35	96.64
L3	L2	54	7.86	82.27	26.66	114.58	114.58	0.015	350.00	7.20	100.00	98.04	96.46	1.96	0.0045	98.93	97.35
		N/A				0.00		0.015	350.00	0.00	100.00	98.04	96.46	N/A	0.0045	98.93	97.35
L4	L3	54	7.26	7.26	20.	13.46	13.46	0.015	330.00	5.50	101.00	98.94	98.92	2.06	0.0001	98.95	98.93
		N/A				0.00		0.015	330.00	0.00	101.00	98.94	98.92	N/A	0.0001	98.95	98.93
L5	L3	48	15.41	67.15	25.66	97.13	97.13	0.015	480.00	8.00	103.00	100.83	97.91	2.17	0.0061	101.86	98.93
		N/A				0.00		0.015	480.00	0.00	103.00	100.83	97.91	N/A	0.0061	101.86	98.93
L6	L5	48	10.14	51.74	24.74	77.79	77.79	0.015	340.00	6.19	106.00	102.53	101.20	3.47	0.0039	103.19	101.86
		N/A				0.00		0.015	340.00	0.00	106.00	102.53	101.20	N/A	0.0039	103.19	101.86
L7	L6	42	4.88	41.6	23.97	64.68	64.68	0.015	310.00	6.72	108.00	104.11	102.41	3.89	0.0055	104.89	103.19
		N/A				0.00		0.015	310.00	0.00	108.00	104.11	102.41	N/A	0.0055	104.89	103.19
L8	L7	24	5.6	5.6	20.	10.34	10.34	0.015	415.00	6.92	110.00	105.86	104.71	4.14	0.0028	106.05	104.89
		N/A				0.00		0.015	415.00	0.00	110.00	105.86	104.71	N/A	0.0028	106.05	104.89
L9	L7	36	9.8	31.12	22.97	50.65	50.65	0.015	525.00	8.75	110.00	108.04	104.01	1.96	0.0077	108.93	104.89
		N/A				0.00		0.015	525.00	0.00	110.00	108.04	104.01	N/A	0.0077	108.93	104.89
L10	L9	36	7.11	21.32	21.97	36.46	36.46	0.015	490.00	8.17	112.00	110.42	108.47	1.58	0.0040	110.88	108.93
		N/A				0.00		0.015	490.00	0.00	112.00	110.42	108.47	N/A	0.0040	110.88	108.93
L11	L10	30	0.3	14.21	21.	25.61	25.61	0.015	305.00	5.22	113.10	111.99	110.41	1.11	0.0052	112.46	110.88
		N/A				0.00		0.015	305.00	0.00	113.10	111.99	110.41	N/A	0.0052	112.46	110.88
L12	L11	30	7.5	7.5	20.	14.17	14.17	0.015	650.00	10.83	113.90	113.35	112.32	0.55	0.0016	113.50	112.46
		N/A				0.00		0.015	650.00	0.00	113.90	113.35	112.32	N/A	0.0016	113.50	112.46
L13	L11	24	6.41	6.41	20.	12.11	12.11	0.015	550.00	9.17	115.00	114.31	112.21	0.69	0.0038	114.56	112.46
		N/A				0.00		0.015	550.00	0.00	115.00	114.31	112.21	N/A	0.0038	114.56	112.46
LL12	LL5	24	7.52	7.52	15.	16.14	17.12	0.015	410.00	6.83	100.50	100.30	97.52	0.20	0.0068	100.75	97.97
		N/A				0.98		0.015	410.00	6.83	100.50	100.70	97.97	-0.20	0.0068	100.75	97.97
LL11	LL10	24	7.22	7.22	15.	0.85	16.27	0.015	470.00	7.83	107.00	107.55	106.64	-0.55	0.0021	107.62	106.65
		N/A				15.42		0.015	470.00	7.83	107.00	107.58	106.62	-0.58	0.0021	107.62	106.65
LL10	LL9	24	7.46	14.68	15.95	10.76	32.22	0.015	430.00	7.17	106.00	106.30	104.57	-0.30	0.0034	106.57	105.11
		N/A				21.45		0.015	430.00	7.17	106.00	106.53	104.25	-0.53	0.0055	106.65	104.27
LL9	LL8	36	6.56	21.23	16.61	9.69	45.76	0.015	400.00	6.67	103.50	103.87	102.41	-0.37	0.0037	104.27	102.81
		N/A				36.07		0.015	400.00	6.67	103.50	104.16	102.34	-0.66	0.0047	104.27	102.38
LL8	LL7	42	5.64	26.87	17.4	5.30	56.55	0.015	450.00	7.50	101.50	102.11	100.40	-0.61	0.0043	102.38	100.45
		N/A				51.25		0.015	450.00	7.50	101.50	102.25	100.37	-0.75	0.0043	102.38	100.44
LL7	LL6	42	8.62	35.49	18.31	10.10	72.74	0.015	350.00	5.83	99.50	100.06	98.39	-0.56	0.0056	100.44	98.48
		N/A				62.64		0.015	350.00	5.83	99.50	100.27	98.37	-0.77	0.0056	100.44	98.48
LL6	LL5	42	6.1	41.59	19.17	16.90	83.	0.015	130.00	4.50	97.50	97.96	97.86	-0.46	0.0039	98.48	97.97
		N/A				66.10		0.015	130.00	2.83	97.50	98.34	97.76	-0.84	0.0039	98.48	97.96
LL5	LL4	42	6.01	55.12	19.78	41.34	108.29	0.015	340.00	8.75	97.00	97.06	94.80	-0.06	0.0081	97.97	95.22

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				66.96		0.015	340.00	5.67	97.00	97.76	94.96	-0.76	0.0086	97.97	95.05
LL4	LL3	48	10.87	65.98	20.64	50.66	127.77	0.015	295.00	10.01	94.00	94.07	91.79	-0.07	0.0096	95.02	92.18
		N/A				77.11		0.015	295.00	4.92	94.00	94.80	92.05	-0.80	0.0098	95.03	92.13
LL3	LL2	54	8.8	74.78	21.44	65.47	140.45	0.015	205.00	9.36	91.00	90.97	89.71	0.03	0.0095	92.02	90.06
		N/A				74.98		0.015	205.00	3.42	91.00	91.79	89.81	-0.79	0.0098	92.01	90.01
LL2	LL1	54	11.85	86.62	22.15	102.26	157.38	0.015	100.00	7.95	89.00	88.82	87.97	0.18	0.0044	89.87	89.42
		N/A				55.13		0.015	100.00	4.64	89.00	89.71	88.93	-0.71	0.0090	89.90	89.00
M4A	M3A	30	7.16	7.16	15.	14.95	14.95	0.015	305.00	5.08	103.00	97.80	97.26	5.20	0.0018	97.96	97.42
		N/A				0.00		0.015	305.00	0.00	103.00	97.80	97.26	N/A	0.0018	97.96	97.42
M3A	M2A	42	0.3	16.86	16.	34.52	34.52	0.015	358.00	5.97	102.40	97.19	96.63	5.21	0.0016	97.42	96.86
		N/A				0.00		0.015	358.00	0.00	102.40	97.19	96.63	N/A	0.0016	97.42	96.86
M5A	M3A	30	9.4	9.4	15.	20.13	20.13	0.015	388.00	6.47	106.00	98.37	97.13	7.63	0.0032	98.66	97.42
		N/A				0.00		0.015	388.00	0.00	106.00	98.37	97.13	N/A	0.0032	98.66	97.42
M2A	M1A	42	4.38	21.24	17.	41.86	41.86	0.015	445.00	7.42	102.00	96.53	95.50	5.47	0.0023	96.85	95.83
		N/A				0.00		0.015	445.00	0.00	102.00	96.53	95.50	N/A	0.0023	96.85	95.83
M10B	M3B	24	0.3	0.3	15.	0.72	0.72	0.015	305.00	5.08	110.00	106.19	106.19	3.81	0.0000	106.19	106.19
		N/A				0.00		0.015	305.00	0.00	110.00	106.19	106.19	N/A	0.0000	106.19	106.19
M9B	M8B	36	10.6	10.6	20.	20.90	20.9	0.015	358.00	5.97	115.00	110.34	109.88	4.66	0.0013	110.49	110.03
		N/A				0.00		0.015	358.00	0.00	115.00	110.34	109.88	N/A	0.0013	110.49	110.03
M8B	M7B	36	7.16	17.76	21.	33.22	33.22	0.015	388.00	6.47	112.50	109.64	108.36	2.86	0.0033	110.03	108.74
		N/A				0.00		0.015	388.00	0.00	112.50	109.64	108.36	N/A	0.0033	110.03	108.74
M7B	M3B	36	6.45	24.21	22.	43.79	43.79	0.015	445.00	7.42	110.00	108.08	105.53	1.92	0.0057	108.74	106.19
		N/A				0.00		0.015	445.00	0.00	110.00	108.08	105.53	N/A	0.0057	108.74	106.19
M6B	M5B	36	10.29	10.29	20.	19.52	19.52	0.015	417.00	6.95	109.40	107.07	106.60	2.33	0.0011	107.20	106.73
		N/A				0.00		0.015	417.00	0.00	109.40	107.07	106.60	N/A	0.0011	107.20	106.73
M5B	M4B	36	7.41	17.7	21.	0.90	31.09	0.015	450.00	7.50	104.00	106.73	106.73	-2.73	0.0000	106.73	106.73
		N/A				30.19		0.015	450.00	7.50	104.00	106.71	106.72	-2.71	0.0000	106.71	106.73
M4B	M3B	48	9.37	27.07	21.97	44.48	44.48	0.015	420.00	7.00	105.00	106.51	105.97	-1.51	0.0013	106.73	106.19
		N/A				0.00		0.015	420.00	0.00	105.00	106.51	105.97	N/A	0.0013	106.73	106.19
M3B	M2B	54	2.29	53.87	22.97	89.01	89.01	0.015	330.00	5.59	108.30	105.65	104.75	2.65	0.0027	106.19	105.29
		N/A				0.00		0.015	330.00	0.00	108.30	105.65	104.75	N/A	0.0027	106.19	105.29
M2B	M1B	54	0.01	53.88	23.95	85.43	85.43	0.015	114.00	5.37	107.00	104.79	104.50	2.21	0.0025	105.29	105.00
		N/A				0.00		0.015	114.00	0.00	107.00	104.79	104.50	N/A	0.0025	105.29	105.00
M2C	M1C	18	3.67	3.67	15.	7.82	7.82	0.015	384.00	6.40	108.50	107.33	104.50	1.17	0.0074	107.67	104.84
		N/A				0.00		0.015	384.00	0.00	108.50	107.33	104.50	N/A	0.0074	107.67	104.84
M4D	M2D	24	7.25	7.25	15.	15.38	15.55	0.015	384.00	6.40	110.00	109.72	107.36	0.28	0.0062	110.14	107.78
		N/A				0.18		0.015	384.00	6.40	110.00	110.12	109.11	-0.12	0.0026	110.14	109.13
M3D	M2D	30	4.52	4.52	15.	9.71	9.71	0.015	222.00	3.70	111.00	107.68	107.51	3.32	0.0007	107.74	107.58
		N/A				0.00		0.015	222.00	0.00	111.00	107.68	107.51	N/A	0.0007	107.74	107.58
M2D	M1D	36	0.3	12.07	15.99	25.00	25.	0.015	192.00	3.54	108.00	107.36	107.00	0.64	0.0019	107.58	107.22
		N/A				0.00		0.015	192.00	0.00	108.00	107.36	107.00	N/A	0.0019	107.58	107.22
M6E	M2E	36	49.9	49.9	15.	86.45	106.84	0.015	384.00	12.23	118.00	116.06	107.47	1.94	0.0224	118.64	110.05
		N/A				20.39		0.015	384.00	6.40	118.00	118.52	111.94	-0.52	0.0174	118.64	111.95
M4E	M3E	24	3.05	3.05	15.	6.55	6.55	0.015	222.00	3.70	114.00	110.30	110.05	3.70	0.0011	110.38	110.13
		N/A				0.00		0.015	222.00	0.00	114.00	110.30	110.05	N/A	0.0011	110.38	110.13
M3E	M2E	36	2.55	5.6	16.	11.61	11.61	0.015	192.00	3.20	111.00	110.08	110.00	0.92	0.0004	110.13	110.05
		N/A				0.00		0.015	192.00	0.00	111.00	110.08	110.00	N/A	0.0004	110.13	110.05
M5E	M2E	24	2.68	2.68	15.	5.84	5.84	0.015	280.00	4.67	111.90	110.24	109.99	1.66	0.0009	110.30	110.05
		N/A				0.00		0.015	280.00	0.00	111.90	110.24	109.99	N/A	0.0009	110.30	110.05
M2E	M1E	42	0.3	58.48	15.42	123.42	123.42	0.015	112.00	12.83	108.00	107.21	104.97	0.79	0.0200	110.05	107.80
		N/A				0.00		0.015	112.00	0.00	108.00	107.21	104.97	N/A	0.0200	110.05	107.80
M7F	M6F	36	2.87	2.87	20.	5.43	5.43	0.015	625.00	10.42	121.00	119.08	119.03	1.92	0.0001	119.09	119.04
		N/A				0.00		0.015	625.00	0.00	121.00	119.08	119.03	N/A	0.0001	119.09	119.04
M6F	M4F	36	6.48	9.35	21.	17.72	17.72	0.015	436.00	7.27	120.20	118.93	118.52	1.27	0.0009	119.04	118.63
		N/A				0.00		0.015	436.00	0.00	120.20	118.93	118.52	N/A	0.0009	119.04	118.63
M5F	M4F	42	10.79	10.79	20.	19.60	19.6	0.015	297.00	4.95	119.50	118.70	118.55	0.80	0.0005	118.78	118.63
		N/A				0.00		0.015	297.00	0.00	119.50	118.70	118.55	N/A	0.0005	118.78	118.63
M4F	M3F	42	6.98	27.12	21.	48.24	48.24	0.015	500.00	8.33	119.00	118.19	116.66	0.81	0.0031	118.62	117.09
		N/A				0.00		0.015	500.00	0.00	119.00	118.19	116.66	N/A	0.0031	118.62	117.09
M3F	M2F	48	11.48	38.6	22.	65.02	65.08	0.015	495.00	8.25	117.00	116.63	115.28	0.37	0.0027	117.09	115.74
		N/A				0.07		0.015	495.00	8.25	117.00	117.08	115.74	-0.08	0.0027	117.09	115.74
M8F	M2F	36	2.56	2.56	20.	4.31	4.75	0.015	242.00	4.03	115.50	115.75	115.73	-0.25	0.0001	115.76	115.74

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.43		0.015	242.00	4.03	115.50	115.75	115.74	-0.25	0.0001	115.75	115.74
M2F	M1F	48	10.69	51.85	23.	82.76	83.6	0.015	82.00	11.17	115.00	114.36	112.10	0.64	0.0120	115.74	114.76
		N/A				0.84		0.015	82.00	1.60	115.00	115.69	115.20	-0.69	0.0061	115.74	115.24
M2G	M1G	36	8.02	8.02	15.	0.72	15.99	0.015	92.00	1.76	114.00	114.48	114.29	-0.48	0.0023	114.58	114.37
		N/A				15.27		0.015	92.00	3.94	114.00	114.48	113.71	-0.48	0.0093	114.58	113.73
M2H	M1H	24	4.99	4.99	15.	4.95	11.32	0.015	80.00	3.88	112.00	112.12	111.79	-0.12	0.0041	112.44	112.11
		N/A				6.37		0.015	80.00	3.94	112.00	112.36	111.59	-0.36	0.0105	112.44	111.60
MM2	MM1	18	4.22	4.22	13.5	8.96	8.96	0.015	120.00	5.07	90.00	86.52	85.36	3.48	0.0097	86.97	85.80
		N/A				0.00		0.015	120.00	0.00	90.00	86.52	85.36	N/A	0.0097	86.97	85.80
N6A	N5A	18	3.64	3.64	15.	7.68	8.04	0.015	433.00	7.22	104.00	103.85	100.77	0.15	0.0071	104.17	101.09
		N/A				0.36		0.015	433.00	7.22	104.00	104.14	102.13	-0.14	0.0046	104.17	102.17
N5A	N4A	36	13.64	17.28	15.96	37.33	37.33	0.015	471.00	7.85	102.00	100.61	98.65	1.39	0.0042	101.09	99.13
		N/A				0.00		0.015	471.00	0.00	102.00	100.61	98.65	N/A	0.0042	101.09	99.13
N4A	N3A	42	6.84	29.32	16.96	62.45	62.45	0.015	600.00	10.00	100.20	98.40	95.32	1.80	0.0051	99.12	96.05
		N/A				0.00		0.015	600.00	0.00	100.20	98.40	95.32	N/A	0.0051	99.12	96.05
N3A	N2A	48	5.98	35.3	17.96	72.41	72.41	0.015	269.00	5.76	98.00	95.47	94.56	2.53	0.0034	96.04	95.13
		N/A				0.00		0.015	269.00	0.00	98.00	95.47	94.56	N/A	0.0034	96.04	95.13
N7A	N4A	24	5.2	5.2	15.	12.20	12.2	0.015	638.00	10.63	106.00	101.34	98.87	4.66	0.0039	101.60	99.13
		N/A				0.00		0.015	638.00	0.00	106.00	101.34	98.87	N/A	0.0039	101.60	99.13
N2A	N1A	48	1.42	36.72	18.73	73.61	73.61	0.015	111.00	5.86	98.00	94.39	94.00	3.61	0.0035	94.98	94.59
		N/A				0.00		0.015	111.00	0.00	98.00	94.39	94.00	N/A	0.0035	94.98	94.59
N3B	N2B	18	2.94	2.94	15.	6.72	6.72	0.015	266.00	4.43	108.00	102.63	101.18	5.37	0.0054	102.88	101.43
		N/A				0.00		0.015	266.00	0.00	108.00	102.63	101.18	N/A	0.0054	102.88	101.43
N2B	N1B	24	1.9	4.84	16.	10.44	10.44	0.015	82.00	3.32	106.70	101.23	101.00	5.47	0.0028	101.43	101.19
		N/A				0.00		0.015	82.00	0.00	106.70	101.23	101.00	N/A	0.0028	101.43	101.19
N8C	N2C	24	5.76	5.76	13.5	13.08	13.08	0.015	266.00	4.43	108.00	103.26	102.07	4.74	0.0045	103.55	102.37
		N/A				0.00		0.015	266.00	0.00	108.00	103.26	102.07	N/A	0.0045	103.55	102.37
N7C	N6C	18	0.1	0.1	13.5	0.25	0.25	0.015	196.00	3.27	102.00	102.56	102.57	-0.56	0.0000	102.56	102.57
		N/A				0.00		0.015	196.00	0.00	102.00	102.56	102.57	N/A	0.0000	102.56	102.57
N5C	N6C	24	2.61	2.61	13.5	5.85	5.85	0.015	105.00	1.90	112.00	102.59	102.51	9.41	0.0007	102.65	102.57
		N/A				0.00		0.015	105.00	0.00	112.00	102.59	102.51	N/A	0.0007	102.65	102.57
N6C	N4C	30	0.1	2.81	14.42	6.12	6.12	0.015	192.00	3.20	109.00	102.54	102.49	6.46	0.0003	102.57	102.52
		N/A				0.00		0.015	192.00	0.00	109.00	102.54	102.49	N/A	0.0003	102.57	102.52
N4C	N3C	30	1.84	4.65	15.42	9.74	9.74	0.015	197.00	3.28	110.00	102.45	102.30	7.55	0.0007	102.52	102.37
		N/A				0.00		0.015	197.00	0.00	110.00	102.45	102.30	N/A	0.0007	102.52	102.37
N3C	N3C1	30	0.21	4.86	16.42	9.88	9.88	0.015	150.00	3.62	109.00	101.22	100.68	7.78	0.0025	101.42	101.04
		N/A				0.00		0.015	150.00	0.00	109.00	101.22	100.68	N/A	0.0025	101.42	101.04
N2C	N1C	36	1.96	7.72	14.5	16.84	16.84	0.015	79.00	2.38	108.00	102.07	102.00	5.93	0.0008	102.17	102.10
		N/A				0.00		0.015	79.00	0.00	108.00	102.07	102.00	N/A	0.0008	102.17	102.10
N2D	N1D	24	1.89	1.89	15.	4.04	4.04	0.015	109.00	2.79	105.00	102.14	101.71	2.86	0.0025	102.26	101.99
		N/A				0.00		0.015	109.00	0.00	105.00	102.14	101.71	N/A	0.0025	102.26	101.99
N5E	N4E	30	10.46	10.46	15.	22.40	22.4	0.015	461.00	7.68	107.00	106.17	104.34	0.83	0.0040	106.53	104.70
		N/A				0.00		0.015	461.00	0.00	107.00	106.17	104.34	N/A	0.0040	106.53	104.70
N4E	N3E	30	7.06	17.52	16.	36.18	36.18	0.015	285.00	7.37	109.00	103.76	100.81	5.24	0.0104	104.70	101.74
		N/A				0.00		0.015	285.00	0.00	109.00	103.76	100.81	N/A	0.0104	104.70	101.74
N3E	N2E	30	0.3	17.82	16.64	36.07	36.07	0.015	154.00	7.35	110.00	100.81	99.23	9.19	0.0103	101.74	100.16
		N/A				0.00		0.015	154.00	0.00	110.00	100.81	99.23	N/A	0.0103	101.74	100.16
N2E	N1E	36	0.01	17.83	16.99	35.67	35.67	0.015	119.00	5.05	109.00	99.72	99.26	9.28	0.0038	100.15	99.70
		N/A				0.00		0.015	119.00	0.00	109.00	99.72	99.26	N/A	0.0038	100.15	99.70
N2F	N1F	30	5.5	5.5	15.	11.78	11.78	0.015	105.00	3.81	109.00	95.71	95.15	13.29	0.0028	95.93	95.64
		N/A				0.00		0.015	105.00	0.00	109.00	95.71	95.15	N/A	0.0028	95.93	95.64
N2G	N1G	30	4.5	4.5	15.	9.63	9.63	0.015	83.00	3.58	101.00	97.50	97.04	3.50	0.0028	97.70	97.47
		N/A				0.00		0.015	83.00	0.00	101.00	97.50	97.04	N/A	0.0028	97.70	97.47
N2H	N1H	36	7.3	7.3	15.	15.63	15.63	0.015	108.00	3.95	105.00	101.82	101.26	3.18	0.0025	102.06	101.79
		N/A				0.00		0.015	108.00	0.00	105.00	101.82	101.26	N/A	0.0025	102.06	101.79
N2I	N1I	18	2.1	2.1	15.	4.50	4.5	0.015	90.00	2.86	110.00	103.71	103.50	6.29	0.0023	103.82	103.61
		N/A				0.00		0.015	90.00	0.00	110.00	103.71	103.50	N/A	0.0023	103.82	103.61
N2J	N1J	18	2.8	2.8	15.	6.00	6.	0.015	77.00	3.39	104.00	99.84	99.50	4.16	0.0043	100.04	99.70
		N/A				0.00		0.015	77.00	0.00	104.00	99.84	99.50	N/A	0.0043	100.04	99.70
N2k	N1k	60	62.79	62.79	15.	134.44	134.44	0.015	120.00	6.85	105.00	99.32	98.89	5.68	0.0035	100.13	99.70
		N/A				0.00		0.015	120.00	0.00	105.00	99.32	98.89	N/A	0.0035	100.13	99.70
NN7	NN6	18	0.1	0.1	13.5	0.25	0.25	0.015	518.00	8.63	97.00	88.42	88.38	8.58	0.0001	88.44	88.38

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	518.00	0.00	97.00	88.42	88.38	N/A	0.0001	88.44	88.38
NN6	NN5	18	0.1	0.2	14.5	0.48	0.48	0.015	462.00	7.70	96.00	88.38	88.37	7.62	0.0000	88.38	88.37
		N/A				0.00		0.015	462.00	0.00	96.00	88.38	88.37	N/A	0.0000	88.38	88.37
NN5	NN4	18	0.1	0.3	15.5	0.70	0.7	0.015	402.00	6.70	94.00	88.37	88.34	5.63	0.0001	88.37	88.35
		N/A				0.00		0.015	402.00	0.00	94.00	88.37	88.34	N/A	0.0001	88.37	88.35
NN4	NN3	18	0.01	0.31	16.5	0.70	0.7	0.015	438.00	7.30	93.00	88.34	88.32	4.66	0.0001	88.34	88.32
		N/A				0.00		0.015	438.00	0.00	93.00	88.34	88.32	N/A	0.0001	88.34	88.32
NN3	NN2	24	5.71	6.02	17.5	13.18	13.18	0.015	400.00	6.67	91.50	88.01	86.20	3.49	0.0045	88.32	86.51
		N/A				0.00		0.015	400.00	0.00	91.50	88.01	86.20	N/A	0.0045	88.32	86.51
NN2	NN1	24	0.1	6.12	18.5	13.03	13.03	0.015	47.00	4.15	90.00	86.21	86.00	3.79	0.0044	86.51	86.30
		N/A				0.00		0.015	47.00	0.00	90.00	86.21	86.00	N/A	0.0044	86.51	86.30
OO2	OO1	30	6.91	6.91	13.5	15.68	15.68	0.015	105.00	3.19	90.00	85.83	85.63	4.17	0.0019	86.01	85.80
		N/A				0.00		0.015	105.00	0.00	90.00	85.83	85.63	N/A	0.0019	86.01	85.80
P5	P4	36	10.66	10.66	13.5	24.71	24.71	0.015	270.00	4.50	100.50	97.54	97.05	2.96	0.0018	97.75	97.26
		N/A				0.00		0.015	270.00	0.00	100.50	97.54	97.05	N/A	0.0018	97.75	97.26
P4	P3	36	8.47	19.13	14.5	43.19	43.19	0.015	298.00	6.11	99.00	96.61	94.95	2.39	0.0056	97.26	95.59
		N/A				0.00		0.015	298.00	0.00	99.00	96.61	94.95	N/A	0.0056	97.26	95.59
P3	P2	42	8.29	27.42	15.31	60.54	60.54	0.015	346.00	6.29	97.60	94.91	93.24	2.69	0.0048	95.59	93.92
		N/A				0.00		0.015	346.00	0.00	97.60	94.91	93.24	N/A	0.0048	95.59	93.92
P2	P1	48	9.54	36.96	16.23	79.39	79.39	0.015	57.00	6.32	95.00	93.23	93.00	1.77	0.0041	93.92	93.69
		N/A				0.00		0.015	57.00	0.00	95.00	93.23	93.00	N/A	0.0041	93.92	93.69
PP3	PP2	48	19.55	19.55	15.	46.21	46.21	0.015	259.00	4.32	89.00	86.47	86.11	2.53	0.0014	86.70	86.34
		N/A				0.00		0.015	259.00	0.00	89.00	86.47	86.11	N/A	0.0014	86.70	86.34
PP2	PP1	48	0.3	19.85	16.	45.41	45.41	0.015	85.00	3.61	88.00	86.12	86.00	1.88	0.0013	86.34	86.23
		N/A				0.00		0.015	85.00	0.00	88.00	86.12	86.00	N/A	0.0013	86.34	86.23
Q7	Q6	36	8.95	8.95	13.5	20.40	20.4	0.015	418.00	6.97	106.00	101.24	100.72	4.76	0.0012	101.38	100.86
		N/A				0.00		0.015	418.00	0.00	106.00	101.24	100.72	N/A	0.0012	101.38	100.86
Q6	Q4	36	9.69	18.64	14.5	40.91	40.91	0.015	612.00	10.20	103.50	100.28	97.22	3.22	0.0050	100.86	97.79
		N/A				0.00		0.015	612.00	0.00	103.50	100.28	97.22	N/A	0.0050	100.86	97.79
Q5	Q4	36	8.34	8.34	13.5	18.40	18.4	0.015	604.00	10.07	105.00	98.29	97.68	6.71	0.0010	98.41	97.79
		N/A				0.00		0.015	604.00	0.00	105.00	98.29	97.68	N/A	0.0010	98.41	97.79
Q4	Q3	42	7.04	34.02	15.5	71.48	71.48	0.015	425.00	7.43	100.30	96.84	93.99	3.46	0.0067	97.79	94.94
		N/A				0.00		0.015	425.00	0.00	100.30	96.84	93.99	N/A	0.0067	97.79	94.94
Q3	Q2	48	14.93	48.95	16.45	99.36	99.36	0.015	380.00	7.91	96.20	93.86	91.44	2.34	0.0064	94.93	92.51
		N/A				0.00		0.015	380.00	0.00	96.20	93.86	91.44	N/A	0.0064	94.93	92.51
Q2	Q1	54	3.1	52.05	17.25	97.88	103.	0.015	108.00	6.15	92.00	91.86	91.50	0.14	0.0033	92.51	92.15
		N/A				5.12		0.015	108.00	1.80	92.00	92.50	92.34	-0.50	0.0009	92.51	92.41
QQ3	QQ2	42	0.3	0.3	15.	0.62	0.62	0.015	450.00	7.50	88.00	87.21	87.21	0.79	0.0000	87.21	87.21
		N/A				0.00		0.015	450.00	0.00	88.00	87.21	87.21	N/A	0.0000	87.21	87.21
QQ2	QQ1	42	33.85	34.15	16.	66.08	66.08	0.015	156.00	6.87	88.00	86.40	85.50	1.60	0.0057	87.21	86.31
		N/A				0.00		0.015	156.00	0.00	88.00	86.40	85.50	N/A	0.0057	87.21	86.31
R2	R1	42	14.36	14.36	13.5	32.25	32.25	0.015	129.00	3.35	94.00	91.68	91.50	2.32	0.0014	91.87	91.70
		N/A				0.00		0.015	129.00	0.00	94.00	91.68	91.50	N/A	0.0014	91.87	91.70
S5	S3	30	13.04	13.04	15.	29.45	29.45	0.015	708.00	11.80	103.00	100.53	95.67	2.47	0.0069	101.15	96.29
		N/A				0.00		0.015	708.00	0.00	103.00	100.53	95.67	N/A	0.0069	101.15	96.29
S4	S3	36	13.33	13.33	13.5	32.01	32.01	0.015	441.00	7.35	99.00	97.29	95.94	1.71	0.0031	97.64	96.29
		N/A				0.00		0.015	441.00	0.00	99.00	97.29	95.94	N/A	0.0031	97.64	96.29
S3	S2	42	8.55	34.92	14.5	79.14	80.66	0.015	724.00	12.07	96.00	95.12	89.16	0.88	0.0082	96.29	90.32
		N/A				1.53		0.015	724.00	12.07	96.00	96.23	90.32	-0.23	0.0082	96.29	90.32
S2	S1	48	6.61	41.53	15.48	91.61	92.71	0.015	75.00	7.29	89.80	89.41	89.00	0.39	0.0054	90.32	89.92
		N/A				1.08		0.015	75.00	1.25	89.80	90.32	90.22	-0.52	0.0009	90.32	90.26
SS2	SS1	30	7.63	7.63	13.5	16.19	16.19	0.015	160.00	3.30	85.00	81.85	81.51	3.15	0.0021	82.03	81.70
		N/A				0.00		0.015	160.00	0.00	85.00	81.85	81.51	N/A	0.0021	82.03	81.70
U8	U7	30	8.26	8.26	15.	19.54	19.54	0.015	350.00	5.83	131.00	128.36	127.31	2.64	0.0030	128.64	127.58
		N/A				0.00		0.015	350.00	0.00	131.00	128.36	127.31	N/A	0.0030	128.64	127.58
U7	U6	30	7.46	15.72	16.	35.99	35.99	0.015	430.00	7.33	128.00	126.65	122.24	1.35	0.0102	127.58	123.17
		N/A				0.00		0.015	430.00	0.00	128.00	126.65	122.24	N/A	0.0102	127.58	123.17
U6	U5	36	6.88	22.6	16.98	48.22	48.22	0.015	450.00	7.50	124.00	122.37	119.23	1.63	0.0070	123.17	120.04
		N/A				0.00		0.015	450.00	0.00	124.00	122.37	119.23	N/A	0.0070	123.17	120.04
U5	U4	48	6.8	29.4	17.98	59.93	59.93	0.015	500.00	8.33	123.00	119.64	118.48	3.36	0.0023	120.03	118.88
		N/A				0.00		0.015	500.00	0.00	123.00	119.64	118.48	N/A	0.0023	120.03	118.88
U4	U3	48	7.1	36.5	22.	64.39	64.39	0.015	475.00	7.92	120.00	118.42	117.15	1.58	0.0027	118.87	117.60

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	475.00	0.00	120.00	118.42	117.15	N/A	0.0027	118.87	117.60
U3	U2	48	7.69	44.19	23.	74.07	74.07	0.015	160.00	5.89	118.00	117.00	116.44	1.00	0.0035	117.60	117.03
		N/A				0.00		0.015	160.00	0.00	118.00	117.00	116.44	N/A	0.0035	117.60	117.03
U2	U1	48	0.3	44.49	23.45	73.16	73.16	0.015	130.00	5.82	117.70	116.45	116.00	1.25	0.0035	117.03	116.58
		N/A				0.00		0.015	130.00	0.00	117.70	116.45	116.00	N/A	0.0035	117.03	116.58
T2B	T1B	18	3.29	3.29	13.5	7.46	7.46	0.015	91.00	4.22	88.00	85.11	84.50	2.89	0.0067	85.42	84.81
		N/A				0.00		0.015	91.00	0.00	88.00	85.11	84.50	N/A	0.0067	85.42	84.81
T2C	T1C	30	7.05	7.05	13.5	15.94	15.94	0.015	100.00	3.25	89.00	84.70	84.50	4.30	0.0020	84.88	84.68
		N/A				0.00		0.015	100.00	0.00	89.00	84.70	84.50	N/A	0.0020	84.88	84.68
T5A	T4A	36	15.39	15.39	16.5	32.92	32.92	0.015	401.00	6.68	93.00	89.54	88.24	3.46	0.0032	89.92	88.62
		N/A				0.00		0.015	401.00	0.00	93.00	89.54	88.24	N/A	0.0032	89.92	88.62
T4A	T3A	36	7.45	22.84	17.5	46.62	46.62	0.015	419.00	6.98	91.00	87.87	85.14	3.13	0.0065	88.62	85.89
		N/A				0.00		0.015	419.00	0.00	91.00	87.87	85.14	N/A	0.0065	88.62	85.89
T3A	T2A	42	10.02	32.86	18.5	64.31	64.31	0.015	354.00	6.68	88.50	85.12	83.19	3.38	0.0054	85.89	83.96
		N/A				0.00		0.015	354.00	0.00	88.50	85.12	83.19	N/A	0.0054	85.89	83.96
T2A	T1A	48	4.71	37.57	19.38	71.37	71.37	0.015	123.00	5.68	86.00	83.41	83.00	2.59	0.0033	83.96	83.56
		N/A				0.00		0.015	123.00	0.00	86.00	83.41	83.00	N/A	0.0033	83.96	83.56
TT5	TT4	30	0.7	0.7	15.	1.67	1.67	0.015	465.00	7.75	83.00	85.01	85.03	-2.01	0.0000	85.01	85.03
		N/A				0.00		0.015	465.00	0.00	83.00	85.01	85.03	N/A	0.0000	85.01	85.03
TT4	TT2	48	6.1	16.09	16.	33.81	33.81	0.015	506.00	8.43	84.00	84.91	84.53	-0.91	0.0007	85.03	84.66
		N/A				0.00		0.015	506.00	0.00	84.00	84.91	84.53	N/A	0.0007	85.03	84.66
TT3	TT4	48	9.29	9.29	15.	19.93	20.01	0.015	243.00	4.05	85.00	85.05	84.99	-0.05	0.0003	85.09	85.03
		N/A				0.07		0.015	243.00	4.05	85.00	85.08	85.03	-0.08	0.0003	85.09	85.03
TT2	TT1	48	0.1	16.19	17.	32.92	32.92	0.015	63.00	2.62	85.00	84.05	84.00	0.95	0.0007	84.16	84.12
		N/A				0.00		0.015	63.00	0.00	85.00	84.05	84.00	N/A	0.0007	84.16	84.12
V3	V2	12	0.3	0.3	13.5	0.76	0.76	0.015	423.00	7.05	120.00	114.55	114.31	5.45	0.0006	114.58	114.33
		N/A				0.00		0.015	423.00	0.00	120.00	114.55	114.31	N/A	0.0006	114.58	114.33
V2	V1	12	0.3	0.6	14.5	1.46	1.46	0.015	55.00	1.86	119.00	114.27	114.14	4.73	0.0022	114.32	114.20
		N/A				0.00		0.015	55.00	0.00	119.00	114.27	114.14	N/A	0.0022	114.32	114.20
W2	W1	54	0.01	62.84	18.28	120.90	120.9	0.015	75.00	7.60	114.00	111.88	111.50	2.12	0.0050	112.87	112.50
		N/A				0.00		0.015	75.00	0.00	114.00	111.88	111.50	N/A	0.0050	112.87	112.50
W3	W2	54	7.5	62.83	17.91	122.28	122.28	0.015	170.00	7.69	114.50	112.73	111.86	1.77	0.0051	113.75	112.87
		N/A				0.00		0.015	170.00	0.00	114.50	112.73	111.86	N/A	0.0051	113.75	112.87
W4	W3	48	13.2	55.33	17.63	107.91	108.73	0.015	150.00	8.59	114.60	113.61	112.48	0.99	0.0075	114.88	113.75
		N/A				0.83		0.015	150.00	2.50	114.60	114.87	114.69	-0.27	0.0010	114.88	114.74
W5	W4	42	14.	42.13	16.63	50.55	85.68	0.015	425.00	7.08	115.50	115.83	114.41	-0.33	0.0034	116.31	114.88
		N/A				35.13		0.015	425.00	7.08	115.50	116.25	115.22	-0.75	0.0022	116.31	115.36
W6	W5	36	11.12	28.13	15.95	39.79	58.73	0.015	340.00	5.67	117.30	117.38	115.77	-0.08	0.0047	117.92	116.31
		N/A				18.95		0.015	340.00	5.67	117.30	117.81	116.30	-0.51	0.0047	117.92	116.31
W7	W6	30	12.69	12.69	15.	26.27	27.71	0.015	340.00	5.67	119.50	119.29	117.43	0.21	0.0055	119.78	117.93
		N/A				1.44		0.015	340.00	5.67	119.50	119.73	117.92	-0.23	0.0055	119.78	117.92
W8	W6	24	4.32	4.32	15.	8.59	9.25	0.015	680.00	11.33	119.00	119.10	117.80	-0.10	0.0019	119.23	117.92
		N/A				0.66		0.015	680.00	11.33	119.00	119.21	117.92	-0.21	0.0019	119.23	117.92
X13	X2	48	21.23	21.23	15.	48.16	48.16	0.015	337.00	5.62	113.50	113.42	112.92	0.08	0.0015	113.67	113.17
		N/A				0.00		0.015	337.00	0.00	113.50	113.42	112.92	N/A	0.0015	113.67	113.17
X3	X2	42	15.5	30.82	18.86	64.91	64.91	0.015	315.00	6.75	115.50	114.13	112.39	1.37	0.0055	114.91	113.17
		N/A				0.00		0.015	315.00	0.00	115.50	114.13	112.39	N/A	0.0055	114.91	113.17
X4	X3	36	5.1	15.32	18.	33.04	33.04	0.015	242.00	4.67	116.50	115.33	114.54	1.17	0.0033	115.71	114.92
		N/A				0.00		0.015	242.00	0.00	116.50	115.33	114.54	N/A	0.0033	115.71	114.92
X5	X4	36	0.01	10.22	17.	22.69	22.69	0.015	272.00	4.53	117.30	115.95	115.53	1.35	0.0015	116.13	115.71
		N/A				0.00		0.015	272.00	0.00	117.30	115.95	115.53	N/A	0.0015	116.13	115.71
X6	X5	36	5.4	10.21	16.	23.37	23.37	0.015	307.00	5.12	120.00	116.44	115.94	3.56	0.0016	116.63	116.13
		N/A				0.00		0.015	307.00	0.00	120.00	116.44	115.94	N/A	0.0016	116.63	116.13
X7	X6	30	4.81	4.81	15.	11.38	11.38	0.015	400.00	6.67	122.00	116.95	116.54	5.05	0.0010	117.04	116.63
		N/A				0.00		0.015	400.00	0.00	122.00	116.95	116.54	N/A	0.0010	117.04	116.63
X12	X11	24	0.3	0.3	15.	0.71	0.71	0.015	354.00	5.90	122.50	113.88	113.88	8.62	0.0000	113.88	113.88
		N/A				0.00		0.015	354.00	0.00	122.50	113.88	113.88	N/A	0.0000	113.88	113.88
X11	X10	30	0.3	0.6	16.	1.37	1.37	0.015	918.00	15.30	121.50	113.88	113.86	7.62	0.0000	113.88	113.86
		N/A				0.00		0.015	918.00	0.00	121.50	113.88	113.86	N/A	0.0000	113.88	113.86
X10	X9	30	0.3	0.9	17.	2.00	2.	0.015	260.00	4.33	121.00	113.86	113.85	7.14	0.0000	113.86	113.85
		N/A				0.00		0.015	260.00	0.00	121.00	113.86	113.85	N/A	0.0000	113.86	113.85
X9	X8	36	7.5	8.4	18.	18.12	18.12	0.015	260.00	4.33	119.00	113.74	113.48	5.26	0.0010	113.85	113.60

Up Node Name	Down Node Name	Convey Size (in or ft)	Trib. Area at Up Node (acre)	Cumul. Area (acre)	Total Computed Response Time at Up Node (min.)	Computed Flow at Convey (cfs)	Total Flow at Convey (cfs)	Convey 'n' Value	Total Length (feet)	Average Velocity (fps)	Rim/Grate at Up Node (feet)	Up HGL (feet)	Down HGL (feet)	Up (RIM-HGL) (feet)	Energy Slope (ft/ft)	Up EGL (feet)	Down EGL (feet)
		N/A				0.00		0.015	260.00	0.00	119.00	113.74	113.48	N/A	0.0010	113.85	113.60
X8	X2	42	7.	15.4	19.	32.32	32.32	0.015	310.00	5.17	116.00	113.40	112.97	2.60	0.0014	113.59	113.17
		N/A				0.00		0.015	310.00	0.00	116.00	113.40	112.97	N/A	0.0014	113.59	113.17
X2	X1	60	0.3	67.75	19.64	137.73	137.73	0.015	354.00	7.01	114.50	112.32	111.00	2.18	0.0037	113.17	111.85
		N/A				0.00		0.015	354.00	0.00	114.50	112.32	111.00	N/A	0.0037	113.17	111.85
Z2	Z1	48	20.19	20.19	13.5	50.27	50.27	0.015	73.00	4.00	111.00	108.12	108.00	2.88	0.0016	108.40	108.28
		N/A				0.00		0.015	73.00	0.00	111.00	108.12	108.00	N/A	0.0016	108.40	108.28

APPENDIX H

STORMWATER QUALITY CALCULATIONS

LID CALCULATIONS

On-site drainage improvements consist of a combination of conventional subsurface and surface drainage systems, construction of pipe conveyance systems, and construction of culverts and bridges at roadway and trail crossings of creeks and tributaries. Stormwater will be discharged through outfalls into open space corridors. Vegetated swales, soft armoring, mechanical storm filters, structural interceptors and other best management practices will be utilized at pipe outfalls or other appropriate locations for water quality management, and to convey stormwater runoff to receiving waters while minimizing impacts to open space resources. All outfall structures will be extended past any bikeways that are constructed within the open spaces.

The number and location of outfalls in Figure 8-4 is based on the best available information and is subject to refinement during the subdivision map and improvement plan approvals, as well as state/federal permitting. Drainage facilities will be designed and constructed in conformance with the City of Roseville Improvement Standards, the City's Stormwater Quality Design Manual, the Placer County Flood Control Agency's Stormwater Management Manual, and the open space preserve Operations and Maintenance (O&M) Plan, the preparation of which is a requirement of the Clean Water Act 404 permit. The Preserve O&M Plan will include requirements to minimize erosion and direct drainage away from vernal pool habitat by employing conceptual drainage improvements to be outlined in the plan (swales, outfalls, energy dissipation, and erosion control).

Specific detail regarding the project drainage system is contained in the Drainage and Storm Water Master Plan on file with the City.

8.6 Stormwater Quality

Sierra Vista provides a comprehensive plan for the management of urban runoff for flow control and water quality improvement. The integrated stormwater management plan, which is reflected within the specific design criteria contained in this section. The objectives of the Sierra Vista stormwater management plan (SWMP) are intended to fulfill the requirements of the City's National Pollutant Discharge Elimination System (NPDES) Phase II Permit, as issued by the State Water Resources Control Board, and to minimize the effects of urban storm water runoff on the natural open space areas, including wetland areas and principal drainage corridors.

The Sierra Vista SWMP will be in accordance with the then current permit criteria applicable at the time of development. The SWMP provides the frame work for stormwater treatment during two distinct components of the development process, first, during the construction phase while infrastructure is being built to support the community, and the post construction phase which will be part of the improvements that make up the community and continue to protect the natural resources in perpetuity.

The Sierra Vista SWMP will be in accordance with the then current permit criteria applicable at the time of development. The SWMP provides the frame work for stormwater treatment during two district

components of the development process, first, during the construction phase while infrastructure is being built to support the community, and the post construction phase which will be part of the improvements that make up the community and continue to protect the natural resources in perpetuity.

A. Stormwater Management During Construction Activities

The release of on-site stormwater runoff during Construction activities is regulated by the State General Construction Permit issued by the Regional Water Quality Control Board for all construction sites greater than one acre. The General Construction permit requires that a Storm Water Pollution Prevention Plan (SWPPP) is created to address how the storm water from a particular construction site will be maintained and treated prior to being discarded from the site. The SWPPP is an evolving document that changes with the dynamics of the site development.

The use of Best Management Practices (BMPs) during the construction process will generally incorporate erosion controls and sediment controls. Erosion and sediment control BMPs include such things as applying straw mulch to disturbed areas, the use of fiber rolls and silt fences, sedimentation basins, drain inlet protection, stabilized construction accesses, and material management. The final sizing and selection of BMPs will consider requirements specific to the Curry Creek watershed and proposed developed activities.

B. Post Construction Stormwater Management

Post construction stormwater management is intended to treat the urban runoff generated on-site in perpetuity. The BMP techniques within the plan area will reduce and/or eliminate the pollutants from the urban stormwater runoff and prevent the contamination of receiving waters. Sierra Vista will work with the then current permit criteria applicable at the time of development and in conformance with the City of Roseville Improvement Standards, the City's Stormwater Quality Design Manual, the Placer County Flood Control Agency's Stormwater Management Manual, the open space preserve Operations and Maintenance (O&M) Plan, to design and address post construction stormwater treatment.

Post construction stormwater treatment is composed of three general elements, source control, runoff reduction and treatment of runoff. All three elements will be used in the Sierra Vista SWMP. The basic practice of source control is to minimize the potential for constituents to enter runoff at the source. The main tool the project will employ towards the goal of runoff reduction, is the use of Low Impact Development(LID) measures. Implementation of LID includes the construction of decentralized small scale improvements that provide for local infiltration and treatment opportunities that reduce the quantity of runoff which enters the storm drain systems during a rainfall event. LID will be implemented to offset for runoff increases that occur with the development as a matter of the conversion of native ground surfaces to impervious cover. Additional Treatment control BMPs will be located at the end of the pipe and provide further treatment of the stormwater before it enters into the natural creek system.

Low Impact Development (LID)

Low impact development (LID) is an approach to stormwater management that emphasizes the use of small-scale, natural, constructed and proprietary drainage features integrated throughout the city to capture urban runoff and precipitation. LID measures can slow, clean, infiltrate and evapotranspire runoff, which reduce the quantity of urban runoff entering the city storm drain systems. The added opportunities for infiltration offered by the use of LID can add water to local aquifers, increasing water reuse. It is a sustainable practice that benefits water quality protection, stream stability and can contribute to water supply. The intent is to weave the textures of natural processes into the fabric of development. Unlike traditional storm water management, which collects and conveys storm water runoff through storm drains, pipes, or other conveyances to a centralized storm water facility, LID within Sierra Vista will take a different approach by using site design elements, LID and storm water management to minimize changes to the site's pre-development runoff rates and volumes. Sierra Vista's LID elements are will assist with the goal of mimicing the site's predevelopment hydrology by using design techniques that infiltrate, filter, store, evaporate, and detain runoff close to the source of rainfall.

Key principles of low impact development include:

- Decentralize and manage urban runoff to integrate storm water management throughout the watershed
- Preserve the ecosystem's natural hydrological functions and cycles.
- Account for a site's topographic features in its design.
- Reduce directly connected impervious surfaces to slow runoff and provide additional infiltration opportunities.
- Reduce impervious ground cover and maximize infiltration on-site.

As Sierra Vista develops, specific LID techniques, tools, and material will be used and specified in the construction documents that will control the amount of impervious surface, increase infiltration, and improve water quality by reducing runoff from the developed sites. The Sierra Vista master drainage study has accounted for the percent of runoff reduction expected with the implementation of these LID practices. "Additional project design elements within the open space areas will also provide hydrograph modification benefits. The created wetland elements will provide additional floodplain storage capacity which is factored into the project hydrology analysis. The created wetlands also provide LID and treatment potential which has not been factored into the project mitigation, which include: added infiltration opportunities, evapotranspiration opportunities, nutrient uptake, biological filtering, and stream buffers."

LID elements within Sierra Vista may include

- Disconnected roof drains
- Disconnected and separated pavement
- Bioretention facilities, rain gardens, and bioswales
- Tree Planting
- Grass swales and channels,

- Curb cuts and vegetated filter strips,
- Impervious surface reduction – permeable pavements and porous pavements,
- Stream Buffers
- Soil Amendments
- Pollution prevention and good housekeeping.

PROVIDE GRAPHICS OF EXAMPLE BMP'S HERE

The State is beginning to draft new requirements to the Storm Water Management permits. At the time that Sierra Vista develops it is anticipated that the NPDES permit will include requirements that hydrograph modification impacts be addressed to some level". The project drainage study includes a section addressing the potential hydrograph modification impacts of this project, and the net impacts of the project with the mitigations proposed.

End of Pipe Stormwater Treatment Control

In addition to the implementation of the above referenced LID measures, the storm drain system will be designed to provide additional protection of the natural environment and receiving water of Curry Creek by providing end of pipe treatment techniques. This element adds to the treatment train and consists treatment elements such as structural BMP's or Grass treatment swales.

Special consideration will be taken within Sierra Vista to capture, convey and release the urban storm water to the creek system. The treatment and conveyance of storm runoff in and through the open spaces will be made part of the Corp of Engineers 404 permitting process. Standard practices include the use of headwall structures, directly at the outfall location, that stabilize and protect the outlet pipe, surrounding topography and aid in velocity attenuation while minimizing future maintenance costs. Conveyance "grassy swales" that direct the storm water from the pipe outlet to the receiving waters, while avoiding sensitive habitat and distributing the concentrated pipe flows to a spread sheet flow will be used at every outfall in the project. Depending on the size of and frequency of particular storm events, and the actual drainage area being conveyed, the conveyance swales will be armored with geosynthetics that will minimize the potential for future erosion and rilling of the open space. This soft armoring approach will provide opportunity to create grassy swales and additional wetlands that will aide in storm water filtration and infiltration. Additional structural BMP's can be added to the treatment train and end of pipe treatment as needed, they include such devices as:

Installation of "fossil filter" or equivalent petroleum absorbing insert assemblies in the project drop inlets;

Trash screen vaults;

Structural BMPs such as vortex devices;

The final selection of BMPs will consider requirements specific to the Curry Creek watershed and proposed development flows. Other BMPs will involve prompt re-vegetation of disturbed areas and proper erosion protection per the NPDES permit during construction. The end of pipe BMP's will be designed with complete redundancy as if no other stormwater quality improvements or LID measures

were to be implemented (City requirement). Upon exiting the Sierra Vista Treatment Train, runoff will have been reduced, controlled and treated in excess of the minimum standards.

Post-Construction Water Balance Calculator

1	Post-Construction Water Balance Calculator											
2												
3	User may make changes from any cell that is orange or brown in color (similar to the cells to the immediate right). Cells in green are calculated for you.		NON-SWALE LID RVR V	(Step 1a) If you know the 85th percentile storm event for your location enter it in the box below		(Step 1b) If you can not answer 1a then select the county where the project is located (click on the cell to the right for drop-down): This will determine the average 85th percentile 24 hr. storm event for your site, which will appear under precipitation to left.		PLACER				
4			71.3879453			(Step 1c) If you would like a more precise value select the location closest to your site. If you do not recognize any of these locations, leave this drop-down menu at location. The average value for the County will be used.		SACRAMENTO FAA ARPT				
5	Project Information				Runoff Calculations							
6	Project Name:		Sierra Vista Specific Plan		(Step 2) Indicate the Soil Type (dropdown menu to right):		Group D Soils	Very low infiltration. Clay loam, silty clay loam, sandy clay, silty clay, or clay. Infiltration rate 0 to 0.05 inch/hr when wet.				
7	Waste Discharge Identification (WDID):		Curry Creek		(Step 3) Indicate the existing dominant non-built land Use Type (dropdown menu to right):		Pasture/Grassland/Range: 50% to 75% ground cover & not heavily grazed					
8	Date:		9-Jun-08		(Step 4) Indicate the proposed dominant non-built land Use Type (dropdown menu to right):		A mix of lawn, grass, pasture and trees covering less than 50% of the open space					
9	Sub Drainage Area Name (from map):		SAMPLE PQP				Complete Either					
10	Runoff Curve Numbers						Sq Ft	Acres	Acres			
11	Existing Pervious Runoff Curve Number		84		(Step 5) Total Project Site Area:			100.00	100.00			
12	Proposed Development Pervious Runoff Curve Number		96		(Step 6) Sub-watershed Area:			100.00	100.00			
13	Design Storm				Percent of total project :		100%					
14	Based on the County you indicated above, we have included the 85 percentile average 24 hr event - P85 (in)^ for your area.		0.62	in								
15	The Amount of rainfall needed for runoff to occur (Existing runoff curve number -P from existing RCN (in)^)		0.38	in	(Step 7) Sub-watershed Conditions		Complete Either		Calculated Acres			
16	P used for calculations (in) (the greater of the above two criteria)		0.62	in	Sub-watershed Area (acres)		Sq Ft	Acres	100.00			
17	^Available at www.cabmphandbooks.com				Existing Rooftop Impervious Coverage			0	0.00			
18					Existing Non-Rooftop Impervious Coverage			0	0.00			
19					Proposed Rooftop Impervious Coverage			0	0.00			
20					Proposed Non-Rooftop Impervious Coverage			85	85.00			
21					Credits		Acres	Square Feet				
22					Porous Pavement		0.00	0				
23					Tree Planting		3.75	163,350				
24												
25	Pre-Project Runoff Volume (cu ft)		9,770		Cu.Ft.		Downspout Disconnection	0.00	0			
26	Project-Related Runoff Volume Increase w/o credits (cu ft)		149,374		Cu.Ft.		Impervious Area Disconnection	42.50	1,851,300			
27							Green Roof	0.00	0			
28							Stream Buffer	0.00	0			
29							Vegetated Swales	37.78	1,645,697			
30	Project-Related Volume Increase with Credits (cu ft)		-27,658		Cu.Ft.		Subtotal	84.03	3,660,347			
31					Subtotal Runoff Volume Reduction Credit		151674 Cu. Ft.					
32	You have achieved your minimum requirements						(Step 9) Impervious Volume Reduction Credits		Volume (cubic feet)			
33									Rain Barrels/Cisterns	0	Cu. Ft.	
34									Soil Quality	25,358	Cu. Ft.	
35									Subtotal Runoff Volume Reduction	25,358	Cu. Ft.	
36									Total Runoff Volume Reduction Credit		177,032 Cu. Ft.	
37												
38												
39												

Post-Construction Water Balance Calculator

1	Post-Construction Water Balance Calculator											
2												
3	User may make changes from any cell that is orange or brown in color (similar to the cells to the immediate right). Cells in green are calculated for you.		NON-SWALE LID RVR V	(Step 1a) If you know the 85th percentile storm event for your location enter it in the box below		(Step 1b) If you can not answer 1a then select the county where the project is located (click on the cell to the right for drop-down): This will determine the average 85th percentile 24 hr. storm event for your site, which will appear under precipitation to left.		PLACER				
4			81.61538034			(Step 1c) If you would like a more precise value select the location closest to your site. If you do not recognize any of these locations, leave this drop-down menu at location. The average value for the County will be used.		SACRAMENTO FAA ARPT				
5	Project Information				Runoff Calculations							
6	Project Name:		Sierra Vista Specific Plan		(Step 2) Indicate the Soil Type (dropdown menu to right):		Group D Soils	Very low infiltration. Clay loam, silty clay loam, sandy clay, silty clay, or clay. Infiltration rate 0 to 0.05 inch/hr when wet.				
7	Waste Discharge Identification (WVID):		Curry Creek		(Step 3) Indicate the existing dominant non-built land Use Type (dropdown menu to right):		Pasture/Grassland/Range: 50% to 75% ground cover & not heavily grazed					
8	Date:		9-Jun-08		(Step 4) Indicate the proposed dominant non-built land Use Type (dropdown menu to right):		A mix of lawn, grass, pasture and trees covering less than 50% of the open space					
9	Sub Drainage Area Name (from map):		SAMPLE PQP				Complete Either					
10	Runoff Curve Numbers						Sq Ft	Acres	Acres			
11	Existing Pervious Runoff Curve Number		84		(Step 5) Total Project Site Area:			100.00	100.00			
12	Proposed Development Pervious Runoff Curve Number		91		(Step 6) Sub-watershed Area:			100.00	100.00			
13	Design Storm				Percent of total project :		100%					
14	Based on the County you indicated above, we have included the 85 percentile average 24 hr event - P85 (in)^ for your area.		0.62	in								
15	The Amount of rainfall needed for runoff to occur (Existing runoff curve number -P from existing RCN (in)^)		0.38	in	(Step 7) Sub-watershed Conditions		Complete Either		Calculated Acres			
16	P used for calculations (in) (the greater of the above two criteria)		0.62	in	Sub-watershed Area (acres)		Sq Ft	Acres	100.00			
17	^Available at www.cabmphandbooks.com				Existing Rooftop Impervious Coverage			0	0.00			
18					Existing Non-Rooftop Impervious Coverage			0	0.00			
19					Proposed Rooftop Impervious Coverage			16	16.00			
20					Proposed Non-Rooftop Impervious Coverage			24	24.00			
21					Credits		Acres		Square Feet			
22					Porous Pavement		0.00	0				
23					Tree Planting		3.75	163,350				
24												
25	Pre-Project Runoff Volume (cu ft)		9,770	Cu.Ft.	Downspout Disconnection		8.00	348,480				
26	Project-Related Runoff Volume Increase w/o credits (cu ft)		90,024	Cu.Ft.	Impervious Area Disconnection		4.80	209,088				
27					Green Roof		0.00	0				
28					Stream Buffer		0.00	0				
29					Vegetated Swales		22.07	961,369				
30	Project-Related Volume Increase with Credits (cu ft)		-25,939	Cu.Ft.	Subtotal		38.62	1,682,287				
31					Subtotal Runoff Volume Reduction Credit		79737 Cu. Ft.					
32	You have achieved your minimum requirements				(Step 9) Impervious Volume Reduction Credits		Volume (cubic feet)					
33							Rain Barrels/Cisterns		0	Cu. Ft.		
34							Soil Quality		36,226	Cu. Ft.		
35							Subtotal Runoff Volume Reduction		36,226	Cu. Ft.		
36							Total Runoff Volume Reduction Credit		115,963	Cu. Ft.		

Post-Construction Water Balance Calculator

1	Post-Construction Water Balance Calculator													
2														
3	User may make changes from any cell that is orange or brown in color (similar to the cells to the immediate right). Cells in green are calculated for you.		NON-SWALE LID RVR V		(Step 1a) If you know the 85th percentile storm event for your location enter it in the box below		(Step 1b) If you can not answer 1a then select the county where the project is located (click on the cell to the right for drop-down): This will determine the average 85th percentile 24 hr. storm event for your site, which will appear under precipitation to left.		PLACER					
4			99.96558827				(Step 1c) If you would like a more precise value select the location closest to your site. If you do not recognize any of these locations, leave this drop-down menu at location. The average value for the County will be used.		SACRAMENTO FAA ARPT					
5	Project Information				Runoff Calculations									
6	Project Name:		Sierra Vista Specific Plan		(Step 2) Indicate the Soil Type (dropdown menu to right):		Group D Soils	Very low infiltration. Clay loam, silty clay loam, sandy clay, silty clay, or clay. Infiltration rate 0 to 0.05 inch/hr when wet.						
7	Waste Discharge Identification (WVID):		Curry Creek		(Step 3) Indicate the existing dominant non-built land Use Type (dropdown menu to right):		Pasture/Grassland/Range: 50% to 75% ground cover & not heavily grazed							
8	Date:		9-Jun-08		(Step 4) Indicate the proposed dominant non-built land Use Type (dropdown menu to right):		A mix of lawn, grass, pasture and trees covering less than 50% of the open space							
9	Sub Drainage Area Name (from map):		SAMPLE PARK				Complete Either							
10	Runoff Curve Numbers						Sq Ft	Acres	Acres					
11	Existing Pervious Runoff Curve Number		84		(Step 5) Total Project Site Area:		100.00	100.00	100.00					
12	Proposed Development Pervious Runoff Curve Number		87		(Step 6) Sub-watershed Area:		100.00	100.00	100.00					
13	Design Storm				Percent of total project :		100%							
14	Based on the County you indicated above, we have included the 85 percentile average 24 hr event - P85 (in)^ for your area.		0.62	in										
15	The Amount of rainfall needed for runoff to occur (Existing runoff curve number -P from existing RCN (in)^)		0.38	in	(Step 7) Sub-watershed Conditions		Complete Either		Calculated Acres					
16	P used for calculations (in) (the greater of the above two criteria)		0.62	in	Sub-watershed Area (acres)		Sq Ft	Acres	100.00					
17	^Available at www.cabmphandbooks.com				Existing Rooftop Impervious Coverage		0	0	0.00					
18							Existing Non-Rooftop Impervious Coverage		0	0	0.00			
19					Proposed Rooftop Impervious Coverage		1	1	1.00					
20					Proposed Non-Rooftop Impervious Coverage		4	4	4.00					
21					Credits		Acres		Square Feet					
22					Porous Pavement		0.00	0	0					
23					Tree Planting		1.88	81,893	81,893					
24														
25	Pre-Project Runoff Volume (cu ft)		9,770	Cu.Ft.	Downspout Disconnection		0.50	21,780	21,780					
26	Project-Related Runoff Volume Increase w/o credits (cu ft)		27,479	Cu.Ft.	Impervious Area Disconnection		0.80	34,848	34,848					
27					Green Roof		0.00	0	0					
28					Stream Buffer		0.50	21,780	21,780					
29					Vegetated Swales		1.54	67,082	67,082					
30	Project-Related Volume Increase with Credits (cu ft)		8,805	Cu.Ft.	Subtotal		5.22	227,383	227,383					
31					Subtotal Runoff Volume Reduction Credit		11429 Cu. Ft.		11429 Cu. Ft.					
32	You need to do more impervious area reduction to meet minimum requirements				(Step 9) Impervious Volume Reduction Credits		Volume (cubic feet)							
33									Rain Barrels/Cisterns		0	Cu. Ft.	0	
34									Soil Quality		7,245	Cu. Ft.	7,245	
35									Subtotal Runoff Volume Reduction		7,245	Cu. Ft.	7,245	
36									Total Runoff Volume Reduction Credit		18,674	Cu. Ft.	18,674	

Post-Construction Water Balance Calculator

1	Post-Construction Water Balance Calculator										
2	User may make changes from any cell that is orange or brown in color (similar to the cells to the immediate right). Cells in green are calculated for you.		NON-SWALE LID RVR V	(Step 1a) If you know the 85th percentile storm event for your location enter it in the box below	(Step 1b) If you can not answer 1a then select the county where the project is located (click on the cell to the right for drop-down): This will determine the average 85th percentile 24 hr. storm event for your site, which will appear under precipitation to left.	PLACER					
3			76.59388265	(Step 1c) If you would like a more precise value select the location closest to your site. If you do not recognize any of these locations, leave this drop-down menu at location. The average value for the County will be used.	SACRAMENTO FAA ARPT						
4			Project Information		Runoff Calculations						
5	Project Name:	Sierra Vista Specific Plan		(Step 2) Indicate the Soil Type (dropdown menu to right):	Group D Soils	Very low infiltration. Clay loam, silty clay loam, sandy clay, silty clay, or clay. Infiltration rate 0 to 0.05 inch/hr when wet.					
6	Waste Discharge Identification (WDID):	Curry Creek		(Step 3) Indicate the existing dominant non-built land Use Type (dropdown menu to right):	Pasture/Grassland/Range: 50% to 75% ground cover & not heavily grazed						
7	Date:	9-Jun-08		(Step 4) Indicate the proposed dominant non-built land Use Type (dropdown menu to right):	A mix of lawn, grass, pasture and trees covering less than 50% of the open space						
8	Sub Drainage Area Name (from map):	SAMPLE MDR				Complete Either					
9	Runoff Curve Numbers					Sq Ft	Acres	Acres			
10	Existing Pervious Runoff Curve Number		84	(Step 5) Total Project Site Area:		100.00	100.00				
11	Proposed Development Pervious Runoff Curve Number		92	(Step 6) Sub-watershed Area:		100.00	100.00				
12	Design Storm			Percent of total project :		100%					
13	Based on the County you indicated above, we have included the 85 percentile average 24 hr event - P85 (in)^ for your area.		0.62	in							
14	The Amount of rainfall needed for runoff to occur (Existing runoff curve number -P from existing RCN (in)^)		0.38	in	(Step 7) Sub-watershed Conditions		Complete Either		Calculated Acres		
15	P used for calculations (in) (the greater of the above two criteria)		0.62	in	Sub-watershed Area (acres)		Sq Ft	Acres	100.00		
16	^Available at www.cabmphandbooks.com				Existing Rooftop Impervious Coverage		0	0.00			
17					Existing Non-Rooftop Impervious Coverage		0	0.00			
18					Proposed Rooftop Impervious Coverage		20	20.00			
19					Proposed Non-Rooftop Impervious Coverage		30	30.00			
20					Credits		Acres		Square Feet		
21					Porous Pavement		0.00		0		
22					Tree Planting		3.75		163,350		
23	Pre-Project Runoff Volume (cu ft)		9,770	Cu.Ft.	Downspout Disconnection		16.00		696,960		
24	Project-Related Runoff Volume Increase w/o credits (cu ft)		105,634	Cu.Ft.	Impervious Area Disconnection		2.40		104,544		
25					Green Roof		0.00		0		
26					Stream Buffer		0.00		0		
27					Vegetated Swales		26.48		1,153,469		
28	Project-Related Volume Increase with Credits (cu ft)		-29,366	Cu.Ft.	Subtotal		48.63		2,118,323		
29					Subtotal Runoff Volume Reduction Credit		98774 Cu. Ft.				
30	You have achieved your minimum requirements				(Step 9) Impervious Volume Reduction Credits		Volume (cubic feet)				
31					Rain Barrels/Cisterns		0 Cu. Ft.				
32					Soil Quality		36,226 Cu. Ft.				
33					Subtotal Runoff Volume Reduction		36,226 Cu. Ft.				
34					Total Runoff Volume Reduction Credit		135,000 Cu. Ft.				

Post-Construction Water Balance Calculator

1	Post-Construction Water Balance Calculator													
2														
3	User may make changes from any cell that is orange or brown in color (similar to the cells to the immediate right). Cells in green are calculated for you.		NON-SWALE LID RVR V	(Step 1a) If you know the 85th percentile storm event for your location enter it in the box below		(Step 1b) If you can not answer 1a then select the county where the project is located (click on the cell to the right for drop-down): This will determine the average 85th percentile 24 hr. storm event for your site, which will appear under precipitation to left.		PLACER						
4			88.50830604			(Step 1c) If you would like a more precise value select the location closest to your site. If you do not recognize any of these locations, leave this drop-down menu at location. The average value for the County will be used.		SACRAMENTO FAA ARPT						
5	Project Information				Runoff Calculations									
6	Project Name:		Sierra Vista Specific Plan		(Step 2) Indicate the Soil Type (dropdown menu to right):		Group D Soils	Very low infiltration. Clay loam, silty clay loam, sandy clay, silty clay, or clay. Infiltration rate 0 to 0.05 inch/hr when wet.						
7	Waste Discharge Identification (WDID):		Curry Creek		(Step 3) Indicate the existing dominant non-built land Use Type (dropdown menu to right):		Pasture/Grassland/Range: 50% to 75% ground cover & not heavily grazed							
8	Date:		9-Jun-08		(Step 4) Indicate the proposed dominant non-built land Use Type (dropdown menu to right):		A mix of lawn, grass, pasture and trees covering less than 50% of the open space							
9	Sub Drainage Area Name (from map):		SAMPLE LDR				Complete Either							
10	Runoff Curve Numbers						Sq Ft	Acres	Acres					
11	Existing Pervious Runoff Curve Number		84		(Step 5) Total Project Site Area:			100.00	100.00					
12	Proposed Development Pervious Runoff Curve Number		91		(Step 6) Sub-watershed Area:			100.00	100.00					
13	Design Storm				Percent of total project :		100%							
14	Based on the County you indicated above, we have included the 85 percentile average 24 hr event - P85 (in)^ for your area.		0.62	in										
15	The Amount of rainfall needed for runoff to occur (Existing runoff curve number -P from existing RCN (in)^)		0.38	in	(Step 7) Sub-watershed Conditions		Complete Either		Calculated Acres					
16	P used for calculations (in) (the greater of the above two criteria)		0.62	in	Sub-watershed Area (acres)		Sq Ft	Acres	100.00					
17	^Available at www.cabmphandbooks.com				Existing Rooftop Impervious Coverage			0	0.00					
18					Existing Non-Rooftop Impervious Coverage			0	0.00					
19					Proposed Rooftop Impervious Coverage			13.32	13.32					
20					Proposed Non-Rooftop Impervious Coverage			26.64	26.64					
21					Credits									
22					Porous Pavement			0.00	0					
23					Tree Planting			4.50	196,020					
24					Downspout Disconnection			12.65	551,034					
25	Pre-Project Runoff Volume (cu ft)		9,770	Cu.Ft.										
26	Project-Related Runoff Volume Increase w/o credits (cu ft)		89,980	Cu.Ft.	Impervious Area Disconnection			2.13	92,783					
27					Green Roof			0.00	0					
28					Stream Buffer			0.00	0					
29					Vegetated Swales			19.30	840,708					
30	Project-Related Volume Increase with Credits (cu ft)		-25,901	Cu.Ft.	Subtotal			38.58	1,680,545					
31					Subtotal Runoff Volume Reduction Credit				79655 Cu. Ft.					
32	You have achieved your minimum requirements				(Step 9) Impervious Volume Reduction Credits		Volume (cubic feet)							
33							Rain Barrels/Cisterns			0	Cu. Ft.			
34							Soil Quality			36,226	Cu. Ft.			
35									Subtotal Runoff Volume Reduction			36,226	Cu. Ft.	
36									Total Runoff Volume Reduction Credit			115,881	Cu. Ft.	

Post-Construction Water Balance Calculator

1	Post-Construction Water Balance Calculator												
2													
3	User may make changes from any cell that is orange or brown in color (similar to the cells to the immediate right). Cells in green are calculated for you.		NON-SWALE LID RVR V	(Step 1a) If you know the 85th percentile storm event for your location enter it in the box below		(Step 1b) If you can not answer 1a then select the county where the project is located (click on the cell to the right for drop-down): This will determine the average 85th percentile 24 hr. storm event for your site, which will appear under precipitation to left.		PLACER					
4			70.79997123			(Step 1c) If you would like a more precise value select the location closest to your site. If you do not recognize any of these locations, leave this drop-down menu at location. The average value for the County will be used.		SACRAMENTO FAA ARPT					
5	Project Information				Runoff Calculations								
6	Project Name:		Sierra Vista Specific Plan		(Step 2) Indicate the Soil Type (dropdown menu to right):		Group D Soils	Very low infiltration. Clay loam, silty clay loam, sandy clay, silty clay, or clay. Infiltration rate 0 to 0.05 inch/hr when wet.					
7	Waste Discharge Identification (WDID):		Curry Creek		(Step 3) Indicate the existing dominant non-built land Use Type (dropdown menu to right):		Pasture/Grassland/Range: 50% to 75% ground cover & not heavily grazed						
8	Date:		9-Jun-08		(Step 4) Indicate the proposed dominant non-built land Use Type (dropdown menu to right):		A mix of lawn, grass, pasture and tress covering less than 50% of the open space						
9	Sub Drainage Area Name (from map):		SAMPLE HDR				Complete Either						
10	Runoff Curve Numbers						Sq Ft	Acres	Acres				
11	Existing Pervious Runoff Curve Number		84		(Step 5) Total Project Site Area:			100.00	100.00				
12	Proposed Development Pervious Runoff Curve Number		94		(Step 6) Sub-watershed Area:			100.00	100.00				
13	Design Storm				Percent of total project :		100%						
14	Based on the County you indicated above, we have included the 85 percentile average 24 hr event - P85 (in)^ for your area.		0.62	in									
15	The Amount of rainfall needed for runoff to occur (Existing runoff curve number -P from existing RCN (in)^)		0.38	In	(Step 7) Sub-watershed Conditions		Complete Either		Calculated Acres				
16	P used for calculations (in) (the greater of the above two criteria)		0.62	In	Sub-watershed Area (acres)		Sq Ft	Acres	100.00				
17	^Available at www.cabmphandbooks.com				Existing Rooftop Impervious Coverage			0	0.00				
18					Existing Non-Rooftop Impervious Coverage			0	0.00				
19					Proposed Rooftop Impervious Coverage			30	30.00				
20					Proposed Non-Rooftop Impervious Coverage			35	35.00				
21					Credits		Acres		Square Feet				
22					Porous Pavement		0.00	0					
23					Tree Planting		7.50	326,700					
24													
25	Pre-Project Runoff Volume (cu ft)		9,770	Cu.Ft.	Downspout Disconnection		15.00	653,400					
26	Project-Related Runoff Volume Increase w/o credits (cu ft)		128,883	Cu.Ft.	Impervious Area Disconnection		5.25	228,690					
27					Green Roof		0.00	0					
28					Stream Buffer		0.00	0					
29					Vegetated Swales		35.87	1,562,497					
30	Project-Related Volume Increase with Credits (cu ft)		-29,519	Cu.Ft.	Subtotal		63.62	2,771,287					
31					Subtotal Runoff Volume Reduction Credit		122176 Cu. Ft.						
32	You have achieved your minimum requirements				(Step 9) Impervious Volume Reduction Credits		Volume (cubic feet)						
33					Rain Barrels/Cisterns		0	Cu. Ft.					
34					Soil Quality		36,226	Cu. Ft.					
35					Subtotal Runoff Volume Reduction		36,226	Cu. Ft.					
36					Total Runoff Volume Reduction Credit		158,402	Cu. Ft.					

Post-Construction Water Balance Calculator

1	Post-Construction Water Balance Calculator											
2												
3	User may make changes from any cell that is orange or brown in color (similar to the cells to the immediate right). Cells in green are calculated for you.		NON-SWALE LID RVR V		(Step 1a) If you know the 85th percentile storm event for your location enter it in the box below		(Step 1b) If you can not answer 1a then select the county where the project is located (click on the cell to the right for drop-down): This will determine the average 85th percentile 24 hr. storm event for your site, which will appear under precipitation to left.		PLACER			
4			74.21563317				(Step 1c) If you would like a more precise value select the location closest to your site. If you do not recognize any of these locations, leave this drop-down menu at location. The average value for the County will be used.		SACRAMENTO FAA ARPT			
5	Project Information				Runoff Calculations							
6	Project Name:		Sierra Vista Specific Plan		(Step 2) Indicate the Soil Type (dropdown menu to right):		Group D Soils		Very low infiltration. Clay loam, silty clay loam, sandy clay, silty clay, or clay. Infiltration rate 0 to 0.05 inch/hr when wet.			
7	Waste Discharge Identification (WVID):		Curry Creek		(Step 3) Indicate the existing dominant non-built land Use Type (dropdown menu to right):		Pasture/Grassland/Range: 50% to 75% ground cover & not heavily grazed					
8	Date:		9-Jun-08		(Step 4) Indicate the proposed dominant non-built land Use Type (dropdown menu to right):		A mix of lawn, grass, pasture and tress covering less than 50% of the open space					
9	Sub Drainage Area Name (from map):		SAMPLE COMMERCIAL				Complete Either					
10	Runoff Curve Numbers						Sq Ft		Acres			
11	Existing Pervious Runoff Curve Number		84		(Step 5) Total Project Site Area:		100.00		100.00			
12	Proposed Development Pervious Runoff Curve Number		95		(Step 6) Sub-watershed Area:		100.00		100.00			
13	Design Storm				Percent of total project :		100%					
14	Based on the County you indicated above, we have included the 85 percentile average 24 hr event - P85 (in)^ for your area.		0.62		in							
15	The Amount of rainfall needed for runoff to occur (Existing runoff curve number -P from existing RCN (in)^)		0.38		In		(Step 7) Sub-watershed Conditions		Complete Either			
16	P used for calculations (in) (the greater of the above two criteria)		0.62		In		Sub-watershed Area (acres)		Calculated Acres			
17	^Available at www.cabmphandbooks.com						Sq Ft		Acres			
18					Existing Rooftop Impervious Coverage		0		0.00			
19					Existing Non-Rooftop Impervious Coverage		0		0.00			
20					Proposed Rooftop Impervious Coverage		28		28.00			
21					Proposed Non-Rooftop Impervious Coverage		47		47.00			
22					Credits		Acres		Square Feet			
23					Porous Pavement		0.00		0			
24					Tree Planting		7.50		326,700			
25	Pre-Project Runoff Volume (cu ft)		9,770		Cu.Ft.		Downspout Disconnection		609,840			
26	Project-Related Runoff Volume Increase w/o credits (cu ft)		140,300		Cu.Ft.		Impervious Area Disconnection		307,098			
27							Green Roof		0			
28							Stream Buffer		0			
29							Vegetated Swales		1,939,291			
30	Project-Related Volume Increase with Credits (cu ft)		-47,483		Cu.Ft.		Subtotal		3,182,929			
31					Subtotal Runoff Volume Reduction Credit		137066 Cu. Ft.					
32	You have achieved your minimum requirements				(Step 9) Impervious Volume Reduction Credits		Volume (cubic feet)					
33					Rain Barrels/Cisterns		0		Cu. Ft.			
34					Soil Quality		50,717		Cu. Ft.			
35					Subtotal Runoff Volume Reduction		50,717		Cu. Ft.			
36					Total Runoff Volume Reduction Credit		187,783		Cu. Ft.			

APPENDIX I

CD-ROM of Project Files

APPENDIX K

Oversized Exhibits